

SUPPLEMENTAL DATA REPORT

Sheldon Meadow

20 Hancock Street 1139 West Street

Wrentham, Massachusetts



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Existing Conditions

The subject site for the Sheldon Meadow Development exists as the eastern portion of property located at 1139 West Street and 20 Hancock Street, Wrentham, MA. The subject site is considered a single lot in common ownership, however for the purposes of the Sheldon Meadow Development, this report will focus on a portion of the 20 Hancock Street portion of the property and 1139 West Street Property with frontage on Hancock Street.

The property located totals approximately 20.16 acres in the R-87 Agricultural and Residential Zoning District. The parcel is developed by a single family home that is currently a rental property. The remainder of the property consists of field and patches of trees and shrubs that turn into dense forest as it approaches the wetlands and perennial stream to the west. The site is bounced to the north by an intermittent stream and to the south by single family residences.

Throughout the site, the topography generally slopes from northeast to southwest where a bordering vegetated wetland and a perennial stream exist. A portion of the front of the existing site flows to the abutter, directly southeast of the existing home on 20 Hancock Street. In addition, a portion of the southeastern edge of the property flows to the abutting property along the southeast edge of the site.

The existing parcel entirely drains to three analysis points representing the wetland system to the southwest of the site; the abutter along the southeastern property line; and the abutter to the southeast of the frontage on Hancock Street.

Soil conditions on site are characterized as Sudbury fine sandy loam which has a hydrologic soil group of B. Soil testing has been performed and has confirmed these soil groups.

The site currently services water and electric utility via the Hancock Street Right of Way. There is not believed to be the availability of gas or sewer services within the Hancock Street Right of Way.

Proposed Conditions

The Sheldon Meadow Development proposes to construct 16 single family homes within a Senior Living Community (SLC), with an internal, formal greenspace and paved walking loop and with an exterior accessible walking loop to add to the existing wooded trails that navigate through the wooded, natural areas adjacent to the proposed development. The exterior walking loop is proposed to be six (6) feet wide to aide in two-way walking pedestrian traffic and is also depicted with benches every 150'± apart to increase the usability.



The single-family homes have been located and designed as two clusters to provide a communal area between the clusters of homes. Both clusters feature entrances on the courtyard side of the units which enter the interconnected walkways. This allows direct connections from each unit to the communal spaces in the center of the development. The road for Sheldon Meadow wraps around the exterior of the clusters, where it closes a loop before exiting to the Hancock Street intersection.

The road has been designed as 22' wide, per SLC requirements, and totals approximately 1,771 LF. The exterior of the road is proposed to be curbed while the interior is proposed as a 10' pervious parking shoulder. The use of the pervious parking shoulder will allow for the reduction in overall runoff, while still allowing adequate parking on site.

Electric, cable, and communications service as well as water service will be provided through the available connections within Hancock Street. Sewer will be serviced through an on-site community septic system. The septic system is proposed to be located beneath the centralized green space within the community.

The proposed stormwater management utilizes treatment BMP's, grassed swales, subsurface infiltration systems, as well as an infiltration basin on the southwestern side of the site. Providing the low point on the southwestern edge of the site most closely mimics the existing topographical conditions and allows the site to remain as close to existing as possible. The main entry drive is proposed to be superelevated towards the northwest, where the stormwater will enter a catch basin to be treated, and flow to a subsurface infiltration system. The loop will be superelevated towards the outside of the road, entering the stormwater management system via curb cuts, or catch basins. The stormwater management system runs along the exterior of the road in the form of stone lined grassed swales leading to headwalls with subsurface piping. Stormwater will be piped toward proprietary treatment devices prior to be discharged to the infiltration basin. The infiltration basin will accept stormwater on site, further treat the stormwater, infiltrate, and release excess stormwater via an overflow weir. A portion of the proposed loop will drain to a catch basin which is connected to a subsurface infiltration system within the center common of the site. This subsurface infiltration system will treat, infiltrate, and release excess stormwater via an overflow grate within the center common. Discharged stormwater from the subsurface infiltration system will continue to flow southwest toward the infiltration basin where it will be captured, further treated, and infiltrated within the larger basin.

Using these series of treatment BMP's, swales, and the infiltration basin, the stormwater management system is designed to capture, treat, and infiltrate stormwater as required by the Massachusetts Stormwater Standards, as well as the local Wrentham Board of Health Stormwater Regulations. See the hydrology section of this report for further detail and information.



Zoning

The proposed parcel will remain within the R-87 Agricultural and Residential Zoning District. Within this district, a Senior Living Community (SLC) is approvable via a Site Plan and Special Permit Application to the Planning Board. This section with demonstrate compliance with appropriate dimensional requirements and special permit criteria as it relates to Section 13.5 (Senior Living Community) of the Wrentham Zoning Bylaws.

Dimensional Requirements

While the parcel exists within the R-87 Agricultural and Residential Zoning District, the SLC Special Permit allows adjusted dimensional requirements from the R-87 Agricultural and Residential Zoning District.

20 Hancock Street, Wrentham, MA R-87 Agricultural and Residential Proposed Use: Senior Living Community

Dimensional Requirements (R-87 Agricultural and Residential) (SLC)	Required	Proposed
Lot Area (SLC)	871,200 SF (20 AC)	878,327 SF (20.1 AC)
Continuous Lot Frontage (SLC)	100' min.	134'±
Minimum Front Yard (SLC)	30' min.	221'±
Minimum Side Yard (SLC)	30' min.	54'±
Minimum Rear Yard (SLC)	30' min.	1,810'±
Maximum Building Coverage (SLC)	35% max.	3.6%
Minimum Open Space (SLC)	30% min.	40%
Maximum Stories (SLC)	2 max.	2
Maximum Building Height (SLC)	28' max.	23'-8"
Maximum Density (SLC)	4 Units/AC	0.80 Units/AC
Average Distance Between (SLC)	15'	25.3'
Parking Requirements	Required	Proposed
Number of Parking Spaces	40 Spaces	32 Garage Spaces 29 Surface Spaces 61 Total Spaces



OPEN SPACE CALCULATION

Total Site Area = 878,327 SF Total Wetland Area = 445,084 SF Total Non-Usable Space = 145,385 SF Required Open Space = $(0.30) \times 878,327$ SF = 263,498 SF* *Per the requirements of the SLC, not more than 25% of the required common open space may be wetland.' Allowable Wetland Area = $(0.25) \times 263,498$ SF = 65,875 SF Upland Open Space = 878,327 SF - (445,084 SF + 145,385 SF) = 287,858 SF Total Open Space = 287,858 + 65,875 = 353,733 SF Open Space % = 353,733 SF / 878,327 SF = 40%

Senior Living Community – Special Permit Criteria

Per Wrentham Bylaws Section 13.5.4 – Basic Requirements

A. A Senior Living Community Shall Comply with the following density regulations: 4 Units/Acre

As shown within the Zoning Table, the Sheldon West project has an overall density of 0.80 Units/Acre.

B. Maximum building coverage shall not exceed thirty-five percent (35%) of the lot area for new construction or expansion of existing structures.

The proposed building coverage on site totals 3.6%.

C. For single family, cottage dwellings, duplexes or triplex style dwellings, the minimum setback shall be thirty feet (30') from all property lines in the Residential Districts, unless the Planning Board determines that a reduced setback is necessary to achieve the purposes of this section and will not have a detrimental impact on the neighborhood.

All units on site are proposed as single family. All proposed buildings remain at least thirty feet from all property lines.

D. No dwelling unit in a SLC shall have more than two bedrooms.

No dwelling unit is proposed to have more than two (2) bedrooms.



E. The minimum distance between buildings in any SLC shall be fifteen feet (15').

The minimum distance between buildings proposed on site is equal or more than 15'. Many of the buildings maintain approximately 20' or more of separation.

F. The minimum common open space in the development shall be thirty percent (30%) of the lot area and not more than twenty-five percent (25%) of the required minimum common open space shall consist of wetlands (as defined in MGL c.131, s40). The upland open space shall be contiguous and usable by residents of the development. A permanent Conversation Restriction running to or enforceable by the Town shall be recorded for the common open space area and shall include restrictions that the land be retained in perpetuity for conservation or passive recreation.

See Dimensional Requirements section for calculation of Common Open Space.

G. All SLC dwelling units shall be subject to an age restriction described in a deed, deed rider, restrictive covenant, or other document approved by the Planning Board that shall be recorded at the Registry of Deeds and/or Land Court. The age restriction shall limit occupancy of dwelling units to at least one individual age fifty-five (55) or over ad their spouse/partner and may provide for time-limited guest visitation rights of not more than one (1) month per year. The restriction, if the Planning Board so approved and specifies in the special permit, may authorize special exceptions that allow persons of all ages to live in a dwelling unit together with a senior resident for purposes such as care of a senior in ill health or enabling seniors to fulfill legal responsibilities of guardianship or custody. The special permit including age restriction shall run with the land in perpetuity and shall be enforceable by the Town and/or any owner(s) of the SLC dwelling units. In the event of the death of a qualifying owner or occupant(s) of a dwelling unit, or foreclosure or other involuntary transfer of a unit within the SLC, a one-year exemption to the restriction shall be allowed for the transfer of the unit to another eligible occupant.

This requirement is understood and agreeable to the applicant.

- H. Minimum off-street parking requirements shall comply with Article 6.4, except as modified by the following standards:
 - a. Single Family or Cottage style dwellings: two (2) spaces per unit
 - b. Guest parking: one (1) space per two (2) units or three (3) beds, as applicable.

- I. Per these requirements, the project is subject to the requirement of 40 parking spaces. The project proposes a total of 32 garage spaces and 29 surface spaces for a total of 61 proposed parking spaces on site.
- J. All streets within a SLC shall be private, and all sewerage, drainage facilities and utilities shall be designed and constructed in compliance with the Town of Wrentham Subdivision Rules and Regulations, except as modified by the following standards:
 - a. The minimum width of paved roadways shall be twenty-two feet (22').
 - b. There shall be a five-foot (5') sidewalk installed along one side of the roadway.

The roadway within the proposed SLC is shown at 22' wide, with an additional 10' wide pervious paver shoulder and a 6' sidewalk along the exterior loop of the roadway with direct connection to the intersection of Hancock Street.

K. A SLC may have one (1) free standing sign at each principal access to the development from a public way, indicating the name and/or street address of the SLC. Such sign shall not exceed twelve (12) square feet in area per side or four (4') feet in height. The provisions of Article 18 shall also apply to signage within the SLC.

Signage has not been proposed at this time. Proposed signage will be designed and addressed at a later date further along in the permitting process and will meet the requirements of the SLC.

L. A SLC shall have an amenity structure designed to allow for a variety of passive and active recreational activities that support the residents of the SLC. Such uses that may be considered are community program spaces, fitness/therapeutic space, educational, recreational and accessory space; areas for neighborhood meetings and event space; and any other amenities and opportunities that are intended to create and promote an integrated neighborhood type environment.

A community amenity space has been provided within the common green area on the interior of the proposed units. This space can be utilized for a number of different activities and includes an interconnected walking loop to each unit, as well an open green space to be utilized as desired by the residents. The project is also serviced by a 6' wide meandering sidewalk with benches placed intermittently at the outside of the project allowing a longer, uninterrupted loop at the edge of the wetland and forested areas for the use of the residents.



$Per \ Wrentham \ By laws \ Section \ 13.5.8-Development \ Standards$

As part of the Planning Board's special permit review process, the Board shall evaluate the proposed Senior Living Community (SLC) for conformance to the following minimum design standards.

A. Architectural planning and design shall incorporate energy efficient design techniques, such as natural heating and cooling systems, use of sun and wind energy generation systems, and so forth.

The architectural design of the single-family homes of the development will incorporate solar panel ready roof design for future installation of solar panels by unit owners. Also, all habitable rooms will have operable double hung windows to take advantage of natural cooling/ventilation at the unit owners' discretion. The building envelope will be high efficiency so that mechanical system design loads can be reduced and be more energy efficient. The mechanical heating and cooling systems will be high efficiency electric heat pump split systems in conjunction with electric high efficiency water heaters.

Also, exterior bollard style pedestrian lighting for the interior green space is a solar charged light fixture.

B. Structures located near the project property lines shall be designed and located in a manner that reflects consistency and compatibility with neighboring areas, and shall include appropriate use of building density, heights, and design to minimize intrusion on neighbors.

Though the structures are not near the property lines due to the nature of the development, the new homes being constructed could possibly be seen by adjacent abutting properties, so attention has been given to the design of the new homes as if they were close to the adjacent property lines. All the new homes are over 50' from adjacent property lines. The new homes being built are consistent with the existing neighborhood in building footprint as well as in building height. The design of the roof lines is consistent with the surrounding neighborhood incorporating a main gable roof, gable and shed dormers, asphalt shingle roofs, horizontal lap and shake siding and double hung windows. Options are provided to allow for diversity within the development and provide visual interest. Providing (3) garage door styles and (3) exterior siding color options also provide further design diversity within the development. Most importantly, the development is treating the new homes as having (2) front elevations – one front elevation facing the ring road and the adjacent abutters as well as one front elevation facing inward to the "village green". The architectural design will not detract from the current feel of the surrounding neighborhood as this development enhances, reflects and is consistent with the surrounding existing neighborhood. C. Outdoor recreation or gathering areas, particularly those that may generate significant noise and/or light and glare, shall be located to minimize intrusion on neighboring properties.

The outdoor gathering area is located away from all property lines within the center of all the units, proposed as a "village green". All noise, light, or glare generated from this area will be shielded by the proposed structures. There is also a meandering perimeter sidewalk along the ring road of the development that allows for pedestrian walking without having to cross individual home driveways making for an uninterrupted walk within benches placed intermittently along the path. This increases the safety of pedestrians walking in and around the development. Lighting is provided along the ring road that are full cut off light fixtures so that there will not be any light spillage or glare onto adjacent abutting properties. These measures minimize any intrusion on neighboring properties.

D. Structures shall be clustered to reduce site disturbance and protect open spaces, natural and environmentally sensitive areas.

The proposed new homes are clustered around an internal "village green" common area to minimize site disturbance and protect open spaces on the site. The site plan and building placement respects natural land features and environmentally sensitive areas of the site.

E. Site design shall limit large grass areas and provide adequate access to shared amenities.

The site has been designed utilizing interior sidewalks within the formal greenspace and an exterior walking loop to interconnect and create universal accessibility to all areas on site. Large grass areas are avoided and the natural environment is celebrated by clustering the project and limiting the land disturbance to allow for significant existing natural areas to be preserved by this development.

F. Building design shall avoid use of long unbroken facades, and shall include use of balconies, offset wall, trellises and other design elements to provide visual interest.

The structures have been designed with several gables to break up long facades on both the exterior facing and interior faces of the structure. Patios and porches have also been incorporated into the design to provide visual interest to the exterior of the building.

G. Building design, colors and materials shall generally correspond to the natural setting of the project site and promote the appearance of the Town's New England character.



The buildings have been designed with the New England aesthetic in mind utilizing colors such as brown, blue, and gray with vinyl shake siding, and vinyl lap siding. The visual design also promotes the New England aesthetic by incorporating gables to the structure.

H. Walking trails shall be accessible to all abilities and installed throughout the project.

An exterior walking loop has been proposed to meander around the exterior of the site. This walking loop, as well as the interior sidewalks interconnecting the units, has been proposed to be paved and adhere to all current ADA requirements.

I. The development shall be served by public water.

This development will be served by public water, accessed from the Hancock Street Right of Way.

Hydrology

Standard 1. No New Untreated Discharges

The Massachusetts Stormwater Handbook requires that the project demonstrates that no new stormwater conveyances (e.g. outfalls) discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

The proposed project will not discharge stormwater directly to, or cause erosion in, wetlands or water of the Commonwealth and will treat stormwater prior to discharge or infiltration.

The infiltration basin is adjacent to a wetland and has been proposed with an outlet weir to allow treated discharge to flow from the pond to the wetland. All outlets have been designed to incorporate rip rap to minimize or eliminate erosion to wetlands.

Storm Event	2-inch	2-year	10-year	50-year	100-year
AP1 Peak Discharge (cfs)	0.05	0.48	1.61	7.71	10.34

Standard 2. Post-development Peak Discharge Rates Not to Exceed Pre-development Peak Discharge Rates

Post-development peak discharge rates do not exceed the pre-development peak discharge rates and total runoff volumes for all storm events. The proposed condition reduces rates by collecting and controlling the stormwater runoff within the stormwater management system.

Storm Event	2-inch	2-year	10-year	50-year	100-year
Pre-development rates (cfs) AP1 to Wetland System	0.03	0.76	3.15	8.24	11.59
Volume (cf)	734	6,028	17,950	42,343	58,515
Post-development rates (cfs) AP1 to Wetland System	0.05	0.48	1.61	7.71	10.34
Volume (cf)	320	2,283	8,054	25,098	37,942
Rate reductions (cfs)	0.02	-0.28	-1.54	-0.53	-1.25
Volume Reductions (cf)	-414	-3745	-9,896	-17,245	-20,573

Storm Event	2-inch	2-year	10-year	50-year	100-year
Pre-development rates (cfs) AP2 to Abutter	0.03	0.62	2.29	5.66	7.84
Volume (cf)	648	4,218	11,807	26,912	36,798
Post-development rates (cfs) AP2 to Abutter	0.02	0.40	1.34	3.15	4.30
Volume (cf)	311	1,688	4,469	9,864	13,352
Rate reductions (cfs)	-0.01	-0.22	-0.95	-2.51	-3.54
Volume Reductions (cf)	-337	-2,530	-7,338	-17,048	-23,446



Storm Event	2-inch	2-year	10-year	50-year	100-year
Pre-development rates (cfs) AP3 to Abutter Depression	0.00	0.05	0.86	2.04	2.76
Volume (cf)	0	28	1,064	3,641	5,425
Post-development rates (cfs) AP3 to Abutter Depression	0.00	0.0	0.64	1.73	2.41
Volume (cf)	0	0	934	3,718	5,701
Rate reductions (cfs)	0.0	-0.05	-0.22	-0.31	-0.35
Volume Reductions (cf)	-0	-28	-130	77	276

*AP3 sees negligible increase in 50-Year and 100-Year 24-Hour Storm Events totaling 2% and 5%, respectively. Overall, there is a negligible **decrease** in ponding elevation on abutting property by 0.01' in the 100-Year Storm.

Standard 3. Minimize or Eliminate Loss of Annual Recharge to Groundwater

Groundwater recharge will be accomplished using the surface infiltration practices. As shown in the table summary for Standard 2, the project decreases the total volume and runoff for all storm events. All storms have a significant decrease over the existing condition for both volume and rate of runoff. This reduction in volume is generated by collecting and infiltrating all the impervious surfaces created on site.

RECHARGE VOLUME REQUIREMENT

- Rv = F x impervious area
- Rv = Required Recharge Volume, expressed in Ft3, cubic yards, or acre-feet
- **F**= Target Depth Factor associated with each Hydrologic Soil Group
- Impervious Area = pavement and rooftop area on site

RECHARGE VOLUME FOR THE ENTIRE SITE

Hydrologic Group Volume to Recharge (x Total Impervious Area)

A: 0.60 inches of runoff	No A soils were found on site
B: 0.35 inches of runoff	0.35 in x (1 ft/12 in) x 108,116 sf = 3,154 cf
C: 0.25 inches of runoff	No C soils were found on site
D: 0.10 inches of runoff	No D soils were found on site

■ Total Volume Provided Pond P1: 20,179 CF



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- Total Volume Provided in Subsurface Infiltration System 1 (SIS1): 2,620 CF
- Total Volume Provided in Subsurface Infiltration System 2 (SIS2): 3,670 CF

Capture Area Adjustment

Total Recharge volume required: 3,154 cf Impervious areas that drain to recharge areas: 476 SF Total Site Impervious/Impervious to Infiltration= 108,116 SF/ 107,640 SF = 1.004 Total adjusted recharge needed= 3,167 CF

Volumes and surface area for ponds acquired from HydroCAD stage storage tables. These tables are attached as an appendix at the end of the HydroCAD analysis.

TOTAL SITE RECHARGE PROVIDED = 26,469 CF RECHARGE VOLUME (BELOW THE OUTLET) > 3,167 CF REQUIRED

DRAWDOWN REQUIREMENT

DRAWDOWN WITHIN 72 HOURS

- Pond P1: 20,179 cf / [(8.27 in/hr)*(1 ft/12 in)*(8,532 sf)] = 3.43 hours < 72 hours, OK
- Pond SIS1: 2,620 cf / [(8.27 in/hr)*(1 ft/12 in)*(1,242 sf)] = 3.06 hours < 72 hours, OK
- Pond SIS2: 3,670 cf / [(8.27 in/hr)*(1 ft/12 in)*(1,735 sf)] = 3.07 hours < 72 hours, OK

10 YEAR DRAWDOWN WITHIN 24 HOURS

- Pond P1: 9,092 cf / [(8.27 in/hr)*(1 ft/12 in)*(8,532 sf)] = 1.55 hours < 24 hours, OK
- Pond SIS1: 676 cf / [(8.27 in/hr)*(1 ft/12 in)*(1,242 sf)] = 0.79 hours < 24 hours, OK
- Pond SIS2: 1,556 cf / [(8.27 in/hr)*(1 ft/12 in)*(1,735 sf)] = 1.30 hours < 24 hours, OK

Volumes and surface area for ponds acquired from HydroCAD stage storage tables. These tables are attached as an appendix at the end of the HydroCAD analysis.



Standard 4. Stormwater Management System to Remove 80% of the Average Annual Load of Total Suspended Solids (TSS)

The stormwater management system is designed to remove > 80% annual total suspended solids (TSS) from the proposed roadway, driveways, and sidewalks.

TSS REMOVAL CALCULATION

TREATMENT TRAIN #1 – CB TO ISOLATOR ROW TO SUBSURFACE INFILTRATION SYSTEM (R1, R2)

Area of Impervious = 21,788 SF

BMP	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load
Deep Sump				
Hooded Catch	0.25	1.00	0.25	0.75
Basin				
Isolator Row				
and Stormtech	0.80	0.75	0.60	0.15
Chambers				
	Tot	tal TSS Removal	85.0%	

TREATMENT TRAIN #2 – SWALE TO DOWNSTREAM DEFENDER TO INFILTRATION BASIN (S3, S4, S8)

Area of Impervious = 4,791 SF

BMP	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load
Downstream Defender	0.50	1.00	0.50	0.50
Infiltration Basin	0.80	0.50	0.40	0.10
Total TSS Removal			90.0%	

TREATMENT TRAIN #3 – RGB TO DOWNSTREAM DEFENDER TO INFILTRATION BASIN (R3, R9)

Area of Impervious = 13,690 SF



BMP	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load
Rain Guardian Bunker	0.91	1.00	0.91	0.09
Downstream Defender	0.50	0.09	0.05	0.04
Infiltration Basin	0.80	0.04	0.03	0.01
Total TSS Removal			99.0%	

TREATMENT TRAIN #4 – RGB TO DOWNSTREAM DEFENDER TO INFILTRATION BASIN (R8)

Area of Impervious = 11,017 SF

ВМР	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load
Rain Guardian Bunker	0.75	1.00	0.75	0.25
Downstream Defender	0.50	0.25	0.12	0.13
Infiltration Basin	0.80	0.13	0.10	0.03
Total TSS Removal			97.0%	

TREATMENT TRAIN #5 – CB TO DOWNSTREAM DEFENDER TO INFILTRATION BASIN (R4, R6, R7)

Area of Impervious = 34,915 SF

BMP	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load
Deep Sump				
Hooded Catch	0.25	1.00	0.25	0.75
Basin				



Downstream	0.50	0.75	0.37	0.38
Defender	0.50	0.75	0.57	0.30
Infiltration	0.80	0.38	0.30	0.08
Basin	0.80	0.58	0.50	0.08
	Total TSS Removal			

TREATMENT TRAIN #6 - RAIN GUARDIAN FOXHOLE TO INFILTRATION BASIN (R5)

BMP	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load
Rain Guardian Foxhole	0.79	1.00	0.79	0.21
Infiltration Basin	0.80	0.21	0.16	0.05
	Tot	tal TSS Removal	95.0%	

Area of Impervious = 17,705 SF

TREATMENT TRAIN #7 – UNTREATED IMPERVIOUS

Area of Impervious = 476 SF

■ No Treatment – 0%

WEIGHTED TSS REMOVAL CALCULATION

On-Site Impervious Area – 104,382 (Total analyzed impervious [112,470 SF] – off-site impervious [4,354 SF] – untreated clean roof runoff [3,734])

■ Treatment Train # 1 – 21,788 SF Percentage of Site Impervious = 21,788 SF / 104,382 SF = 20.9%

Weighted TSS Removal = 85% x 20.9% = 17.8%

Treatment Train # 2 - 4,791 SF
 Percentage of Site Impervious = 4,791 SF / 104,382 SF = 4.6%

Weighted TSS Removal = 90% x 4.6% = 4.1%



Treatment Train # 3 – 13,690 SF
 Percentage of Site Impervious = 13,690 SF / 104,382 SF = 13.1%

Weighted TSS Removal = 99% x 23.7% = 23.5%

Treatment Train # 4 – 11,017 SF
 Percentage of Site Impervious = 11,017 SF / 104,382 SF = 10.5%

Weighted TSS Removal = 97% x 23.7% = 23.5%

Treatment Train # 5 – 34,915 SF
 Percentage of Site Impervious = 34,915 SF / 104,382 SF = 33.4%

Weighted TSS Removal = 92% x 33.4% = 30.7%

Treatment Train # 6 – 17,705 SF
 Percentage of Site Impervious = 17,705 SF / 104,382 SF = 17.0%

Weighted TSS Removal = 95% x 17.0% = 16.1%

Treatment Train # 7 - 476 SF
 Percentage of Site Impervious = 476 SF / 104,382 SF = 0.5%

Weighted TSS Removal = $0\% \ge 0.5\% = 0\%$

Total Sitewide TSS removal = 17.8% + 4.1% + 13.1% + 10.5% + 30.7% +16.1% = 92.3% > 80% OK

WATER QUALITY VOLUME

For new development, stormwater management systems must be designed to remove 80% of the average annual load (post-development conditions) of Total Suspended Solids (TSS). It is presumed that this standard is met when:

- a) Suitable nonstructural practices for source control and pollution prevention are implemented.
- b) Stormwater management best management practices (BMPs) are sized to capture the prescribed runoff volume; and
- c) Stormwater management BMPs are maintained as designed.

In order to achieve the rated TSS Removal Rates, each BMP must be sized adequately. This development proposes to use ACF Rain Guardian Turrets, ACF Rain Guardian Foxholes, as well as



an infiltration basin. The ACF Rain Guardian Turrets and ACF Rain Guardian Foxholes are flow based devices, and the flow calculations can be found below.

Flow rate associated with ACF Rain Guardian Bunker 1:

 $\mathbf{Q} = (\mathbf{qu})^*(\mathbf{A})^*(\mathbf{W}\mathbf{Q}\mathbf{V}),$ where:

 $\mathbf{Q}=\mathbf{Peak}$ flow rate associated with first 1-inch of runoff

qu = the unit peak discharge, in csm/in (774 csm/in for Tc associated with 6 minutes)

A = impervious surface drainage area (in square miles)

WQV = water quality volume in watershed inches

ACF Rain Guardian Bunker 1 (RGB1):

ACF Rain Guardian Bunker rated for 75% removal up to 0.50 cfs ACF Rain Guardian Bunker rated for 91% removal up to 0.25 cfs

 $Q = (774 \text{ csm/in})^*(0.000230 \text{ square miles})^*(1 \text{ inch})$ Q = 0.18 CFS

Required Capacity = 0.18 CFS ACF Bunker 91% Removal Capacity = 0.25 CFS (See Appendix C for calculation)

 $0.25 \ \mathrm{CFS} > 0.18 \ \mathrm{CFS},$ OK 91% Removal

ACF Rain Guardian Bunker 2 (RGB2):

ACF Rain Guardian Bunker rated for 75% removal up to 0.50 cfs ACF Rain Guardian Bunker rated for 91% removal up to 0.25 cfs

 $Q = (774 \text{ csm/in})^*(0.000395 \text{ square miles})^*(1 \text{ inch})$ Q = 0.31 CFS

Required Capacity = 0.31 CFS ACF Bunker 75% Removal Capacity = 0.50 CFS (See Appendix C for calculation)

0.50 CFS > 0.31 CFS, **OK 75% Removal**



ACF Rain Guardian Bunker 3 (RGB3):

ACF Rain Guardian Bunker rated for 75% removal up to 0.50 cfs ACF Rain Guardian Bunker rated for 91% removal up to 0.25 cfs

Q = (774 csm/in)*(0.000260 square miles)*(1 inch) Q = 0.20 CFS

Required Capacity = 0.20 CFS ACF Bunker 91% Removal Capacity = 0.25 CFS (See Appendix C for calculation)

$0.25 \ \mathrm{CFS} > 0.20 \ \mathrm{CFS},$ OK 91% Removal

Downstream Defender 8ft dia. (DD-1): Downstream Defender rated for 50% removal up to 4.49 cfs

 $Q = (774 \text{ csm/in})^*(0.00159 \text{ square miles})^*(1 \text{ inch})$ Q = 1.23 CFS

Required Capacity = 1.23 CFS DD 8ft dia 50% Removal Capacity = 4.49 CFS

4.49 CFS > 1.23 CFS, **OK 50% Removal**

<u>Downstream Defender 8ft dia. (DD-2):</u>

 $Downstream \ Defender \ rated \ for \ 50\% \ removal \ up \ to \ 4.49 \ cfs$

Q = (774 csm/in)*(0.00072 square miles)*(1 inch) Q = 0.56 CFS

Required Capacity = 0.56 CFS DD 8ft dia 50% Removal Capacity = 4.49 CFS

 $4.49~\mathrm{CFS}$ > 0.56 CFS, OK 50% Removal

ACF Rain Guardian Foxhole (FH):

ACF Rain Guardian Foxhole rated for 79% removal up to 0.50 cfs



```
Q = (774 \text{ csm/in})*(0.000635 \text{ square miles})*(1 \text{ inch})
Q = 0.49 \text{ CFS}
```

Required Capacity = 0.49 CFS ACF Foxhole 79% Removal Capacity = 0.49 CFS (See Appendix C for calculation)

 $0.50~\mathrm{CFS} \geq 0.49~\mathrm{CFS},$ OK 79% Removal

ADS Stormtech SC-740 Isolator Row:

SIS1

 $Q = (774 \text{ csm/in})^*(0.00028 \text{ square miles})^*(1 \text{ inch})$ Q = 0.22 CFS

Required Capacity = 0.22 CFS ADS Stormtech SC-740 Isolator Row 80% Removal Capacity = 0.15 cfs/chamber x 8 chambers = 1.2 cfs

 $1.20~\mathrm{CFS} \geq 0.22~\mathrm{CFS},$ OK 80% Removal

$\mathbf{SIS2}$

$$\label{eq:Q} \begin{split} \mathbf{Q} &= (774 \mbox{ csm/in})^* (0.00050 \mbox{ square miles})^* (1 \mbox{ inch}) \\ \mathbf{Q} &= 0.39 \mbox{ CFS} \end{split}$$

Required Capacity = 0.39 CFS ADS Stormtech SC-740 Isolator Row 80% Removal Capacity = 0.15 cfs/chamber x 5 chambers = 0.75 cfs

 $0.75~\mathrm{CFS} \geq 0.39~\mathrm{CFS},$ OK 80% Removal



Standard 5. Land Uses with Higher Potential Pollutant Loads

The development is not considered a land use that generally produces higher potential pollutant loads.

Standard 6. Stormwater Discharges to Critical Areas

The proposed stormwater system does not discharge to a critical area.

Standard 7. Redevelopment Projects

The project is not considered a redevelopment project.

Standard 8. Control Construction-related Impacts

The project will install erosion and sediment controls prior to any earthwork activity. Erosion control barriers will be placed down slope from the proposed construction to prevent erosion and sedimentation into the surrounding areas. The barriers will be maintained and inspected periodically during construction; sediment buildup will be removed, and any damaged barrier will be replaced as needed.

Standard 9. Long-Term Operation and Maintenance Plan

See **Appendix A** for the operation and maintenance requirements of the stormwater management system.

Standard 10. No Illicit Discharges

An illicit discharge compliance statement will be provided by the property owner under separate cover.

Appendix A – Operation and **Maintenance Plan**



Infiltration Basin

System Owner: Sheldon Meadow, LLC, or future owner. Estimated Annual Maintenance: \$1,000 (Per DEP Stormwater Structural BMP's Vol 2)

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Inspect the basin and outlet structure to ensure no structural damage has occurred and that they are functioning properly and up to design standards.

Inspection and preventive maintenance are required at least twice per year, and after each major storm event. Note how long water remains standing in the basin after a storm. If water remains standing after 48 to 72 hours after a storm, the infiltration basin may be clogged.

At least twice per year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings, accumulated organic matter, trash and debris at this time.

Remove sediment from the basin as necessary when the basin is dry. Use light equipment when removing the top layer, as to not compact the underlying soil. Use deep tilling to break and remove any clogged surfaces and revegetate immediately.

Important items to check during inspections include:

- Signs of differential settlement
- Cracking
- Erosion
- Leakage in the embankments
- Tree growth on the embankments
- Condition of rip rap
- Sediment accumulation
- Health of vegetation, turf



* Paying careful attention to pretreatment, and operation and maintenance can extend the life of the soil media

Date	Inspector	Condition	Maintenance Performed*



Deep Sump Hooded Catch Basin

System Owner: Sheldon Meadow, LLC, or future owner. Estimated Annual Maintenance: \$2,000-\$4,000

(Per DEP Stormwater Structural BMP's Vol 2)

Inspect or clean deep sump basins at least four times per year and at the end of the foliage and snow removal seasons. Sediments must also be removed four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin. If handling runoff from land uses with higher potential pollutant loads or discharging runoff near or to a critical area, more frequent cleaning may be necessary. Clamshell buckets are typically used to remove sediment in Massachusetts. However, vacuum trucks are preferable because they remove more trapped sediment and supernatant than clamshells. Vacuuming is also a speedier process and is less likely to snap the cast iron hood within the deep sump catch basin.

Date	Inspector	Condition	Maintenance Performed*



ACF Rain Guardian Bunker

System Owner: Sheldon Meadow, LLC, or future owner. Estimated Annual Maintenance: \$250-\$500 (Per Manufacturer)

Depending on the characteristics of the contributing watershed and seasonal variation, common maintenance needs include periodic removal of accumulated leaves (and other organic debris) and garbage from the top grate and sediment and fine debris from the concrete dry filter box. Contributing watersheds with high sediment concentrations may require inspections monthly and clean them out at least four times a year. More frequent visits may be needed to satisfy maintenance needs.

If sediment accumulates beyond an acceptable level in the system, it will be necessary to remove. This can be done by manual removal with a shovel or mechanical device. The filter screen can be cleaned manually through brushing or with pressurized water.

Date	Inspector	Condition	Maintenance Performed*



ACF Rain Guardian Foxhole

System Owner: Sheldon Meadow, LLC, or future owner. Estimated Annual Maintenance: \$250-\$500 (Per Manufacturer)

Depending on the characteristics of the contributing watershed and seasonal variation, common maintenance needs include periodic removal of accumulated leaves (and other organic debris) and garbage from the top grate and sediment and fine debris from the concrete dry filter box. Contributing watersheds with high sediment concentrations may require inspections monthly and clean them out at least four times a year. More frequent visits may be needed to satisfy maintenance needs.

If sediment accumulates beyond an acceptable level in the system, it will be necessary to remove. This can be done by manual removal with a shovel or mechanical device. The filter screen can be cleaned manually through brushing or with pressurized water.

Date	Inspector	Condition	Maintenance Performed*



Subsurface Infiltration System

System Owner: Sheldon Meadow, LLC, or future owner.

Estimated Annual Maintenance: \$500-\$750

(Per DEP Stormwater Structural BMP's Vol 2)

For the first 3 months after construction, the subsurface infiltration system should be inspected after every storm greater than 1" for standing water for periods in excess of 72 hours. Therein after, the subsurface infiltration system should be inspected biannually. If standing water is observed for longer than 72 hours, a pump should be placed in the basin and discharged through the outlet pipe. After the system is dewatered, it should be observed by a Professional Engineer. A Professional Engineer should provide an opinion as to why the infiltration system is not draining and provide recommendations to restore infiltration capacity to the system.

Date	Inspector	Condition	Maintenance Performed*



Stormtech Isolator Row Plus

System Owner: Sheldon Meadow, LLC, or future owner.

Estimated Annual Maintenance: \$250-\$500

(Per StormTech Maintenance Manual)

In the first year of operation, the Isolator Row should be inspected every 6 months for depth of sediment. Therein after, the Isolator Row should be inspected annually. If sediment is present, a stadia rod should be inserted into the inspection port to determine depth of sediment. If/when the depth exceeds 3 inches throughout the length of the Isolator Row, clean out should be performed. Please see the Isolator Row Maintenance Manual for cleanout procedures.

Date	Inspector	Condition	Maintenance Performed*



Rip Rap and Swale Areas

System Owner: Sheldon Meadow, LLC, or future owner.

Estimated Annual Maintenance: \$250-\$500

Inspect semi-annually the first year, and at least once a year thereafter. For swales inspect the grass for growth and the side slopes for signs of erosion and formation of rills and gullies. Plant an alternative grass species if the original grass cover is not successfully established. If grass growth is impaired by winter road salt or other deicer use, re-establish the grass in the spring. For rip-rap and swale areas: *Trash/Debris Removal:* Remove accumulated trash and debris. *Sediment removal:* Check on a yearly basis and clean as needed. Use hand methods (i.e., a person with a shovel) when cleaning to minimize disturbance to vegetation and or rip rap and underlying soils. Mow on an as-needed basis during the growing season so that the grass height does not exceed 6 inches.

Date	Inspector	Condition	Maintenance Performed*



Downstream Defender

System Owner: Sheldon Meadow, LLC, or future owner. Estimated Annual Maintenance: \$500-\$750 See manufacturer specific maintenance information below.



Appendix B – Erosion and Sediment Control Notes and General Construction Sequence



Erosion and Sediment Control Notes

- Erosion and sediment control measures must be installed prior to the start of construction and maintained and upgraded as necessary during construction by the contractor. It is the contractor's responsibility to inspect and install additional control measures as needed during construction.
- 2) All catch basins receiving drainage from the project site must be provided with a catch basin filter.
- 3) Stabilization of all re-graded and soil stockpile areas must be maintained during all phases of construction.
- 4) Sediment removed from erosion and sediment control devices must be properly removed and disposed. All damaged controls must be removed and replaced.
- 5) The contractor is responsible for implementing the erosion and sediment control plan. This includes the installation and maintenance of control measures, informing all parties engaged on the construction site of the requirements and objectives of the plan, and notifying the proper city agency of any transfer of this responsibility.
- 6) The contractor shall be responsible for controlling wind erosion and dust throughout the life of his contract. Dust control may include, but is not limited to, sprinkling of water on exposed soils and street sweeping adjacent roadways.
- 7) If final grading is to be delayed for more than 21 days after land disturbance activities cease, temporary vegetation or mulch shall be used to stabilize soils within 14 days of the last disturbance.
- 8) If a disturbed area will be exposed for greater than one year, permanent grasses or other approved cover must be installed.
- 9) The contractor must keep on-site at all times additional silt fence and hay bales for the installation at the direction of the engineer or the city to mitigate any emergency condition.
- 10) The construction fencing and erosion and sediment controls as shown may not be practical during all stages of construction. Earthwork activity on-site must be done in a manner such that runoff is directed to a sediment control device or infiltrated to the ground.
- 11) Demolition and construction debris must be properly contained and disposed of.
- 12) Disposal of all demolished materials is the responsibility of the contractor and must be hauled off-site in accordance with all federal, state, and local requirements.



General Construction Sequence

- 1) Install erosion and sediment controls prior to starting any earthworks activity.
- 2) Begin clearing, grubbing and demolition.
- 3) Begin utility installations.
- 4) Construct building foundation.
- 5) Install site furnishings.
- 6) Install landscaping.
- 7) Erosion and sediment controls shall be maintained until permanent cover is established.



Appendix C – ACF Rain Guardian Supplemental Information





RAIN GUARDIAN TURRET AND FOXHOLE ENGINEERING PROPERTIES

RAIN GUARDIAN TURRET:

Turret Flow Rate Capacity:

Outflow is possible through three locations. Please note the vertical filter within the chamber was assumed to be 100% clogged because its primary function is to allow the chamber to dry out between rain events.

1) Filter overflow – water can pass between the top of the filter and the bottom of the metal grate; calculated using the continuity equation (i.e. $Q=V^*A$)

2) Grate overflow – water can pass through the top metal grate beyond the vertical filter wall; calculated using an orifice equation (i.e. $Q=0.0108*A*\sqrt{d}$)

3) High volume overflow – water can overtop the front debris wall onto the splash pad; calculated using a standard broad crested weir equation (i.e. $Q=C*L*H^{(3/2)}$)

Filter overflow – 0.45 CFS

Grate overflow – 2.59 CFS

Emergency overflow - 0.41 CFS

TOTAL: 3.45 CFS

Turret Internal Storage Vol: (i.e. storage capacity below the top of the filter wall): 4.02 ft³

RAIN GUARDIAN FOXHOLE:

Below are the flow and storage data for the Rain Guardian Foxhole with an inlet, middle, and outlet (i.e. 6' top lid). (the addition of mid section (for longer units) would improve the sediment storage capacity).

Foxhole Flow Rate Capacity:

Outflow is possible through three locations. Please note the vertical filter within the chamber was assumed to be 100% clogged because its primary function is to allow the chamber to dry out between rain events.

1) Filter overflow – water can pass between the top of the filter and the bottom of the metal grate; calculated using the continuity equation (i.e. $Q=V^*A$)

2) Grate overflow – water can pass through the top metal grate beyond the vertical filter wall; calculated using an orifice equation (i.e. $Q=0.0108*A*\sqrt{d}$)

3) High volume overflow – water can overtop the front debris wall onto the splash pad; calculated using a standard broad crested weir equation (i.e. $Q=C*L*H^{(3/2)}$)

Filter overflow – 0.30 CFS Grate overflow – 2.69 CFS Emergency overflow - 0.52 CFS TOTAL: **3.51 CFS**

<u>Foxhole Internal Storage Volume</u> (i.e. storage capacity below the top of the filter wall):

Inlet + Outlet: 2.0 ft^3

Middle: 2.65 ft^3

TOTAL: 4.65 ft³

From:	Lee Jones
To:	Patrick Bogle
Subject:	FW: ACF Environmental - Rain Guardians
Date:	Friday, December 4, 2020 9:30:52 AM
Attachments:	2017.11.21 Flow Data.pdf

Patrick,

Good morning

Great talking with you yesterday.

Per our discussion, I have reviewed the study.

The study was performed by flow rates which are shown and not by storm events. I am also attaching the Flow Data pdf.

- Rain Guardian Gross Solids and Sediment Removal Report
 - Bunker sediment capture 75.6% at 0.5 CFS and 91.7% at 0.25 CFS
 - Bunker gross solids capture 61.4% at 0.5 CFS and 78.8% at 0.25 CFS
 - Turret sediment capture 79.1% at 0.5 CFS and 88.4% at 0.25 CFS
 - Turret gross solids capture 72.4% at 0.5 CFS and 86.7% at 0.25 CFS
 - NOTE: Grass and rock lined inlets were also tested and achieved similar removal efficiencies. While the grass lined inlet and rock lined inlets removed similar amounts of sediment under the flow rates tested, the ease of maintenance, long-term effectiveness, storage capacity, and stability of the Rain Guardians set them apart from the grass and rock. The 'Maintenance Considerations' section (5.4) on pages 65 68 of the report highlights some advantages of the Rain Guardian products.

Please let me know if I can be of help Best regards, Lee

Leland (Lee) Jones, QSM BMP Specialist – New England ACF Environmental 508-745-7052 cell <u>ljones@acfenv.com</u> <u>www.acfenvironmental.com</u> "Start each day with a positive thought and a grateful heart" Roy T. Bennett



Appendix D – ADS Stormtech Isolator Row TSS Removal Rates

4. Technology

Specific size/capacity of MTD assessed (include units):

The StormTech Isolator row can be sized to meet the needs of the project. Sizing can be either volume based for sites with good infiltrative soils, or the more commonly used practice of rate based sizing using a maximum water quality flow of less than 2.5 gpm/sqft of bottom area using two layers of woven geotextile (315W by ADS).

Range of drainage areas served by MTD (acres):

Site configuration is the only limiting factor as to the drainage area that can be served. In general, StormTech recommends the length of the Isolator Row be limited to less than 175 feet for cleaning/maintenance purposes. Multiple Isolator Rows can be placed side by side to increase the size of the area served.

Include sizing chart or describe sizing criteria:

Volume Based - For sites with good infiltration rates, a volume based approach can be used with a corresponding stage storage curves based on the number of StormTech Isolator Chambers provided. Bare chamber storage volumes listed in table are in cubic feet per chamber:

SC-310	14.7	cf/chamber			
SC-740	45.9	cf/chamber			
DC-780	46.2	cf/chamber			
MC3500	113.0	cf/chamber			
MC4500	106.5	cf/chamber			
Table 1 - Storage Volume per Chamber					

Rate Based - For sites with where a combination of infiltration and detention/retention is used, at rate based approach is typically used. The treatments rates are based on available surface treatment area and factors of safety that were developed from extensive testing. 80% TSS removal and 40% TP removal can be achieved by sizing the Isolator Rows base on these maximum flows per chamber:

	Specific	Bottom	Flow Per
Chamber	Flow Rate	Area	Chamber
SC-310	2.5 gpm/sf	17.7 sf	0.10 cfs
RC-310	2.5 gpm/sf	17.7 sf	0.10 cfs
SC-740	2.5 gpm/sf	27.8 sf	0.15 cfs
RC-750	2.5 gpm/sf	27.8 sf	0.15 cfs
DC-780	2.5 gpm/sf	27.8 sf	0.15 cfs
MC-3500	2.5 gpm/sf	43.2 sf	0.24 cfs
MC-4500	2.5 gpm/sf	30.1 sf	0.17 cfs

Table 2 – Treatment Rate per Chamber



Appendix E – Downstream Defender Supplemental Information





Operation and Maintenance Manual

Downstream Defender®

Vortex Separator for Stormwater Treatment

Turning Water Around ...®

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 - Determining You Maintenance Schedule
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 - Inspection
 - Floatables and Sediment Cleanout
- 8 Downstream Defender[®] Installation Log
- 9 Downstream Defender[®] Inspection and Maintenance Log

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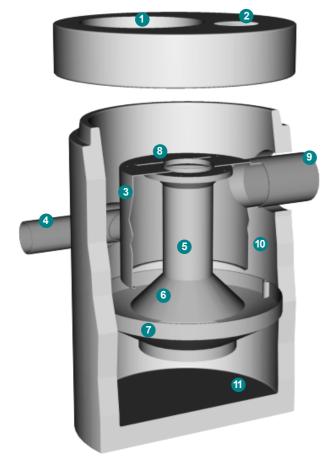
DISCLAIMER: Information and data contained in this manual is exclusively for the purpose of assisting in the operation and maintenance of Hydro International plc's Downstream Defender[®]. No warranty is given nor can liability be accepted for use of this information for any other purpose. Hydro International plc have a policy of continuous product development and reserve the right to amend specifications without notice.

Downstream Defender® by Hydro International

The Downstream Defender[®] is an advanced Hydrodynamic Vortex Separator designed to provide high removal efficiencies of settleable solids and their associated pollutants, oil, and floatables over a wide range of flow rates.

The Downstream Defender[®] has unique, flow-modifying internal components developed from extensive full-scale testing, CFD modeling and over thirty years of hydrodynamic separation experience in wastewater, combined sewer and stormwater applications. These internal components distinguish the Downstream Defender[®] from simple swirl-type devices and conventional oil/grit separators by minimizing turbulence and headlosses, enhancing separation, and preventing washout of previously stored pollutants.

The high removal efficiencies and inherent low headlosses of the Downstream Defender[®] allow for a small footprint making it a compact and economical solution for the treatment of non-point source pollution.



Benefits of the Downstream Defender®

- · Removes sediment, floatables, oil and grease
- No pollutant washouts
- Small footprint
- · No loss of treatment capacity between clean-outs
- Low headloss
- Efficient over a wide ranges of flows
- · Easy to install
- · Low maintenance

Applications

- · New developments and retrofits
- · Utility yards
- · Streets and roadways
- Parking lots
- · Pre-treatment for filters, infiltration and storage
- · Industrial and commercial facilities
- Wetlands protection

Downstream Defender® Components

- 1. Central Access Port
- 2. Floatables Access Port (6-ft., 8-ft. and 10-ft. models only)
- 3. Dip Plate
- 4. Tangential Inlet
- 5. Center Shaft
- 6. Center Cone
- 7. Benching Skirt
- 8. Floatables Lid
- 9. Outlet Pipe
- 10. Floatables Storage
- 11. Isolated Sediment Storage Zone

Operation

Introduction

The Downstream Defender[®] operates on simple fluid hydraulics. It is self-activating, has no moving parts, no external power requirement and is fabricated with durable non-corrosive components. No manual procedures are required to operate the unit and maintenance is limited to monitoring accumulations of stored pollutants and periodic clean-outs. The Downstream Defender[®] has been designed to allow for easy and safe access for inspection/monitoring and clean-out procedures. Entry into the unit or removal of the internal components is not necessary for maintenance, thus safety concerns related to confined-space-entry are avoided.

Pollutant Capture and Retention

The internal components of the Downstream Defender[®] have been designed to protect the oil, floatables and sediment storage volumes so that separator performance is not reduced as pollutants accumulate between clean-outs. Additionally, the Downstream Defender[®] is designed and installed into the storm drain system so that the vessel remains wet between storm events. Oil and floatables are stored on the water surface in the outer annulus separate from the sediment storage volume in the sump of the unit providing the option for separate oil disposal, and accessories such as adsorbant pads. Since the oil/floatables and sediment storage volumes are isolated from the active separation region, the potential for re-suspension and washout of stored pollutants between clean-outs is minimized.

Wet Sump

The sump of the Downstream Defender[®] retains a standing water level between storm events. The water in the sump prevents stored sediment from solidifying in the base of the unit. The cleanout procedure becomes more difficult and labor intensive if the system allows fine sediment to dry-out and consolidate. Dried sediment must be manually removed by maintenance crews. This is a labor intensive operation in a hazardous environment.

Blockage Protection

The Downstream Defender[®] has large clear openings and no internal restrictions or weirs, minimizing the risk of blockage and hydraulic losses. In addition to increasing the system headloss, orifices and internal weirs can increase the risk of blockage within the unit.

Maintenance

Overview

The Downstream Defender[®] protects the environment by removing a wide range of pollutants from stormwater runoff. Periodic removal of these captured pollutants is essential to the continuous, long-term functioning of the Downstream Defender[®]. The Downstream Defender[®] will capture and retain sediment and oil until the sediment and oil storage volumes are full to capacity. When sediment and oil storage capacities are reached, the Downstream Defender[®] will no longer be able to store removed sediment and oil. Maximum pollutant storage capacities are provided in Table 1.

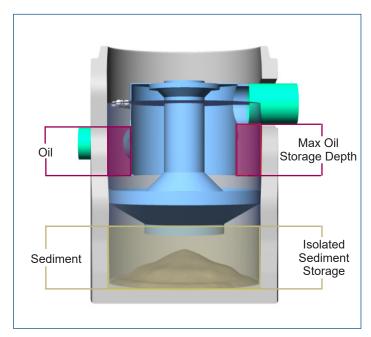


Fig.1 Pollutant storage volumes of the Downswtream Defender®.

The Downstream Defender[®] allows for easy and safe inspection, monitoring and clean-out procedures. A commercially or municipally owned sump-vac is used to remove captured sediment and floatables. Access ports are located in the top of the manhole. On the 6-ft, 8-ft and 10-ft units, the floatables access port is above the outlet pipe between the concrete manhole wall and the dip plate. The sediment removal access ports for all Downstream Defender[®] models are located directly over the hollow center shaft.

Maintenance events may include Inspection, Oil & Floatables Removal, and Sediment Removal. Maintenance events do not require entry into the Downstream Defender[®], nor do they require the internal components of the Downstream Defender[®] to be removed. In the case of inspection and floatables removal, a vactor truck is not required. However, a vactor truck is required if the maintenance event is to include oil removal and/or sediment removal.

Determining Your Maintenance Schedule

The frequency of cleanout is determined in the field after installation. During the first year of operation, the unit should be inspected every six months to determine the rate of sediment and floatables accumulation. A simple probe such as a Sludge Judge[®] can be used to determine the level of accumulated solids stored in the sump. This information can be recorded in the maintenance log (see page 9) to establish a routine maintenance schedule.

The vactor procedure, including both sediment and oil/flotables removal, for a 6-ft Downstream Defender[®] typically takes less than 30 minutes and removes a combined water/oil volume of about 500 gallons.

Inspection Procedures

Inspection is a simple process that does not involve entry into the Downstream Defender[®]. Maintenance crews should be familiar with the Downstream Defender[®] and its components prior to inspection.

Scheduling

- It is important to inspect your Downstream Defender[®] every six months during the first year of operation to determine your site-specific rate of pollutant accumulation
- Typically, inspection may be conducted during any season of the year
- Sediment removal is not required unless sediment depths exceed 75% of maximum clean-out depths stated in Table 1

Recommended Equipment

- Safety Equipment and Personal Protective Equipment (traffic cones, work gloves, etc.)
- · Crow bar or other tool to remove grate or lid
- Pole with skimmer or net
- Sediment probe (such as a Sludge Judge®)
- Trash bag for removed floatables
- Downstream Defender® Maintenance Log

Unit Diameter	Total Oil Storage	Oil Clean-out Depth	Total Sediment Storage	Sediment Clean-out Depth	Max. Liquid Volume Removed
(feet)	(gallons)	(inches)	(gallons)	(inches)	(gallons)
4	70	<16	141	<18	384
6	216	<23	424	<24	1,239
8	540	<33	939	<30	2,884
10	1,050	<42	1,757	<36	5,546
12	1,770	<49	2,970	<42	9,460

Table 1. Downstream Defender® Pollutant Storage Capacities and Max. Cleanout Depths.

NOTES

1. Refer to Dowmstream Defender® Clean-out Detail (Fig. 1) for measurement of depths.

2. Oil accumulation is typically less than sediment, however, removal of oil and sediment during the same service is recommended.

3. Remove floatables first, then remove sediment storage volume.

4. Sediment removal is not required unless sediment depths exceed 75% of maximum clean-out depths stated in Table 1.

Downstream Defender® Operation and Maintenance Manual





Fig.5





- Inspection Procedures
- Set up any necessary safety equipment around the access port or grate of the Downstream Defender[®] as stipulated by local ordinances. Safety equipment should notify passing pedestrian and road traffic that work is being done.
- Remove the lids to the manhole (Fig. 4). NOTE: The 4-ft Downstream Defender[®] will only have one lid.
- Without entering the vessel, look down into the chamber to inspect the inside. Make note of any irregularities. See Fig.7 and 8 for typical inspection views.
- Without entering the vessel, use the pole with the skimmer net to remove floatables and loose debris from the outer annulus of the chamber.
- Using a sediment probe such as a Sludge Judge[®], measure the depth of sediment that has collected in the sump of the vessel (Fig.5).
- 6. On the Maintenance Log (see page 9), record the date, unit location, estimated volume of floatables and gross debris removed, and the depth of sediment measured. Also note any apparent irregularities such as damaged components or blockages.



Fig.7 View over center shaft into sediment storage zone.

- 7. Securely replace the grate or lid.
- 8. Take down safety equipment.
- Notify Hydro International of any irregularities noted during inspection.

Floatables and Sediment Cleanout

Floatables cleanout is typically done in conjunction with sediment removal. A commercially or municipally owned sump-vac is used to remove captured sediment and floatables (Fig.6).

Floatables and loose debris can also be netted with a skimmer and pole. The access port located at the top of the manhole provides unobstructed access for a vactor hose and skimmer pole to be lowered to the base of the sump.

Scheduling

- Floatables and sump cleanout are typically conducted once a year during any season.
- If sediment depths are greater than 75% of maximum cleanout depths stated in Table 1, sediment removal is required.
- Floatables and sump cleanout should occur as soon as possible following a spill in the contributing drainage area.



Fig.8 View of outer annulus of floatables and oil collection zone.

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Downstream Defender® Operation and Maintenance Manual

Recommended Equipment

- Safety Equipment (traffic cones, etc)
- · Crow bar or other tool to remove grate or lid
- Pole with skimmer or net (if only floatables are being removed)
- Sediment probe (such as a Sludge Judge®)
- Vactor truck (6-inch flexible hose recommended)
- Downstream Defender® Maintenance Log
- Set up any necessary safety equipment around the access port or grate of the Downstream Defender[®] as stipulated by local ordinances. Safety equipment should notify passing pedestrian and road traffic that work is being done.
- Remove the lids to the manhole (NOTE: The 4-ft Downstream Defender[®] will only have one lid).
- **3.** Without entering the vessel, look down into the chamber to inspect the inside. Make note of any irregularities.
- **4.** Using the Floatables Port for access, remove oil and floatables stored on the surface of the water with the vactor hose or the skimmer net (Fig.9).
- Using a sediment probe such as a Sludge Judge[®], measure the depth of sediment that has collected in the sump of the vessel and record it in the Maintenance Log (Pg.9).
- Once all floatables have been removed, drop the vactor hose to the base of the sump via the Central Access Port. Vactor out the sediment and gross debris off the sump floor (Fig.6).

- 7. Retract the vactor hose from the vessel.
- On the Maintenance Log provided by Hydro International, record the date, unit location, estimated volume of floatables and gross debris removed, and the depth of sediment measured. Also note any apparent irregularities such as damaged components or blockages.
- 9. Securely replace the grate or lid.

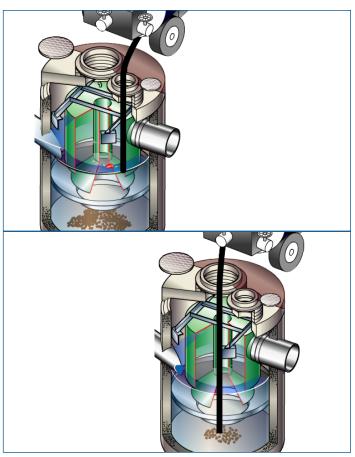


Fig.9 Floatables and sediment are removed with a vactor hose

Maintenance at a Glance

Activity	Frequency
Inspection	 Regularly during first year of installation Every 6 months after the first year of installation
Oil and Floatables Removal	 Once per year, with sediment removal Following a spill in the drainage area
Sediment Removal	- Once per year or as needed - Following a spill in the drainage area
NOTE: For most clean	nouts it is not necessary to remove the entire volume of liquid in the vessel.

Only removing the first few inches of oils/floatables and the sediment storage volume is required.

HYDRO MAINTENANCE SERVICES

Hydro International has been engineering stormwater treatment systems for over 30 years. We understand the mechanics of removing pollutants from stormwater and how to keep systems running at an optimal level.

NOBODY KNOWS OUR SYSTEMS BETTER THAN WE DO



AVOID SERVICE NEGLIGENCE

Sanitation services providers not intimately familiar with stormwater treatment systems are at risk of the following:

- Inadvertently breaking parts or failing to clean/replace system components appropriately.
- Charging you for more frequent maintenance because they lacked the tools to service your system properly in the first place.
- Billing you for replacement parts that might have been covered under your Hydro warranty plan
- Charging for maintenance that may not yet have been required.

LEAVE THE DIRTY WORK TO US

Trash, sediment and polluted water is stored inside treatment systems until they are removed by our team with a vactor truck. Sometimes teams must physically enter the system chambers in order to prepare the system for maintenance and install any replacement parts. Services include but are not limited to:

- · Solids removal
- · Removal of liquid pollutants
- Replacement media installation (when applicable)



BETTER TOOLS, BETTER RESULTS

Not all vactor trucks are created equal. Appropriate tools and suction power are needed to service stormwater systems appropriately. Companies who don't specialize in stormwater treatment won't have the tools to properly clean systems or install new parts.



SERVICE WARRANTY

Make sure you're not paying for service that is covered under your warranty plan. Only Hydro International's service teams can identify tune-ups that should be on us, not you.

TREATMENT SYSTEMS SERVICED BY HYDRO:

- Stormwwater filters
- Stormwater separators
- Baffle boxes
- · Biofilters/biorention systems
- Storage structures
- Catch basins
- Stormwater ponds
- Permeable pavement



SAVE TIME & MONEY: CALL HYDRO FOR A QUOTE

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Downstream Defender® Installation Log

HYDRO INTERNATIONAL REFERENCE NUMBER:				
SITE NAME:				
SITE LOCATION:				
OWNER:	CONTRACTOR:			
CONTACT NAME:	CONTACT NAME:			
COMPANY NAME:	COMPANY NAME:			
ADDRESS:	ADDRESS:			
TELEPHONE:	TELEPHONE:			
FAX:	FAX:			

INSTALLATION DATE: / /

MODEL (CIRCLE ONE):	4-FT	6-FT	8-FT	10-FT	CUSTOM
---------------------	------	------	------	-------	--------



Downstream Defender[®] Inspection and Maintenance Log

Date	Initials	Depth of Floatables and Oils	Sediment * Depth Measured	Volume of Sediment Removed	Site Activity and Comments

*Note: Sediment removal is not required unless sediment depths exceed 75% of maximum clean-out depths stated in Table 1.



DO IT RIGHT THE FIRST TIME

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CALL 1 (888) 382-7808 TO SCHEDULE AN INSPECTION

Stormwater Solutions

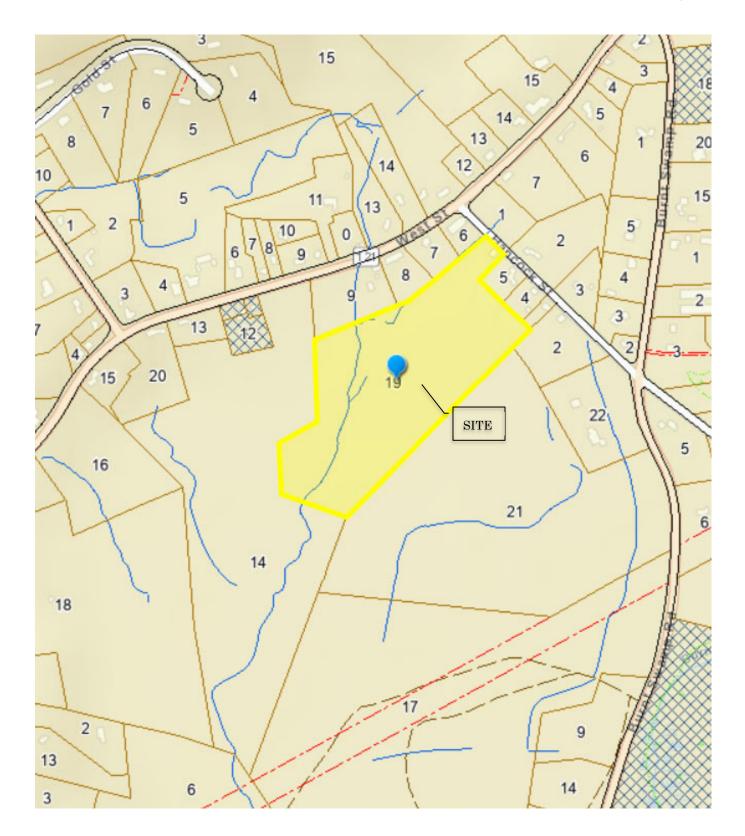
94 Hutchins Drive Portland, ME 04102

Tel: (207) 756-6200 Fax: (207) 756-6212 stormwaterinquiry@hydro-int.com

www.hydro-int.com

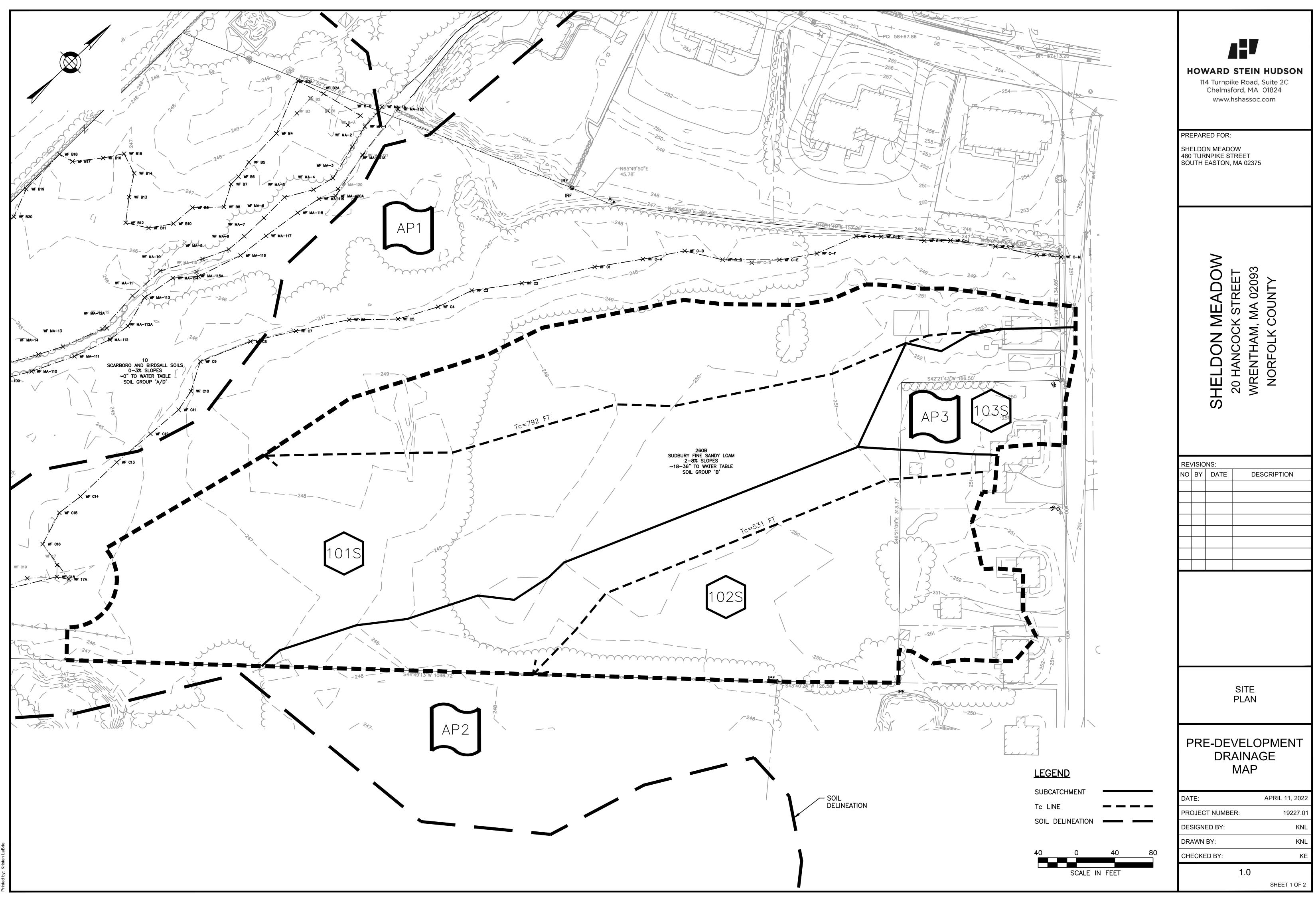


Appendix F – Locus Map

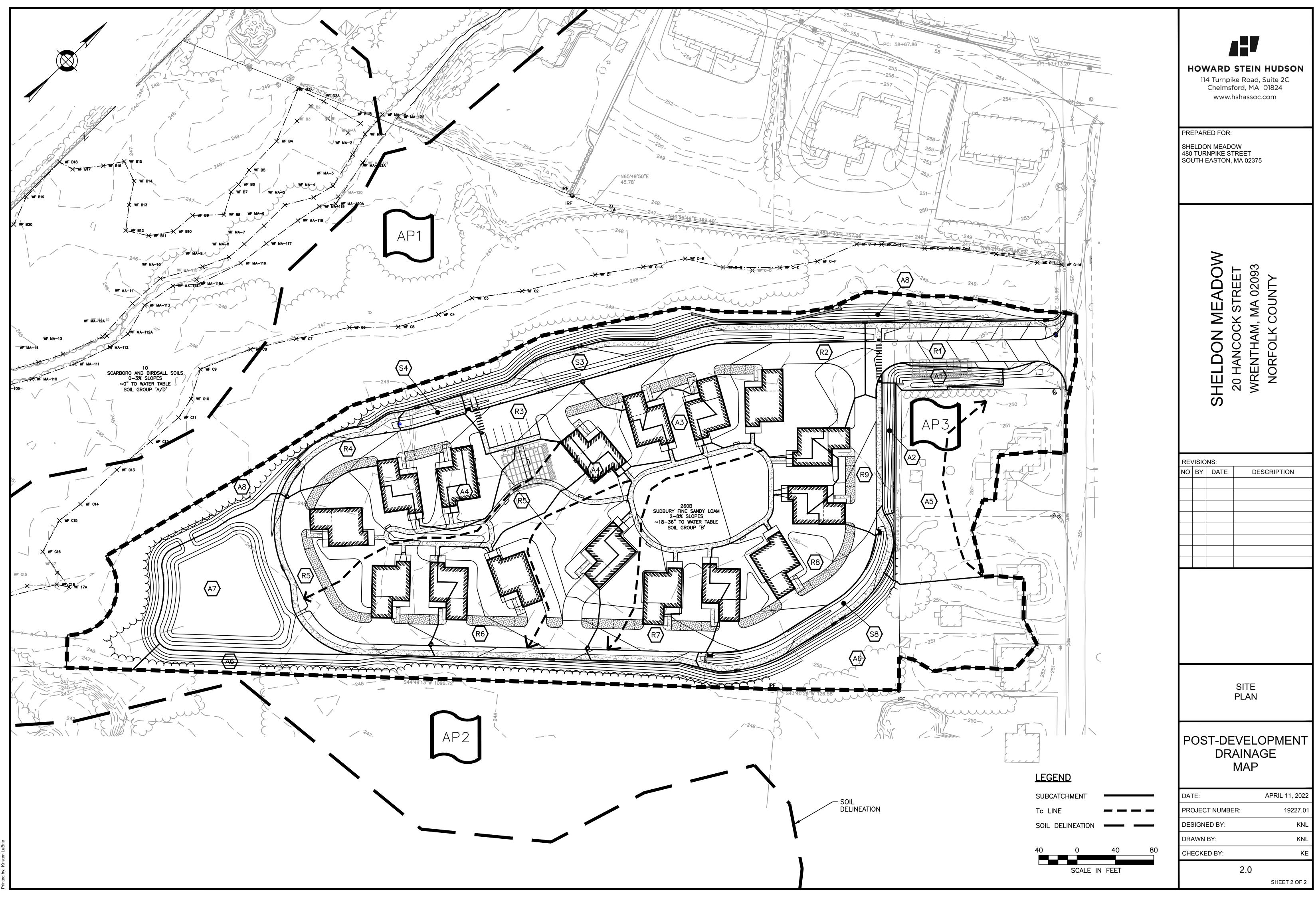
LOCUS MAP 20 Hancock Street &1139 West Street – Wrentham, MA 



Appendix G – Pre and Post Drainage Maps

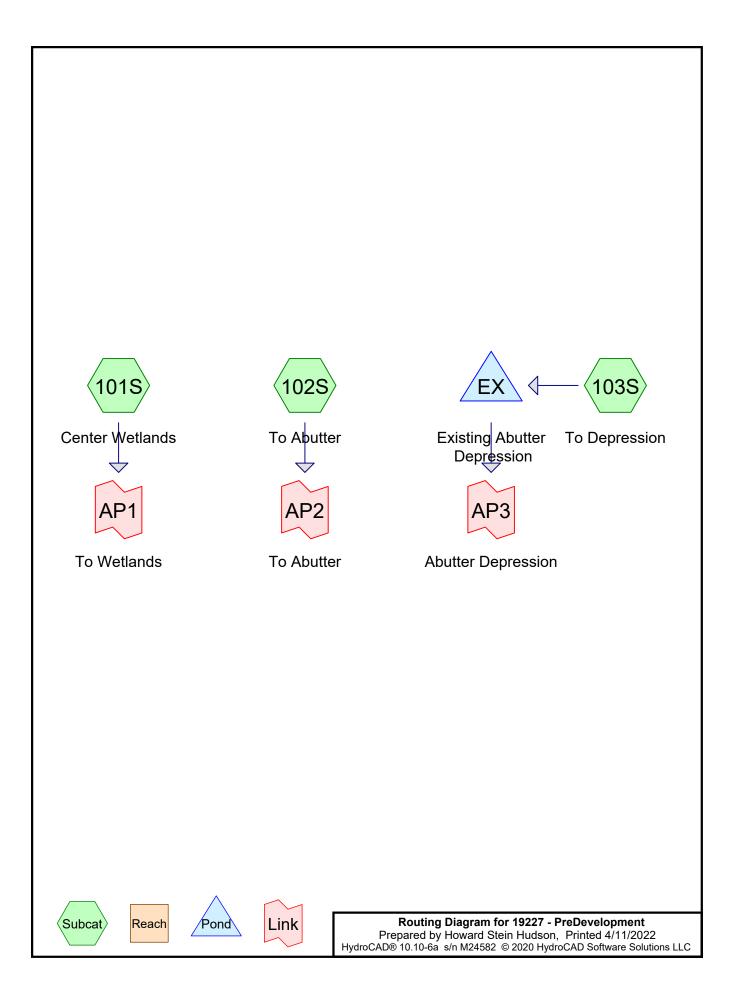


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Appendix H – HydroCAD, Stage Storage and Hydrographs



Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Inch	Type III 24-hr		Default	24.00	1	2.00	2
2	2-yr	Type III 24-hr		Default	24.00	1	3.27	2
3	10-yr	Type III 24-hr		Default	24.00	1	4.92	2
4	50-yr	Type III 24-hr		Default	24.00	1	7.42	2
5	100-yr	Type III 24-hr		Default	24.00	1	8.86	2

Rainfall Events Listing

Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
205,109	61	>75% Grass cover, Good, HSG B (101S, 102S, 103S)
3,780	98	Paved parking, HSG B (101S, 102S, 103S)
5,006	98	Roofs, HSG B (101S, 102S, 103S)
99,520	55	Woods, Good, HSG B (101S, 102S)
313,415	60	TOTAL AREA

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
313,415	HSG B	101S, 102S, 103S
0	HSG C	
0	HSG D	
0	Other	
313,415		TOTAL AREA

19227 - PreDevelopment

Prepared by Howard Stein Hudson	
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Printed 4/11/2022 Page 5

Sub	Ground	Total	Other	HSG-D	HSG-C	HSG-B	HSG-A
Nun	Cover	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)
-	>75% Grass	205,109	0	0	0	205,109	0
	cover, Good						
	Paved parking	3,780	0	0	0	3,780	0
	Roofs	5,006	0	0	0	5,006	0
	Woods, Good	99,520	0	0	0	99,520	0
	TOTAL AREA	313,415	0	0	0	313,415	0

Ground Covers (all nodes)

19227 - PreDevelopment Prepared by Howard Stein Hudson <u>HydroCAD® 10.10-6a s/n M24582 © 2020 Hy</u>	<i>Type III 24-hr 2-Inch Rainfall=2.00"</i> Printed 4/11/2022 droCAD Software Solutions LLC Page 6							
Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method								
Subcatchment101S: Center Wetlands	Runoff Area=182,426 sf 0.74% Impervious Runoff Depth>0.05" Flow Length=789' Tc=24.4 min CN=59 Runoff=0.03 cfs 734 cf							
Subcatchment 102S: To Abutter	Runoff Area=107,837 sf 3.38% Impervious Runoff Depth>0.07" Flow Length=530' Tc=20.7 min CN=61 Runoff=0.03 cfs 648 cf							
Subcatchment103S: To Depression	Runoff Area=23,152 sf 16.37% Impervious Runoff Depth>0.17" Tc=6.0 min CN=67 Runoff=0.04 cfs 334 cf							
Pond EX: Existing Abutter Depression Discarde	Peak Elev=249.91' Storage=30 cf Inflow=0.04 cfs 334 cf ed=0.02 cfs 333 cf Primary=0.00 cfs 0 cf Outflow=0.02 cfs 333 cf							
Link AP1: To Wetlands	Inflow=0.03 cfs 734 cf Primary=0.03 cfs 734 cf							
Link AP2: To Abutter	Inflow=0.03 cfs 648 cf Primary=0.03 cfs 648 cf							

Link AP3: Abutter Depression

Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Total Runoff Area = 313,415 sf Runoff Volume = 1,716 cfAverage Runoff Depth = 0.07"97.20% Pervious = 304,629 sf2.80% Impervious = 8,786 sf

Summary for Subcatchment 101S: Center Wetlands

Runoff = 0.03 cfs @ 14.99 hrs, Volume= Routed to Link AP1 : To Wetlands 734 cf, Depth> 0.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Inch Rainfall=2.00"

Α	rea (sf)	CN E	Description			
	943	98 F	aved park	ing, HSG B	3	
1	02,339	61 >	75% Gras	s cover, Go	bod, HSG B	
	78,735	55 V	Voods, Go	od, HSG B		
	409	<u>98</u> F	<u>Roofs, HSG</u>	ЪВ		
	82,426		Veighted A			
1	81,074	-		vious Area		
	1,352 0.74% Impervious Area					
_						
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
4.7	50	0.0300	0.18		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.27"	
12.5	524	0.0100	0.70		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
7.2	215	0.0100	0.50		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
24.4	789	Total				

Summary for Subcatchment 102S: To Abutter

Runoff = 0.03 cfs @ 13.81 hrs, Volume= Routed to Link AP2 : To Abutter 648 cf, Depth> 0.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Inch Rainfall=2.00"

Α	rea (sf)	CN E	Description		
	1,237	98 F	aved park	ing, HSG B	}
	83,409	61 >	75% Gras	s cover, Go	bod, HSG B
	20,785	55 V	Voods, Go	od, HSG B	
	2,406	<u>98</u> F	<u>Roofs, HSG</u>	ЪВ	
	07,837	61 V	Veighted A	verage	
	04,194	9	6.62% Per	vious Area	
	3,643	3	.38% Impe	ervious Are	а
_		<u>.</u>			
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.3	50	0.0100	0.11		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
12.7	446	0.0070	0.59		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.7	34	0.0300	0.87		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
20.7	530	Total			

Summary for Subcatchment 103S: To Depression

Runoff	=	0.04 cfs @	12.31 hrs,	Volume=	334 cf,	Depth> 0.17	, 11
Routed	to Pond	I EX : Existing	g Abutter De	epression			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Inch Rainfall=2.00"

Ar	rea (sf)	CN	Description					
	1,600	98	Paved park	ing, HSG B	В			
	19,361	61	>75% Ġras	s cover, Go	ood, HSG B			
	2,191	98	Roofs, HSO	БВ				
	23,152	67	Weighted A	verage				
	19,361	1	83.63% Pervious Area					
	3,791		16.37% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)				
6.0					Direct Entry,			

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	23,152 sf, 16.37% Impervious,	Inflow Depth > 0.17" for 2-Inch event
Inflow =	0.04 cfs @ 12.31 hrs, Volume=	334 cf
Outflow =	0.02 cfs @ 12.59 hrs, Volume=	333 cf, Atten= 46%, Lag= 16.8 min
Discarded =	0.02 cfs @ 12.59 hrs, Volume=	333 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf
Routed to Link	AP3 : Abutter Depression	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 249.91' @ 12.59 hrs Surf.Area= 385 sf Storage= 30 cf

Plug-Flow detention time= 9.4 min calculated for 333 cf (100% of inflow) Center-of-Mass det. time= 8.6 min (955.4 - 946.8)

Volume	Invert	Avail.Sto	rage S	Storage D	escription		
#1	249.80'	1,12	22 cf 🕻	Custom S	tage Data (Pr	r ismatic) Listed below	v (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4 250.6	80 90 20 40	urf.Area (sq-ft) 164 562 1,261 2,144 3,127	Inc.S <u>(cubic-f</u>		Cum.Store (cubic-feet) 0 73 255 595 1,122		
Device #1 #2	Routing Discarded Primary	Invert 249.80' 250.25'	2.410 Condu 20.0' I Head	ong x 15 (feet) 0.2	Itration over Groundwater E .0' breadth B 0 0.40 0.60		ngular Weir)1.60

Discarded OutFlow Max=0.02 cfs @ 12.59 hrs HW=249.91' (Free Discharge) **1=Exfiltration** (Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=249.80' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link AP1: To Wetlands

Inflow Are	a =	182,426 sf,	0.74% Impervious,	Inflow Depth >	0.05"	for 2-Inch event
Inflow	=	0.03 cfs @	14.99 hrs, Volume=	734 c	f	
Primary	=	0.03 cfs @	14.99 hrs, Volume=	734 c	f, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: To Abutter

Inflow Are	a =	107,837 sf,	3.38% Impervious,	Inflow Depth > 0.0	7" for 2-Inch event
Inflow	=	0.03 cfs @ 1	13.81 hrs, Volume=	648 cf	
Primary	=	0.03 cfs @ ´	13.81 hrs, Volume=	648 cf, A	tten= 0%, Lag= 0.0 min

Summary for Link AP3: Abutter Depression

Inflow Area =		23,152 sf,	16.37% Impervious,	Inflow Depth = 0.00"	for 2-Inch event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

19227 - PreDevelopment Prepared by Howard Stein Hudson HydroCAD® 10.10-6a s/n M24582 © 2020 F	<i>Type III 24-hr 2-yr Rainfall=3.27"</i> Printed 4/11/2022 IydroCAD Software Solutions LLC Page 14				
Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method					
Subcatchment 101S: Center Wetlands	Runoff Area=182,426 sf 0.74% Impervious Runoff Depth>0.40" Flow Length=789' Tc=24.4 min CN=59 Runoff=0.76 cfs 6,028 cf				
Subcatchment102S: To Abutter	Runoff Area=107,837 sf 3.38% Impervious Runoff Depth>0.47" Flow Length=530' Tc=20.7 min CN=61 Runoff=0.62 cfs 4,218 cf				
Subcatchment103S: To Depression	Runoff Area=23,152 sf 16.37% Impervious Runoff Depth>0.72" Tc=6.0 min CN=67 Runoff=0.38 cfs 1,395 cf				
Pond EX: Existing Abutter Depression Discarded=	Peak Elev=250.26' Storage=337 cf Inflow=0.38 cfs 1,395 cf 0.09 cfs 1,366 cf Primary=0.05 cfs 28 cf Outflow=0.14 cfs 1,394 cf				
Link AP1: To Wetlands	Inflow=0.76 cfs 6,028 cf Primary=0.76 cfs 6,028 cf				
Link AP2: To Abutter	Inflow=0.62 cfs 4,218 cf Primary=0.62 cfs 4,218 cf				
Link AP3: Abutter Depression	Inflow=0.05 cfs 28 cf Primary=0.05 cfs 28 cf				
Total Runoff Area = 313,415	sf Runoff Volume = 11,642 cf Average Runoff Depth = 0.45" 97.20% Pervious = 304,629 sf 2.80% Impervious = 8,786 sf				

Summary for Subcatchment 101S: Center Wetlands

Runoff = 0.76 cfs @ 12.51 hrs, Volume= Routed to Link AP1 : To Wetlands 6,028 cf, Depth> 0.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.27"

A	rea (sf)	CN E	Description		
	943	98 F	aved park	ing, HSG B	}
1	02,339	61 >	75% Gras	s cover, Go	bod, HSG B
	78,735	55 V	Voods, Go	od, HSG B	
	409	98 F	Roofs, HSG	ЪВ	
1	82,426		Veighted A		
1	81,074	9	9.26% Per	vious Area	
	1,352	0	.74% Impe	ervious Are	а
_				_	
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.7	50	0.0300	0.18		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
12.5	524	0.0100	0.70		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
7.2	215	0.0100	0.50		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
24.4	789	Total			

Summary for Subcatchment 102S: To Abutter

Runoff = 0.62 cfs @ 12.41 hrs, Volume= Routed to Link AP2 : To Abutter 4,218 cf, Depth> 0.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.27"

	Area (sf)	CN E	Description		
	1,237	98 F	aved park	ing, HSG B	3
	83,409	61 >	75% Gras	s cover, Go	bod, HSG B
	20,785	55 V	Voods, Go	od, HSG B	
	2,406	98 F	Roofs, HSG	6 B	
	107,837	61 V	Veighted A	verage	
	104,194	g	6.62% Per	vious Area	
	3,643	3	.38% Impe	ervious Are	а
_					
Tc		Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.3	50	0.0100	0.11		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
12.7	446	0.0070	0.59		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.7	34	0.0300	0.87		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
20.7	530	Total			

Summary for Subcatchment 103S: To Depression

Runoff 0.38 cfs @ 12.11 hrs, Volume= 1,395 cf, Depth> 0.72" = Routed to Pond EX : Existing Abutter Depression

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.27"

A	rea (sf)	CN [Description		
	1,600	98 F	Paved park	ing, HSG B	3
	19,361	61 >	>75% Gras	s cover, Go	ood, HSG B
	2,191	98 F	Roofs, HSC	БВ	
	23,152	67 \	67 Weighted Average		
	19,361	8	83.63% Pervious Area		
	3,791		16.37% Imp	pervious Ar	rea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	
6.0					Direct Entry,

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	23,152 sf, 16.37% Impervious,	Inflow Depth > 0.72" for 2-yr event
Inflow =	0.38 cfs @ 12.11 hrs, Volume=	1,395 cf
Outflow =	0.14 cfs @ 12.49 hrs, Volume=	1,394 cf, Atten= 63%, Lag= 22.7 min
Discarded =	0.09 cfs @ 12.49 hrs, Volume=	1,366 cf
Primary =	0.05 cfs @ 12.49 hrs, Volume=	28 cf
Routed to Link	AP3 : Abutter Depression	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 250.26' @ 12.49 hrs Surf.Area= 1,523 sf Storage= 337 cf

Plug-Flow detention time= 39.4 min calculated for 1,391 cf (100% of inflow) Center-of-Mass det. time= 38.8 min (923.1 - 884.4)

Volume	Invert	Avail.Stor	rage Stora	ge Description		
#1	249.80'	1,12	22 cf Cust	om Stage Data (P	rismatic)Listed belov	v (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4	t) 30 90 20 40	urf.Area (sq-ft) 164 562 1,261 2,144 2 127	Inc.Store (cubic-feet) 0 73 182 341	(cubic-feet) 0 73 255 595		
250.6		3,127	527	1,122		
Device	Routing	Invert	Outlet Dev	ices		
#1	Discarded	249.80'		r Exfiltration over tv to Groundwater	Surface area Elevation = 247.00'	Phase-In= 0.01'
#2	Primary	250.25'	20.0' long Head (feet	x 15.0' breadth B) 0.20 0.40 0.60	Broad-Crested Recta 0.80 1.00 1.20 1.40 70 2.64 2.63 2.64	n gular Weir D 1.60

Discarded OutFlow Max=0.09 cfs @ 12.49 hrs HW=250.26' (Free Discharge) **1=Exfiltration** (Controls 0.09 cfs)

Primary OutFlow Max=0.05 cfs @ 12.49 hrs HW=250.26' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.05 cfs @ 0.25 fps)

Summary for Link AP1: To Wetlands

Inflow Area	a =	182,426 sf,	0.74% Impervious,	Inflow Depth >	0.40"	for 2-yr event
Inflow	=	0.76 cfs @ 1	12.51 hrs, Volume=	6,028 c	f	
Primary	=	0.76 cfs @	12.51 hrs, Volume=	6,028 c	f, Atter	n= 0%, Lag= 0.0 min

Summary for Link AP2: To Abutter

Inflow Area	a =	107,837 sf,	3.38% Impervious,	Inflow Depth >	0.47"	for 2-yr event
Inflow	=	0.62 cfs @	12.41 hrs, Volume=	4,218 c	f	
Primary	=	0.62 cfs @	12.41 hrs, Volume=	4,218 c	f, Atter	n= 0%, Lag= 0.0 min

Summary for Link AP3: Abutter Depression

Inflow Are	a =	23,152 sf,	16.37% Impervious,	Inflow Depth = 0.01"	for 2-yr event
Inflow	=	0.05 cfs @	12.49 hrs, Volume=	28 cf	
Primary	=	0.05 cfs @	12.49 hrs, Volume=	28 cf, Atter	n= 0%, Lag= 0.0 min

19227 - PreDevelopment Prepared by Howard Stein Hudson HydroCAD® 10.10-6a s/n M24582 © 2020	<i>Type III 24-hr 10-yr Rainfall=4.92"</i> Printed 4/11/2022 HydroCAD Software Solutions LLC Page 22
Runoff by SCS	.00-24.00 hrs, dt=0.05 hrs, 481 points TR-20 method, UH=SCS, Weighted-CN Ind method . Pond routing by Dyn-Stor-Ind method
Subcatchment 101S: Center Wetlands	Runoff Area=182,426 sf 0.74% Impervious Runoff Depth>1.18" Flow Length=789' Tc=24.4 min CN=59 Runoff=3.15 cfs 17,950 cf
Subcatchment 102S: To Abutter	Runoff Area=107,837 sf 3.38% Impervious Runoff Depth>1.31" Flow Length=530' Tc=20.7 min CN=61 Runoff=2.29 cfs 11,807 cf
Subcatchment103S: To Depression	Runoff Area=23,152 sf 16.37% Impervious Runoff Depth>1.75" Tc=6.0 min CN=67 Runoff=1.03 cfs 3,368 cf
Pond EX: Existing Abutter Depression Discarded=0.	Peak Elev=250.31' Storage=427 cf Inflow=1.03 cfs 3,368 cf I1 cfs 2,287 cf Primary=0.86 cfs 1,064 cf Outflow=0.97 cfs 3,352 cf
Link AP1: To Wetlands	Inflow=3.15 cfs 17,950 cf Primary=3.15 cfs 17,950 cf
Link AP2: To Abutter	Inflow=2.29 cfs 11,807 cf Primary=2.29 cfs 11,807 cf
Link AP3: Abutter Depression	Inflow=0.86 cfs 1,064 cf Primary=0.86 cfs 1,064 cf
Total Runoff Area = 313,41	5 sf Runoff Volume = 33,124 cf Average Runoff Depth = 1.27" 97.20% Pervious = 304,629 sf 2.80% Impervious = 8,786 sf

Summary for Subcatchment 101S: Center Wetlands

Runoff = 3.15 cfs @ 12.39 hrs, Volume= 17,950 cf, Depth> 1.18" Routed to Link AP1 : To Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=4.92"

 A	rea (sf)	CN E	Description		
	943	98 F	aved park	ing, HSG B	
1	02,339	61 >	75% Gras	s cover, Go	bod, HSG B
	78,735	55 V	Voods, Go	od, HSG B	
	409	98 F	Roofs, HSG	ЪВ	
1	82,426	59 V	Veighted A	verage	
1	81,074	g	9.26% Per	vious Area	
	1,352	C	.74% Impe	ervious Are	а
_				_	
Tc	Length	Slope	Velocity	Capacity	Description
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.7	50	0.0300	0.18		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
12.5	524	0.0100	0.70		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
7.2	215	0.0100	0.50		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
24.4	789	Total			

Summary for Subcatchment 102S: To Abutter

Runoff = 2.29 cfs @ 12.32 hrs, Volume= 11,807 cf, Depth> 1.31" Routed to Link AP2 : To Abutter

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=4.92"

_	A	rea (sf)	CN E	Description		
		1,237	98 F	aved park	ing, HSG B	}
		83,409	61 >	75% Gras	s cover, Go	bod, HSG B
		20,785	55 V	Voods, Go	od, HSG B	
_		2,406	98 F	Roofs, HSG	ЪВ	
	1	07,837	61 V	Veighted A	verage	
	1	04,194	ç	6.62% Per	vious Area	
		3,643	3	.38% Impe	ervious Are	а
	_		. .			
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.3	50	0.0100	0.11		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.27"
	12.7	446	0.0070	0.59		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.7	34	0.0300	0.87		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	20.7	530	Total			

Summary for Subcatchment 103S: To Depression

Runoff = 1.03 cfs @ 12.10 hrs, Volume= 3,368 cf, Depth> 1.75" Routed to Pond EX : Existing Abutter Depression

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=4.92"

A	rea (sf)	CN [Description		
	1,600	98 F	Paved park	ing, HSG B	3
	19,361	61 >	>75% Gras	s cover, Go	ood, HSG B
	2,191	98 F	Roofs, HSC	БВ	
	23,152	67 \	Neighted A	verage	
	19,361 83.63% Pervious Area				a
	3,791		16.37% Imp	pervious Ar	rea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	
6.0					Direct Entry,

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	23,152 sf,	16.37% Impervious,	Inflow Depth > 1.75"	for 10-yr event					
Inflow =	1.03 cfs @	12.10 hrs, Volume=	3,368 cf						
Outflow =	0.97 cfs @	12.13 hrs, Volume=	3,352 cf, Atter	n= 6%, Lag= 1.8 min					
Discarded =	0.11 cfs @	12.13 hrs, Volume=	2,287 cf						
Primary =	0.86 cfs @	12.13 hrs, Volume=	1,064 cf						
Routed to Link AP3 : Abutter Depression									

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 250.31' @ 12.13 hrs Surf.Area= 1,763 sf Storage= 427 cf

Plug-Flow detention time= 33.2 min calculated for 3,345 cf (99% of inflow) Center-of-Mass det. time= 30.4 min (886.2 - 855.8)

Volume	Invert	Avail.Sto	rage Storag	e Description		
#1	249.80'	1,12	22 cf Custo	m Stage Data (Pi	rismatic)Listed below	v (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4 250.6	t) 0 0 0 0 0	urf.Area (sq-ft) 164 562 1,261 2,144 3,127	Inc.Store (cubic-feet) 0 73 182 341 527	Cum.Store (cubic-feet) 0 73 255 595 1,122		
<u>Device</u> #1 #2	Routing Discarded Primary	<u>Invert</u> 249.80' 250.25'	Outlet Devic 2.410 in/hr Conductivity 20.0' long 2 Head (feet)	Exfiltration over to Groundwater x 15.0' breadth B 0.20 0.40 0.60	Surface area Elevation = 247.00' road-Crested Recta 0.80 1.00 1.20 1.40 70 2.64 2.63 2.64	n gular Weir D 1.60

Discarded OutFlow Max=0.11 cfs @ 12.13 hrs HW=250.31' (Free Discharge) **1=Exfiltration** (Controls 0.11 cfs)

Primary OutFlow Max=0.84 cfs @ 12.13 hrs HW=250.31' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.84 cfs @ 0.67 fps)

Summary for Link AP1: To Wetlands

Inflow Area	a =	182,426 sf,	0.74% Impervious,	Inflow Depth > 1.18"	for 10-yr event
Inflow	=	3.15 cfs @ 1	12.39 hrs, Volume=	17,950 cf	
Primary	=	3.15 cfs @ 1	12.39 hrs, Volume=	17,950 cf, Atte	n= 0%, Lag= 0.0 min

Summary for Link AP2: To Abutter

 Inflow Area =
 107,837 sf,
 3.38% Impervious, Inflow Depth >
 1.31" for
 10-yr event

 Inflow =
 2.29 cfs @
 12.32 hrs, Volume=
 11,807 cf

 Primary =
 2.29 cfs @
 12.32 hrs, Volume=
 11,807 cf, Atten= 0%, Lag= 0.0 min

Summary for Link AP3: Abutter Depression

Inflow Area	a =	23,152 sf,	16.37% Impervious,	Inflow Depth = 0.55"	for 10-yr event
Inflow	=	0.86 cfs @	12.13 hrs, Volume=	1,064 cf	
Primary	=	0.86 cfs @	12.13 hrs, Volume=	1,064 cf, Atter	n= 0%, Lag= 0.0 min

19227 - PreDevelopment Prepared by Howard Stein Hudson HydroCAD® 10.10-6a s/n M24582 © 2020	<i>Type III 24-hr 50-yr Rainfall=7.42"</i> Printed 4/11/2022 HydroCAD Software Solutions LLC Page 30
Runoff by SCS	0.00-24.00 hrs, dt=0.05 hrs, 481 points TR-20 method, UH=SCS, Weighted-CN Ind method - Pond routing by Dyn-Stor-Ind method
Subcatchment 101S: Center Wetlands	Runoff Area=182,426 sf 0.74% Impervious Runoff Depth>2.79" Flow Length=789' Tc=24.4 min CN=59 Runoff=8.24 cfs 42,343 cf
Subcatchment102S: To Abutter	Runoff Area=107,837 sf 3.38% Impervious Runoff Depth>2.99" Flow Length=530' Tc=20.7 min CN=61 Runoff=5.66 cfs 26,912 cf
Subcatchment103S: To Depression	Runoff Area=23,152 sf 16.37% Impervious Runoff Depth>3.64" Tc=6.0 min CN=67 Runoff=2.22 cfs 7,026 cf
Pond EX: Existing Abutter Depression Discarded=0.	Peak Elev=250.36' Storage=519 cf Inflow=2.22 cfs 7,026 cf 12 cfs 3,319 cf Primary=2.04 cfs 3,641 cf Outflow=2.16 cfs 6,960 cf
Link AP1: To Wetlands	Inflow=8.24 cfs 42,343 cf Primary=8.24 cfs 42,343 cf
Link AP2: To Abutter	Inflow=5.66 cfs 26,912 cf Primary=5.66 cfs 26,912 cf
Link AP3: Abutter Depression	Inflow=2.04 cfs 3,641 cf Primary=2.04 cfs 3,641 cf
Total Runoff Area = 313,41	5 sf Runoff Volume = 76,281 cf Average Runoff Depth = 2.92" 97.20% Pervious = 304,629 sf 2.80% Impervious = 8,786 sf

Summary for Subcatchment 101S: Center Wetlands

Runoff = 8.24 cfs @ 12.36 hrs, Volume= 42,343 cf, Depth> 2.79" Routed to Link AP1 : To Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=7.42"

_	A	rea (sf)	CN E	Description		
		943	98 F	aved park	ing, HSG B	3
	1	02,339	61 >	75% Gras	s cover, Go	bod, HSG B
	78,735 55 Woods, Good, HSG B					
		409	<u>98</u> F	Roofs, HSG	ЪВ	
	1	82,426		Veighted A		
	1	81,074	ç	9.26% Per	vious Area	
		1,352	C	.74% Impe	ervious Are	а
	_					
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.7	50	0.0300	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.27"
	12.5	524	0.0100	0.70		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	7.2	215	0.0100	0.50		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	24.4	789	Total			

Summary for Subcatchment 102S: To Abutter

Runoff = 5.66 cfs @ 12.30 hrs, Volume= 26,912 Routed to Link AP2 : To Abutter

26,912 cf, Depth> 2.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=7.42"

 A	rea (sf)	CN E	Description		
	1,237	98 F	aved park	ing, HSG B	3
	83,409	61 >	75% Gras	s cover, Go	bod, HSG B
	20,785	55 V	Voods, Go	od, HSG B	
	2,406	<u>98</u> F	<u>Roofs, HSG</u>	ЪВ	
	07,837		Veighted A	•	
1	04,194	9	6.62% Per	vious Area	
	3,643	3	.38% Impe	ervious Are	а
_		<u>.</u>			
ŢĊ	Length	Slope	Velocity	Capacity	Description
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.3	50	0.0100	0.11		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
12.7	446	0.0070	0.59		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.7	34	0.0300	0.87		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
20.7	530	Total			

Summary for Subcatchment 103S: To Depression

Runoff = 2.22 cfs @ 12.09 hrs, Volume= 7,026 cf, Depth> 3.64" Routed to Pond EX : Existing Abutter Depression

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=7.42"

Are	ea (sf)	CN I	Description		
	1,600	98	Paved park	ing, HSG B	3
1	9,361	61 :	>75% Gras	s cover, Go	ood, HSG B
	2,191	98	Roofs, HSG	БВ	
2	3,152	67	Neighted A	verage	
1	9,361	8	33.63% Per	vious Area	3
	3,791		16.37% Imp	pervious Are	rea
_					
	Length	Slope		Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	23,152 sf,	, 16.37% Impervious,	Inflow Depth > 3.64" for 50-yr event						
Inflow =	2.22 cfs @	12.09 hrs, Volume=	7,026 cf						
Outflow =	2.16 cfs @	12.11 hrs, Volume=	6,960 cf, Atten= 3%, Lag= 1.2 mir	۱					
Discarded =	0.12 cfs @	12.11 hrs, Volume=	3,319 cf						
Primary =	2.04 cfs @	12.11 hrs, Volume=	3,641 cf						
Routed to Link AP3 : Abutter Depression									

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 250.36' @ 12.11 hrs Surf.Area= 1,981 sf Storage= 519 cf

Plug-Flow detention time= 25.6 min calculated for 6,960 cf (99% of inflow) Center-of-Mass det. time= 20.0 min (854.1 - 834.1)

Volume	Invert	Avail.Sto	rage	Storage D	escription		
#1	249.80'	1,12	22 cf	Custom S	tage Data (Pi	rismatic)Listed belov	v (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4 250.4	t) 60 60 60 60	rrf.Area (sq-ft) 164 562 1,261 2,144 3,127		Store <u>-feet)</u> 73 182 341 527	Cum.Store (cubic-feet) 0 73 255 595 1,122		
Device	Routing	Invert	Outle	et Devices	1,122		
#1	Discarded	249.80'				Surface area	Dhase in 0.04!
#2	Primary	250.25'	Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01' 20.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.63				

Discarded OutFlow Max=0.12 cfs @ 12.11 hrs HW=250.36' (Free Discharge) **1=Exfiltration** (Controls 0.12 cfs)

Primary OutFlow Max=1.98 cfs @ 12.11 hrs HW=250.36' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 1.98 cfs @ 0.89 fps)

Summary for Link AP1: To Wetlands

Inflow Area	a =	182,426 sf,	0.74% Impervious,	Inflow Depth > 2	2.79" for 50-	yr event
Inflow	=	8.24 cfs @ 1	12.36 hrs, Volume=	42,343 cf		
Primary	=	8.24 cfs @ ´	12.36 hrs, Volume=	42,343 cf,	Atten= 0%, L	_ag= 0.0 min

Summary for Link AP2: To Abutter

Inflow Area	a =	107,837 sf,	3.38% Impervious,	Inflow Depth >	2.99"	for 50-yr event
Inflow	=	5.66 cfs @ 1	12.30 hrs, Volume=	26,912 c	f	
Primary	=	5.66 cfs @ 1	12.30 hrs, Volume=	26,912 c	f, Atter	n= 0%, Lag= 0.0 min

Summary for Link AP3: Abutter Depression

Inflow Area	a =	23,152 sf, 16.37% Ir	npervious,	Inflow Depth = 1.89"	for 50-yr event
Inflow	=	2.04 cfs @ 12.11 hrs,	Volume=	3,641 cf	
Primary	=	2.04 cfs @ 12.11 hrs,	Volume=	3,641 cf, Atte	n= 0%, Lag= 0.0 min

19227 - PreDevelopment	Type III 24-h	r 100-yr Rainfall=8.86"							
Prepared by Howard Stein Hudson HydroCAD® 10.10-6a s/n M24582 © 2020 Hy	droCAD Software Solutions LLC	Printed 4/11/2022 Page <u>38</u>							
		Fage 30							
	0-24.00 hrs, dt=0.05 hrs, 481 points R-20 method, UH=SCS, Weighted-CN								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method									
Subcatchment 101S: Center Wetlands	Runoff Area=182,426 sf 0.74% Imper ow Length=789' Tc=24.4 min CN=59 I	•							
Subcatchment102S: To Abutter	Runoff Area=107,837 sf 3.38% Imper low Length=530' Tc=20.7 min CN=61	•							
Subcatchment103S: To Depression	Runoff Area=23,152 sf 16.37% Imper Tc=6.0 min CN=67	vious Runoff Depth>4.84" ′ Runoff=2.96 cfs 9,339 cf							
Pond EX: Existing Abutter Depression Discarded=0.13	Peak Elev=250.39' Storage=571 c cfs 3,822 cf Primary=2.76 cfs 5,425 cf								
Link AP1: To Wetlands		Inflow=11.59 cfs 58,515 cf							
	P	rimary=11.59 cfs 58,515 cf							
Link AP2: To Abutter		Inflow=7.84 cfs 36,798 cf							
		Primary=7.84 cfs 36,798 cf							
Link AP3: Abutter Depression		Inflow=2.76 cfs 5,425 cf							
-		Primary=2.76 cfs 5,425 cf							
	Runoff Volume = 104,652 cf Aver 97.20% Pervious = 304,629 sf 2.8	rage Runoff Depth = 4.01" 0% Impervious = 8,786 sf							

Summary for Subcatchment 101S: Center Wetlands

Runoff = 11.59 cfs @ 12.35 hrs, Volume= 58,515 cf, Depth> 3.85" Routed to Link AP1 : To Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-yr Rainfall=8.86"

A	rea (sf)	CN E	Description		
	943	98 F	aved park	ing, HSG B	3
1	02,339	61 >	75% Gras	s cover, Go	bod, HSG B
	78,735		,	od, HSG B	
	409	98 F	Roofs, HSG	ЪВ	
1	82,426	59 V	Veighted A	verage	
1	81,074	-		vious Area	
	1,352	0	.74% Impe	ervious Are	а
_		. .			
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.7	50	0.0300	0.18		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
12.5	524	0.0100	0.70		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
7.2	215	0.0100	0.50		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
24.4	789	Total			

Summary for Subcatchment 102S: To Abutter

Runoff = 7.84 cfs @ 12.30 hrs, Volume= 36,798 cf, Depth> 4.09" Routed to Link AP2 : To Abutter

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-yr Rainfall=8.86"

A	rea (sf)	CN E	Description		
	1,237	98 F	aved park	ing, HSG B	}
	83,409	61 >	75% Gras	s cover, Go	bod, HSG B
	20,785	55 V	Voods, Go	od, HSG B	
	2,406	98 F	Roofs, HSG	6 B	
1	07,837	61 V	Veighted A	verage	
1	04,194	9	6.62% Per	vious Area	
	3,643	3	.38% Impe	ervious Area	a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.3	50	0.0100	0.11		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
12.7	446	0.0070	0.59		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.7	34	0.0300	0.87		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
20.7	530	Total			

Summary for Subcatchment 103S: To Depression

Runoff 2.96 cfs @ 12.09 hrs, Volume= 9,339 cf, Depth> 4.84" = Routed to Pond EX : Existing Abutter Depression

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-yr Rainfall=8.86"

Are	ea (sf)	CN I	Description			
	1,600	98	Paved park	ing, HSG B	3	
1	9,361	61 :	>75% Gras	s cover, Go	ood, HSG B	
	2,191	98	Roofs, HSG	βB		
2	3,152	67 Weighted Average				
1	9,361	83.63% Pervious Area				
	3,791		16.37% Imp	pervious Are	rea	
_						
	Length	Slope		Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	23,152 sf, 16.37% Impervious,	Inflow Depth > 4.84" for 100-yr event						
Inflow =	2.96 cfs @ 12.09 hrs, Volume=	9,339 cf						
Outflow =	2.89 cfs @ 12.11 hrs, Volume=	9,247 cf, Atten= 3%, Lag= 1.1 min						
Discarded =	0.13 cfs @ 12.11 hrs, Volume=	3,822 cf						
Primary =	2.76 cfs @ 12.11 hrs, Volume=	5,425 cf						
Routed to Link AP3 : Abutter Depression								

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 250.39' @ 12.11 hrs Surf.Area= 2,093 sf Storage= 571 cf

Plug-Flow detention time= 22.9 min calculated for 9,227 cf (99% of inflow) Center-of-Mass det. time= 17.0 min (842.9 - 825.9)

Volume	Invert	Avail.Stor	rage Stora	ge Description			
#1	249.80'	1,12	22 cf Custo	om Stage Data (P	rismatic)Listed belov	v (Recalc)	
Elevatio (fee 249.8 250.0 250.2 250.4 250.6	90 90 20 40	urf.Area (sq-ft) 164 562 1,261 2,144 3,127	Inc.Store (cubic-feet) 0 73 182 341 527	Cum.Store (cubic-feet) 0 73 255 595 1,122			
Device #1	Routing Discarded	Invert 249.80'		Exfiltration over		Phase-In= 0.01'	
#2	Primary	250.25'	Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01' 20.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.63				

Discarded OutFlow Max=0.13 cfs @ 12.11 hrs HW=250.39' (Free Discharge) **1=Exfiltration** (Controls 0.13 cfs)

Primary OutFlow Max=2.69 cfs @ 12.11 hrs HW=250.39' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 2.69 cfs @ 0.99 fps)

Summary for Link AP1: To Wetlands

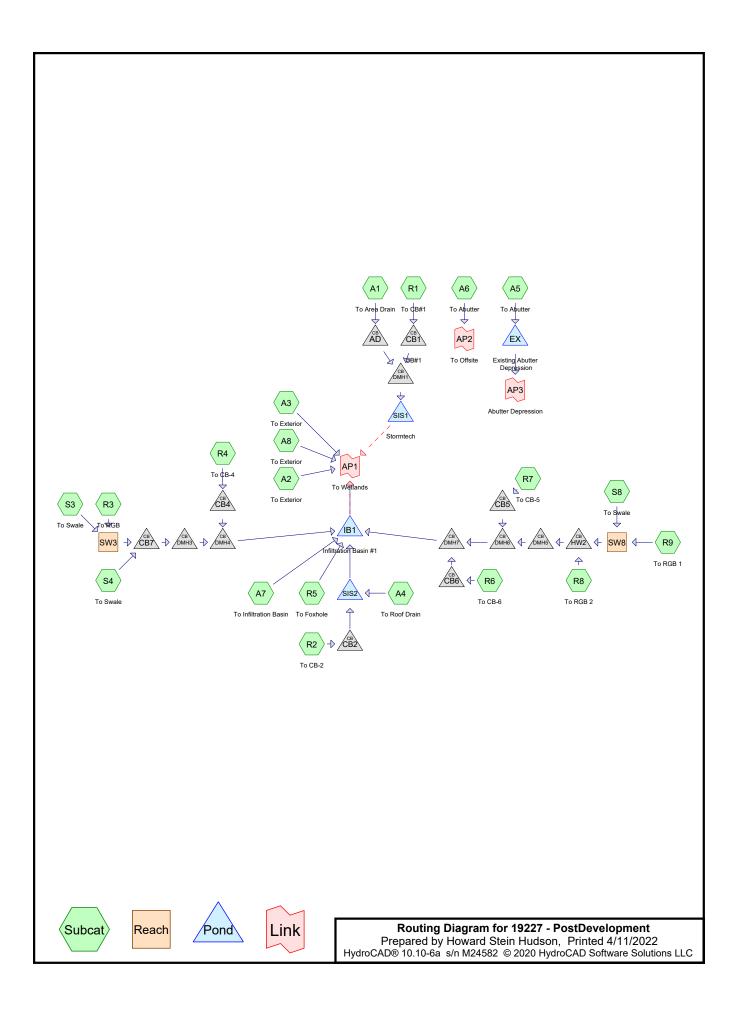
Inflow Are	a =	182,426 sf,	0.74% Impervious,	Inflow Depth >	3.85"	for 100-yr event
Inflow	=	11.59 cfs @ 1	12.35 hrs, Volume=	58,515 cf		
Primary	=	11.59 cfs @ _′	12.35 hrs, Volume=	58,515 cf	, Atter	n= 0%, Lag= 0.0 min

Summary for Link AP2: To Abutter

Inflow Area	a =	107,837 sf,	3.38% Impervious,	Inflow Depth > 4.09"	for 100-yr event
Inflow	=	7.84 cfs @ 1	12.30 hrs, Volume=	36,798 cf	
Primary	=	7.84 cfs @ 1	12.30 hrs, Volume=	36,798 cf, Atte	en= 0%, Lag= 0.0 min

Summary for Link AP3: Abutter Depression

Inflow Area =		23,152 sf, 16.37% Impervious	, Inflow Depth = 2.81" for 100-yr event
Inflow	=	2.76 cfs @ 12.11 hrs, Volume=	5,425 cf
Primary	=	2.76 cfs @ 12.11 hrs, Volume=	5,425 cf, Atten= 0%, Lag= 0.0 min



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Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Inch	Type III 24-hr		Default	24.00	1	2.00	2
2	2-yr	Type III 24-hr		Default	24.00	1	3.27	2
3	10-yr	Type III 24-hr		Default	24.00	1	4.92	2
4	50-yr	Type III 24-hr		Default	24.00	1	7.42	2
5	100-yr	Type III 24-hr		Default	24.00	1	8.86	2

Rainfall Events Listing

Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
182,276	61	>75% Grass cover, Good, HSG B (A1, A2, A3, A5, A6, A7, A8, R1, R2, R3, R4,
		R5, R6, R7, R8, R9, S3, S4, S8)
74,339	98	Paved parking, HSG B (A6, A8, R1, R2, R3, R4, R5, R6, R7, R8, R9, S3, S4, S8)
38,131	98	Roofs, HSG B (A3, A4, A5, A6, R2, R3, R4, R5, R6, R7, R8, R9)
18,669	55	Woods, Good, HSG B (A5, A6, A8)
313,415	74	TOTAL AREA

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
313,415	HSG B	A1, A2, A3, A4, A5, A6, A7, A8, R1, R2, R3, R4, R5, R6, R7, R8, R9, S3, S4, S8
0	HSG C	
0	HSG D	
0	Other	
313,415		TOTAL AREA

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HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Sub
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	Nun
0	182,276	0	0	0	182,276	>75% Grass	
						cover, Good	
0	74,339	0	0	0	74,339	Paved parking	
0	38,131	0	0	0	38,131	Roofs	
0	18,669	0	0	0	18,669	Woods, Good	
0	313,415	0	0	0	313,415	TOTAL AREA	

Ground Covers (all nodes)

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Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)
		. ,	· · /	· · /	, ,		· /	· /	<u>, </u>
1	AD	248.60	248.47	26.4	0.0049	0.011	0.0	8.0	0.0
2	CB1	249.00	248.53	93.3	0.0050	0.011	0.0	12.0	0.0
3	CB2	251.20	250.46	73.9	0.0100	0.011	0.0	12.0	0.0
4	CB4	247.83	247.55	14.5	0.0193	0.011	0.0	12.0	0.0
5	CB5	250.30	250.20	8.5	0.0118	0.011	0.0	12.0	0.0
6	CB6	248.30	248.26	6.3	0.0063	0.011	0.0	12.0	0.0
7	CB7	248.00	247.50	88.3	0.0057	0.011	0.0	18.0	0.0
8	DMH1	248.40	248.35	6.0	0.0083	0.010	0.0	12.0	0.0
9	DMH1	248.28	248.25	4.0	0.0075	0.010	0.0	24.0	0.0
10	DMH3	247.45	247.10	68.2	0.0051	0.011	0.0	18.0	0.0
11	DMH4	246.60	246.43	34.6	0.0049	0.011	0.0	24.0	0.0
12	DMH5	251.65	250.46	116.8	0.0102	0.011	0.0	15.0	0.0
13	DMH6	249.71	248.10	160.8	0.0100	0.011	0.0	18.0	0.0
14	DMH7	247.60	246.65	111.5	0.0085	0.011	0.0	24.0	0.0
15	HW2	253.00	251.75	14.6	0.0856	0.013	0.0	15.0	0.0
16	IB1	247.00	246.00	62.2	0.0161	0.012	0.0	18.0	0.0

Pipe Listing (all nodes)

Time span=2.00-24.00 hrs, dt=0.02 hrs, 1101 points x 3 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment A1: To Area Drain	Runoff Area=2,664 sf 0.00% Impervious Runoff Depth>0.07" Tc=6.0 min CN=61 Runoff=0.00 cfs 16 cf
Subcatchment A2: To Exterior	Runoff Area=1,761 sf 0.00% Impervious Runoff Depth>0.07" Tc=6.0 min CN=61 Runoff=0.00 cfs 11 cf
Subcatchment A3: To Exterior	Runoff Area=3,301 sf 50.05% Impervious Runoff Depth>0.56" Tc=6.0 min CN=80 Runoff=0.05 cfs 154 cf
Subcatchment A4: To Roof Drain	Runoff Area=2,082 sf 100.00% Impervious Runoff Depth>1.77" Tc=6.0 min CN=98 Runoff=0.09 cfs 308 cf
Subcatchment A5: To Abutter	Runoff Area=26,605 sf 5.66% Impervious Runoff Depth>0.10" Flow Length=212' Tc=11.8 min CN=63 Runoff=0.02 cfs 224 cf
Subcatchment A6: To Abutter	Runoff Area=36,823 sf 7.73% Impervious Runoff Depth>0.10" Tc=6.0 min CN=63 Runoff=0.02 cfs 311 cf
Subcatchment A7: To Infiltration Basin	Runoff Area=18,226 sf 0.00% Impervious Runoff Depth>0.07" Tc=6.0 min CN=61 Runoff=0.00 cfs 111 cf
Subcatchment A8: To Exterior	Runoff Area=30,601 sf 1.56% Impervious Runoff Depth>0.06" Tc=6.0 min CN=60 Runoff=0.01 cfs 154 cf
Subcatchment R1: To CB#1	Runoff Area=11,152 sf 70.59% Impervious Runoff Depth>0.90" Tc=6.0 min CN=87 Runoff=0.27 cfs 841 cf
Subcatchment R2: To CB-2	Runoff Area=22,269 sf 62.49% Impervious Runoff Depth>0.74" Tc=6.0 min CN=84 Runoff=0.43 cfs 1,379 cf
Subcatchment R3: To RGB	Runoff Area=10,217 sf 71.15% Impervious Runoff Depth>0.90" Tc=6.0 min CN=87 Runoff=0.25 cfs 770 cf
Subcatchment R4: To CB-4	Runoff Area=14,887 sf 66.43% Impervious Runoff Depth>0.85" Tc=6.0 min CN=86 Runoff=0.33 cfs 1,052 cf
Subcatchment R5: To Foxhole	Runoff Area=33,967 sf 52.12% Impervious Runoff Depth>0.56" Flow Length=327' Tc=10.1 min CN=80 Runoff=0.41 cfs 1,588 cf
Subcatchment R6: To CB-6	Runoff Area=26,016 sf 47.47% Impervious Runoff Depth>0.52" Flow Length=248' Tc=16.0 min CN=79 Runoff=0.24 cfs 1,128 cf
Subcatchment R7: To CB-5	Runoff Area=31,901 sf 39.74% Impervious Runoff Depth>0.41" Flow Length=303' Tc=11.9 min CN=76 Runoff=0.24 cfs 1,096 cf
Subcatchment R8: To RGB 2	Runoff Area=17,230 sf 63.94% Impervious Runoff Depth>0.79" Tc=6.0 min CN=85 Runoff=0.36 cfs 1,140 cf

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Subcatchment R9: To RGB 1	Runoff Area=7,051 sf 91.07% Impervious Runoff Depth>1.48 Tc=6.0 min CN=95 Runoff=0.27 cfs 870 c	
SubcatchmentS3: To Swale	Runoff Area=10,077 sf 25.91% Impervious Runoff Depth>0.27 Tc=6.0 min CN=71 Runoff=0.04 cfs 223 c	
SubcatchmentS4: To Swale	Runoff Area=1,346 sf 29.12% Impervious Runoff Depth>0.29 Tc=6.0 min CN=72 Runoff=0.01 cfs 33 c	
SubcatchmentS8: To Swale	Runoff Area=5,239 sf 34.13% Impervious Runoff Depth>0.35 Tc=6.0 min CN=74 Runoff=0.04 cfs 152 c	
Reach SW3:	Avg. Flow Depth=0.13' Max Vel=0.81 fps Inflow=0.29 cfs 993 on n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.20 cfs 981 of n=0.041 L=501.0' S=0.0100 cfs 981 of n=0.041 L=501.0' S=0.041 L=501.0' S=0.041 L=501.0' S=0.041 L=500 cfs 981 of n=0.041 L=500 cfs 980 of n=0.041 L	
Reach SW8:	Avg. Flow Depth=0.32' Max Vel=0.23 fps Inflow=0.31 cfs 1,023 c n=0.240 L=232.0' S=0.0102 '/' Capacity=4.93 cfs Outflow=0.18 cfs 1,009 c	
Pond AD:	Peak Elev=248.62' Inflow=0.00 cfs 16 o 8.0" Round Culvert n=0.011 L=26.4' S=0.0049 '/' Outflow=0.00 cfs 16 o	
Pond CB1: CB#1	Peak Elev=249.28' Inflow=0.27 cfs 841 of 12.0" Round Culvert n=0.011 L=93.3' S=0.0050 '/' Outflow=0.27 cfs 841 of	
Pond CB2:	Peak Elev=251.53' Inflow=0.43 cfs 1,379 c 12.0" Round Culvert n=0.011 L=73.9' S=0.0100 '/' Outflow=0.43 cfs 1,379 c	
Pond CB4:	Peak Elev=248.11' Inflow=0.33 cfs 1,052 c 12.0" Round Culvert n=0.011 L=14.5' S=0.0193 '/' Outflow=0.33 cfs 1,052 c	
Pond CB5:	Peak Elev=250.55' Inflow=0.24 cfs 1,096 c 12.0" Round Culvert n=0.011 L=8.5' S=0.0118 '/' Outflow=0.24 cfs 1,096 c	
Pond CB6:	Peak Elev=248.58' Inflow=0.24 cfs 1,128 c 12.0" Round Culvert n=0.011 L=6.3' S=0.0063 '/' Outflow=0.24 cfs 1,128 c	
Pond CB7:	Peak Elev=248.21' Inflow=0.21 cfs 1,014 c 18.0" Round Culvert n=0.011 L=88.3' S=0.0057 '/' Outflow=0.21 cfs 1,014 c	
Pond DMH1:	Peak Elev=248.50' Inflow=0.27 cfs 857 o Outflow=0.27 cfs 857 o	
Pond DMH3:	Peak Elev=247.67' Inflow=0.21 cfs 1,014 c 18.0" Round Culvert n=0.011 L=68.2' S=0.0051 '/' Outflow=0.21 cfs 1,014 c	
Pond DMH4:	Peak Elev=247.08' Inflow=0.50 cfs 2,066 c 24.0" Round Culvert n=0.011 L=34.6' S=0.0049 '/' Outflow=0.50 cfs 2,066 c	
Pond DMH5:	Peak Elev=251.98' Inflow=0.51 cfs 2,150 c 15.0" Round Culvert n=0.011 L=116.8' S=0.0102 '/' Outflow=0.51 cfs 2,150 c	
Pond DMH6:	Peak Elev=250.08' Inflow=0.70 cfs 3,245 c 18.0" Round Culvert n=0.011 L=160.8' S=0.0100 '/' Outflow=0.70 cfs 3,245 c	

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Pond DMH7:	Peak Elev=247.98' Inflow=0.88 cfs 4,373 cf 24.0" Round Culvert n=0.011 L=111.5' S=0.0085 '/' Outflow=0.88 cfs 4,373 cf
Pond EX: Existing Abutter De	PressionPeak Elev=249.82' Storage=5 cfInflow=0.02 cfs224 cfDiscarded=0.01 cfs224 cfPrimary=0.00 cfs0 cfOutflow=0.01 cfs224 cf
Pond HW2:	Peak Elev=253.33' Inflow=0.51 cfs 2,150 cf 15.0" Round Culvert n=0.013 L=14.6' S=0.0856 '/' Outflow=0.51 cfs 2,150 cf
Pond IB1: Infiltration Basin # Discarded=1.65 cfs 8,13	I Peak Elev=247.01' Storage=112 cf Inflow=1.77 cfs 8,137 cf 4 cf Primary=0.00 cfs 1 cf Secondary=0.00 cfs 0 cf Outflow=1.65 cfs 8,135 cf
Pond SIS1: Stormtech	Peak Elev=247.77' Storage=10 cf Inflow=0.27 cfs 857 cf Discarded=0.24 cfs 857 cf Secondary=0.00 cfs 0 cf Outflow=0.24 cfs 857 cf
Pond SIS2:	Peak Elev=249.10' Storage=72 cf Inflow=0.52 cfs 1,686 cf Discarded=0.35 cfs 1,686 cf Primary=0.00 cfs 0 cf Outflow=0.35 cfs 1,686 cf
Link AP1: To Wetlands	Inflow=0.05 cfs 320 cf Primary=0.05 cfs 320 cf
Link AP2: To Offsite	Inflow=0.02 cfs 311 cf Primary=0.02 cfs 311 cf
Link AP3: Abutter Depressio	n Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Total Runoff Area = 313,415 sf Runoff Volume = 11,560 cf Average Runoff Depth = 0.44" 64.11% Pervious = 200,945 sf 35.89% Impervious = 112,470 sf

Summary for Subcatchment A1: To Area Drain

Runoff = 0.00 cfs @ 12.50 hrs, Volume= 16 cf, Depth> 0.07" Routed to Pond AD :

A	rea (sf)	CN E	Description					
	2,664	61 >	51 >75% Grass cover, Good, HSG B					
	2,664	1	100.00% Pervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment A2: To Exterior

Runoff = 0.00 cfs @ 12.50 hrs, Volume= Routed to Link AP1 : To Wetlands 11 cf, Depth> 0.07"

A	rea (sf)	CN E	N Description					
	1,761	61 >	61 >75% Grass cover, Good, HSG B					
	1,761	1	100.00% Pervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment A3: To Exterior

Runoff = 0.05 cfs @ 12.10 hrs, Volume= Routed to Link AP1 : To Wetlands 154 cf, Depth> 0.56"

A	rea (sf)	CN	Description				
	1,649	61	>75% Gras	s cover, Go	ood, HSG B		
	1,652	98	Roofs, HSG	БВ			
	3,301	80	Neighted A	verage			
	1,649	4	49.95% Per	vious Area	3		
	1,652	:	50.05% Impervious Area				
-		~		o "			
Tc	Length	Slope	,	Capacity	1		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		
					-		

Summary for Subcatchment A4: To Roof Drain

Runoff = 0.09 cfs @ 12.08 hrs, Volume= 308 cf, Depth> 1.77" Routed to Pond SIS2 :

Area (sf)	CN	Description					
2,082	98	98 Roofs, HSG B					
2,082		100.00% Impervious Area					
Tc Length (min) (feet)	Slop (ft/fl		Capacity (cfs)	Description			
6.0				Direct Entry,			

Summary for Subcatchment A5: To Abutter

Runoff = 0.02 cfs @ 12.51 hrs, Volume= 224 cf, Depth> 0.10" Routed to Pond EX : Existing Abutter Depression

Α	rea (sf)	CN E	Description		
	23,897	61 >	75% Gras	s cover, Go	bod, HSG B
	1,201	55 V	Voods, Go	od, HSG B	
	1,507	98 F	Roofs, HSG	6 B	
	26,605	63 V	Veighted A	verage	
	25,098	g	4.34% Per	vious Area	
	1,507	5	.66% Impe	ervious Area	а
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.8	50	0.0120	0.12		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
5.0	162	0.0060	0.54		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
11.8	212	Total			

Summary for Subcatchment A6: To Abutter

Runoff = 0.02 cfs @ 12.42 hrs, Volume= Routed to Link AP2 : To Offsite 311 cf, Depth> 0.10"

Α	rea (sf)	CN	Description					
	1,281	98	Paved park	ing, HSG B	3			
	25,992	61	>75% Ġras	s cover, Go	ood, HSG B			
	7,984	55	Woods, Go	od, HSG B	8			
	1,566	98	Roofs, HSC	БВ				
	36,823	63	Weighted A	verage				
	33,976		92.27% Pei	vious Area	а			
	2,847		7.73% Impe	ervious Area	28			
_				. .				
Тс	Length	Slope	,	Capacity				
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
6.0					Direct Entry,			
0.0								

Summary for Subcatchment A7: To Infiltration Basin

Runoff = 0.00 cfs @ 12.50 hrs, Volume= 111 cf, Depth> 0.07" Routed to Pond IB1 : Infiltration Basin #1

Are	ea (sf)	CN E	Description						
1	8,226	61 >	>75% Grass cover, Good, HSG B						
1	8,226	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment A8: To Exterior

Runoff = 0.01 cfs @ 13.76 hrs, Volume= Routed to Link AP1 : To Wetlands 154 cf, Depth> 0.06"

A	rea (sf)	CN	Description					
	476	98	Paved park	ing, HSG B	3			
	20,641	61	>75% Gras	s cover, Go	ood, HSG B			
	9,484	55	Woods, Go	od, HSG B				
	30,601	60	Weighted A	verage				
	30,125		98.44% Per	vious Area	3			
	476		1.56% Impe	ervious Area	a			
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment R1: To CB#1

Runoff = 0.27 cfs @ 12.09 hrs, Volume= 841 cf, Depth> 0.90" Routed to Pond CB1 : CB#1

CN	Description		
98	Paved park	ing, HSG B	3
61	>75% Gras	s cover, Go	bod, HSG B
87	Weighted A	verage	
	29.41% Per	vious Area	l
	70.59% Imp	ervious Ar	ea
		Capacity (cfs)	Description
			Direct Entry,
	98 61 87 Slope	98 Paved park 61 >75% Grass 87 Weighted A 29.41% Per 70.59% Imp Slope Velocity	98 Paved parking, HSG E 61 >75% Grass cover, Go 87 Weighted Average 29.41% Pervious Area 70.59% Impervious Ar Slope Velocity Capacity

Summary for Subcatchment R2: To CB-2

Runoff = 0.43 cfs @ 12.09 hrs, Volume= 1,379 cf, Depth> 0.74" Routed to Pond CB2 :

Α	rea (sf)	CN	Description					
	8,993	98	Paved park	ing, HSG B	В			
	8,353	61	>75% Gras	s cover, Go	ood, HSG B			
	4,923	98	Roofs, HSC	БВ				
	22,269	84	Neighted A	verage				
	8,353	;	37.51% Pei	vious Area	а			
	13,916		62.49% Imp	pervious Ar	rea			
т.	1			0	Description			
TC	Length	Slope		Capacity	1			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R3: To RGB

Runoff = 0.25 cfs @ 12.09 hrs, Volume= 770 cf, Depth> 0.90" Routed to Reach SW3 :

Α	rea (sf)	CN	Description					
	5,386	98	Paved park	ing, HSG B	В			
	2,948	61	>75% Gras	s cover, Go	ood, HSG B			
	1,883	98	Roofs, HSC	B				
	10,217	87	Neighted A	verage				
	2,948		28.85% Pei	vious Area	а			
	7,269		71.15% Imp	pervious Ar	rea			
_								
ŢĊ	Length	Slope		Capacity	1			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R4: To CB-4

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 1,052 cf, Depth> 0.85" Routed to Pond CB4 :

A	rea (sf)	CN	Description					
	7,280	98	Paved park	ing, HSG B	3			
	4,998	61	>75% Gras	s cover, Go	ood, HSG B			
	2,609	98	Roofs, HSO	βB				
	14,887	86	Weighted A	verage				
	4,998	:	33.57% Pei	vious Area	a			
	9,889	(66.43% Imp	pervious Ar	rea			
т.	1	0		0				
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R5: To Foxhole

Runoff = 0.41 cfs @ 12.16 hrs, Volume= 1,58 Routed to Pond IB1 : Infiltration Basin #1

1,588 cf, Depth> 0.56"

Α	vrea (sf)	CN [Description						
	11,765	98 F	B Paved parking, HSG B						
	16,262	61 >	>75% Ġras	s cover, Go	bod, HSG B				
	5,940	98 F	Roofs, HSC	βB					
	33,967	80 \	Neighted A	verage					
	16,262	2	17.88% Pei	vious Area					
	17,705	Ę	52.12% Imp	pervious Ar	ea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.6	50	0.0200	0.15		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.27"				
0.5	68	0.0150	2.49		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
3.9	187	0.0130	0.80		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.1	22	0.0300	3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
10.1	327	Total							

Summary for Subcatchment R6: To CB-6

Runoff = 0.24 cfs @ 12.25 hrs, Volume= 1,128 cf, Depth> 0.52" Routed to Pond CB6 :

	A	rea (sf)	CN E	N Description						
		7,642	98 F	aved park	ing, HSG B	3				
		13,667	61 >	75% Gras	s cover, Go	bod, HSG B				
		4,707	<u>98</u> F	<u>Roofs, HSG</u>	Ъ В					
		26,016		Veighted A						
		13,667	5	2.53% Per	vious Area					
		12,349	4	7.47% Imp	pervious Are	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	11.9	50	0.0030	0.07		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.27"				
	3.8	158	0.0100	0.70		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	0.3	40	0.0130	2.31		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	16.0	248	Total							

Summary for Subcatchment R7: To CB-5

Runoff = 0.24 cfs @ 12.19 hrs, Volume= 1,096 cf, Depth> 0.41" Routed to Pond CB5 :

	A	rea (sf)	CN [CN Description						
		7,161	98 F	98 Paved parking, HSG B						
		19,224	61 >	•75% Ġras	s cover, Go	bod, HSG B				
		5,516	98 F	Roofs, HSC	βB					
		31,901	76 V	Veighted A	verage					
		19,224	6	0.26% Pei	vious Area					
		12,677	3	89.74% Imp	pervious Ar	ea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.1	50	0.0160	0.14		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.27"				
	5.7	226	0.0090	0.66		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	0.1	27	0.0500	4.54		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	11.9	303	Total							

Summary for Subcatchment R8: To RGB 2

Runoff = 0.36 cfs @ 12.09 hrs, Volume= 1,140 cf, Depth> 0.79" Routed to Pond HW2 :

A	rea (sf)	CN	Description					
	6,729	98	Paved park	ing, HSG B				
	6,213	61	>75% Gras	s cover, Go	od, HSG B			
	4,288	98	Roofs, HSG	6 B				
	17,230	85	Weighted A	verage				
	6,213		36.06% Per	vious Area				
	11,017	(63.94% Imp	pervious Ar	a			
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment R9: To RGB 1

Runoff = 0.27 cfs @ 12.09 hrs, Volume= 870 cf, Depth> 1.48" Routed to Reach SW8 :

Α	rea (sf)	CN	Description					
	4,963	98	Paved park	ing, HSG E	В			
	630	61	>75% Gras	s cover, Go	lood, HSG B			
	1,458	98	Roofs, HSC	βB				
	7,051	95	Weighted A	verage				
	630		8.93% Perv	vious Area				
	6,421		91.07% Imp	pervious Ar	rea			
_								
ŢĊ	Length	Slope	,	Capacity	· · · · · · · · · · · · · · · · · · ·			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment S3: To Swale

Runoff = 0.04 cfs @ 12.13 hrs, Volume= 223 cf, Depth> 0.27" Routed to Reach SW3 :

A	rea (sf)	CN I	Description						
	2,611	98	Paved parking, HSG B						
	7,466	61 :	>75% Grass cover, Good, HSG B						
	10,077	71	1 Weighted Average						
	7,466	-	74.09% Pervious Area						
	2,611	25.91% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment S4: To Swale

Runoff = 0.01 cfs @ 12.12 hrs, Volume= 33 cf, Depth> 0.29" Routed to Pond CB7 :

Α	rea (sf)	CN	Description						
	392	98	Paved parking, HSG B						
	954	61	>75% Grass cover, Good, HSG B						
	1,346	72	Weighted Average						
	954		70.88% Pervious Area						
	392		29.12% Impervious Area						
-		01		0					
Tc	Length	Slope		Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment S8: To Swale

Runoff = 0.04 cfs @ 12.11 hrs, Volume= 152 cf, Depth> 0.35" Routed to Reach SW8 :

Α	rea (sf)	CN I	Description						
	1,788	98 I	Paved parking, HSG B						
	3,451	61 >	>75% Grass cover, Good, HSG B						
	5,239	74 \	Weighted Average						
	3,451	6	65.87% Pervious Area						
	1,788	(34.13% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)					
6.0					Direct Entry,				

Summary for Reach SW3:

20,294 sf, 48.68% Impervious, Inflow Depth > 0.59" for 2-Inch event Inflow Area = 0.29 cfs @ 12.10 hrs. Volume= Inflow = 993 cf 0.20 cfs @ 12.19 hrs, Volume= Outflow = 981 cf, Atten= 30%, Lag= 5.4 min Routed to Pond CB7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 0.81 fps, Min. Travel Time= 10.3 min Avg. Velocity = 0.32 fps, Avg. Travel Time= 26.5 min Peak Storage= 124 cf @ 12.19 hrs Average Depth at Peak Storage= 0.13', Surface Width= 2.28' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 28.59 cfs 1.50' x 1.50' deep channel, n= 0.041 Riprap, 2-inch Side Slope Z-value = 3.0 '/' Top Width = 10.50' Length= 501.0' Slope= 0.0100 '/' Inlet Invert= 256.12', Outlet Invert= 251.10'

Summary for Reach SW8:

12,290 sf, 66.79% Impervious, Inflow Depth > 1.00" for 2-Inch event Inflow Area = Inflow = 0.31 cfs @ 12.09 hrs, Volume= 1.023 cf 0.18 cfs @ 12.20 hrs, Volume= Outflow = 1,009 cf, Atten= 41%, Lag= 7.0 min Routed to Pond HW2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 0.23 fps, Min. Travel Time= 16.8 min Avg. Velocity = 0.09 fps, Avg. Travel Time= 44.6 min Peak Storage= 183 cf @ 12.20 hrs Average Depth at Peak Storage= 0.32', Surface Width= 3.42' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 4.93 cfs 1.50' x 1.50' deep channel, n= 0.240 Sheet flow over Dense Grass Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 232.0' Slope= 0.0102 '/' Inlet Invert= 255.37', Outlet Invert= 253.00'

Summary for Pond AD:

Inflow Area = 2,664 sf, 0.00% Impervious, Inflow Depth > 0.07" for 2-Inch event 0.00 cfs @ 12.50 hrs, Volume= Inflow = 16 cf 0.00 cfs @ 12.50 hrs, Volume= Outflow = 16 cf, Atten= 0%, Lag= 0.0 min 0.00 cfs @ 12.50 hrs, Volume= Primary = 16 cf Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.62' @ 12.50 hrs Flood Elev= 250.75' Device Routing Invert Outlet Devices #1 Primary 248.60' 8.0" Round Culvert L= 26.4' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.60' / 248.47' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.35 sf Primary OutFlow Max=0.00 cfs @ 12.50 hrs HW=248.62' TW=248.40' (Dynamic Tailwater)

1=Culvert (Barrel Controls 0.00 cfs @ 0.41 fps)

Summary for Pond CB1: CB#1

Inflow Area = 11,152 sf, 70.59% Impervious, Inflow Depth > 0.90" for 2-Inch event Inflow = 0.27 cfs @ 12.09 hrs, Volume= 841 cf 0.27 cfs @ 12.09 hrs, Volume= Outflow = 841 cf, Atten= 0%, Lag= 0.0 min 0.27 cfs @ 12.09 hrs, Volume= Primary = 841 cf Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.28' @ 12.09 hrs Flood Elev= 251.50' Device Routing Invert Outlet Devices #1 249.00' Primary 12.0" Round Culvert L= 93.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.00' / 248.53' S= 0.0050 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=0.27 cfs @ 12.09 hrs HW=249.28' TW=248.50' (Dynamic Tailwater)

1=Culvert (Barrel Controls 0.27 cfs @ 2.26 fps)

Summary for Pond CB2:

Inflow Area = 22,269 sf, 62.49% Impervious, Inflow Depth > 0.74" for 2-Inch event Inflow = 0.43 cfs @ 12.09 hrs, Volume= 1.379 cf 0.43 cfs @ 12.09 hrs, Volume= Outflow = 1,379 cf, Atten= 0%, Lag= 0.0 min 0.43 cfs @ 12.09 hrs, Volume= Primary = 1,379 cf Routed to Pond SIS2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.53' @ 12.09 hrs Flood Elev= 254.02' Device Routing Invert Outlet Devices #1 251.20' Primary 12.0" Round Culvert L= 73.9' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.20' / 250.46' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.43 cfs @ 12.09 hrs HW=251.52' TW=249.06' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.43 cfs @ 1.94 fps)

Summary for Pond CB4:

Inflow Area = 14,887 sf, 66.43% Impervious, Inflow Depth > 0.85" for 2-Inch event Inflow = 0.33 cfs @ 12.09 hrs, Volume= 1.052 cf 0.33 cfs @ 12.09 hrs, Volume= Outflow = 1,052 cf, Atten= 0%, Lag= 0.0 min 0.33 cfs @ 12.09 hrs, Volume= Primary = 1,052 cf Routed to Pond DMH4 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.11' @ 12.09 hrs Flood Elev= 250.69' Device Routing Invert Outlet Devices #1 247.83' Primary 12.0" Round Culvert L= 14.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.83' / 247.55' S= 0.0193 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=0.33 cfs @ 12.09 hrs HW=248.11' TW=247.07' (Dynamic Tailwater)

1=Culvert (Inlet Controls 0.33 cfs @ 1.81 fps)

Summary for Pond CB5:

Inflow Area = 31,901 sf, 39.74% Impervious, Inflow Depth > 0.41" for 2-Inch event Inflow = 0.24 cfs @ 12.19 hrs, Volume= 1.096 cf 0.24 cfs @ 12.19 hrs, Volume= Outflow = 1,096 cf, Atten= 0%, Lag= 0.0 min 0.24 cfs @ 12.19 hrs, Volume= Primary = 1,096 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.55' @ 12.19 hrs Flood Elev= 252.45' Device Routing Invert Outlet Devices #1 250.30' Primary 12.0" Round Culvert L= 8.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 250.30' / 250.20' S= 0.0118 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.24 cfs @ 12.19 hrs HW=250.55' TW=250.07' (Dynamic Tailwater) -1=Culvert (Barrel Controls 0.24 cfs @ 2.27 fps)

Summary for Pond CB6:

Inflow Area = 26,016 sf, 47.47% Impervious, Inflow Depth > 0.52" for 2-Inch event 0.24 cfs @ 12.25 hrs, Volume= Inflow = 1.128 cf 0.24 cfs @ 12.25 hrs, Volume= Outflow = 1,128 cf, Atten= 0%, Lag= 0.0 min 0.24 cfs @ 12.25 hrs, Volume= Primary = 1,128 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.58' @ 12.25 hrs Flood Elev= 250.82' Device Routing Invert Outlet Devices #1 Primary 248.30' 12.0" Round Culvert L= 6.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.30' / 248.26' S= 0.0063 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.24 cfs @ 12.25 hrs HW=248.58' TW=247.97' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.24 cfs @ 1.96 fps)

Summary for Pond CB7:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 0.56" for 2-Inch event 0.21 cfs @ 12.19 hrs. Volume= Inflow = 1.014 cf 0.21 cfs @ 12.19 hrs, Volume= Outflow = 1,014 cf, Atten= 0%, Lag= 0.0 min 0.21 cfs @ 12.19 hrs, Volume= Primary = 1,014 cf Routed to Pond DMH3 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.21' @ 12.19 hrs Flood Elev= 253.00' Device Routing **Outlet Devices** Invert #1 248.00' Primary 18.0" Round Culvert L= 88.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.00' / 247.50' S= 0.0057 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=0.21 cfs @ 12.19 hrs HW=248.21' TW=247.67' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.21 cfs @ 2.07 fps)

Summary for Pond DMH1:

Outflow Primary	= = =	0.27 cfs @ 12 0.27 cfs @ 12	6.98% Impervious, Inflow Depth > 0.74" for 2-Inch event 2.09 hrs, Volume= 857 cf 2.09 hrs, Volume= 857 cf, Atten= 0%, Lag= 0.0 min 2.09 hrs, Volume= 857 cf ch				
Peak Ĕl Flood E	Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.50' @ 12.09 hrs Flood Elev= 252.50'						
Device	Routing	Invert	Outlet Devices				
#1 #2	Primary Primary		12.0" Round MANIFOLD L= 6.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.40' / 248.35' S= 0.0083 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.79 sf 24.0" Round ISOLATOR L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.28' / 248.25' S= 0.0075 '/' Cc= 0.900				
Duine eur		Max-0.07 afa	n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf				

Primary OutFlow Max=0.27 cfs @ 12.09 hrs HW=248.50' TW=247.77' (Dynamic Tailwater) **1=MANIFOLD** (Barrel Controls 0.04 cfs @ 1.41 fps)

2=ISOLATOR (Barrel Controls 0.23 cfs @ 1.79 fps)

Summary for Pond DMH3:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 0.56" for 2-Inch event 0.21 cfs @ 12.19 hrs. Volume= Inflow = 1.014 cf 0.21 cfs @ 12.19 hrs, Volume= Outflow = 1,014 cf, Atten= 0%, Lag= 0.0 min 0.21 cfs @ 12.19 hrs, Volume= Primary = 1,014 cf Routed to Pond DMH4 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.67' @ 12.19 hrs Flood Elev= 251.98' Device Routing **Outlet Devices** Invert #1 247.45' Primary 18.0" Round Culvert L= 68.2' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.45' / 247.10' S= 0.0051 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=0.21 cfs @ 12.19 hrs HW=247.67' TW=247.07' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.21 cfs @ 2.00 fps)

Summary for Pond DMH4:

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=310)

 Inflow Area =
 36,527 sf, 55.19% Impervious, Inflow Depth > 0.68" for 2-Inch event

 Inflow =
 0.50 cfs @ 12.11 hrs, Volume=
 2,066 cf

 Outflow =
 0.50 cfs @ 12.11 hrs, Volume=
 2,066 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.50 cfs @ 12.11 hrs, Volume=
 2,066 cf

 Routed to Pond IB1 : Infiltration Basin #1
 2,066 cf

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.08' @ 12.11 hrs Flood Elev= 251.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	246.60'	24.0" Round Culvert
	-		L= 34.6' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 246.60' / 246.43' S= 0.0049 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf
Primary	/ OutFlow	Max=0.50 cfs @	② 12.11 hrs HW=247.08' TW=247.01' (Dynamic Tailwater)

1=Culvert (Outlet Controls 0.50 cfs @ 1.31 fps)

Summary for Pond DMH5:

Inflow Area = 29,520 sf, 65.13% Impervious, Inflow Depth > 0.87" for 2-Inch event Inflow = 0.51 cfs @ 12.11 hrs, Volume= 2.150 cf 0.51 cfs @ 12.11 hrs, Volume= Outflow = 2,150 cf, Atten= 0%, Lag= 0.0 min 0.51 cfs @ 12.11 hrs, Volume= Primary = 2,150 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.98' @ 12.11 hrs Flood Elev= 254.00' Device Routing Invert Outlet Devices #1 251.65' Primary 15.0" Round Culvert L= 116.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.65' / 250.46' S= 0.0102 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.23 sf

Primary OutFlow Max=0.50 cfs @ 12.11 hrs HW=251.98' TW=250.07' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.50 cfs @ 1.95 fps)

Summary for Pond DMH6:

Inflow Area = 61,421 sf, 51.94% Impervious, Inflow Depth > 0.63" for 2-Inch event Inflow = 0.70 cfs @ 12.13 hrs, Volume= 3.245 cf 0.70 cfs @ 12.13 hrs, Volume= Outflow = 3,245 cf, Atten= 0%, Lag= 0.0 min 0.70 cfs @ 12.13 hrs, Volume= Primary = 3,245 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.08' @ 12.13 hrs Flood Elev= 252.93' Device Routing Invert Outlet Devices #1 249.71' Primary 18.0" Round Culvert L= 160.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.71' / 248.10' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=0.69 cfs @ 12.13 hrs HW=250.08' TW=247.98' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.69 cfs @ 2.07 fps)

Summary for Pond DMH7:

Inflow Area = 87,437 sf, 50.61% Impervious, Inflow Depth > 0.60" for 2-Inch event Inflow = 0.88 cfs @ 12.17 hrs, Volume= 4.373 cf 0.88 cfs @ 12.17 hrs, Volume= Outflow = 4,373 cf, Atten= 0%, Lag= 0.0 min 0.88 cfs @ 12.17 hrs, Volume= Primary = 4,373 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.98' @ 12.17 hrs Flood Elev= 251.25' Device Routing Invert Outlet Devices #1 247.60' Primary 24.0" Round Culvert L= 111.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.60' / 246.65' S= 0.0085 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf Primary OutFlow Max=0.88 cfs @ 12.17 hrs HW=247.98' TW=247.01' (Dynamic Tailwater)

1=Culvert (Inlet Controls 0.88 cfs @ 2.11 fps)

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	26,605 sf, 5.66% Im	pervious, Inflow Depth	> 0.10" for 2-Inch event				
Inflow =	0.02 cfs @ 12.51 hrs,	Volume= 22	4 cf				
Outflow =	0.01 cfs @ 12.70 hrs,	Volume= 22	4 cf, Atten= 23%, Lag= 11.2 min				
Discarded =	0.01 cfs @ 12.70 hrs,	Volume= 22	4 cf				
Primary =	0.00 cfs @ 2.00 hrs,	Volume=	0 cf				
Routed to Link AP3 : Abutter Depression							

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.82' @ 12.70 hrs Surf.Area= 213 sf Storage= 5 cf

Plug-Flow detention time= 3.3 min calculated for 223 cf (100% of inflow) Center-of-Mass det. time= 2.4 min (994.7 - 992.2)

Volume	Invert	Avail.Sto	rage 3	Storage D	escription		
#1	249.80'	1,12	22 cf	Custom S	tage Data (Pr	rismatic)Listed below	/ (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4 250.6	t) 30 90 20 40	urf.Area (sq-ft) 164 562 1,261 2,144 3,127	Inc.§ (cubic-	Store <u>feet)</u> 73 182 341 527	Cum.Store (cubic-feet) 0 73 255 595 1,122		
Device	Routing	Invert	Outlet	Devices	1,122		
#1	Discarded	249.80'			Itration over		
#2	Primary	250.25'	20.0' I Head	long x 15 (feet) 0.2	.0' breadth B 0 0.40 0.60	Elevation = 247.00' road-Crested Recta 0.80 1.00 1.20 1.40 70 2.64 2.63 2.64	ngular Weir)1.60

Discarded OutFlow Max=0.01 cfs @ 12.70 hrs HW=249.82' (Free Discharge) **1=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 2.00 hrs HW=249.80' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond HW2:

[62] Hint: Exceeded Reach SW8 OUTLET depth by 0.06' @ 12.06 hrs

 Inflow Area =
 29,520 sf, 65.13% Impervious, Inflow Depth > 0.87" for 2-Inch event

 Inflow =
 0.51 cfs @
 12.11 hrs, Volume=
 2,150 cf

 Outflow =
 0.51 cfs @
 12.11 hrs, Volume=
 2,150 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.51 cfs @
 12.11 hrs, Volume=
 2,150 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.51 cfs @
 12.11 hrs, Volume=
 2,150 cf

 Routed to Pond DMH5 :
 0.51 cfs
 0.51 cfs

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 253.33' @ 12.11 hrs Flood Elev= 254.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	253.00'	15.0" Round Culvert L= 14.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 253.00' / 251.75' S= 0.0856 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
		M 0 50 6 6	

Primary OutFlow Max=0.50 cfs @ 12.11 hrs HW=253.33' TW=251.98' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.50 cfs @ 1.95 fps)

Summary for Pond IB1: Infiltration Basin #1

[80] Warning: Exceeded Pond DMH4 by 0.31' @ 2.00 hrs (0.76 cfs 2,649 cf)

200,508 sf, 48.93% Impervious, Inflow Depth > 0.49" for 2-Inch event Inflow Area = Inflow 1.77 cfs @ 12.14 hrs, Volume= 8,137 cf = 1.65 cfs @ 12.21 hrs, Volume= Outflow = 8,135 cf, Atten= 7%, Lag= 3.8 min 1.65 cfs @ 12.21 hrs, Volume= Discarded = 8,134 cf 0.00 cfs @ 12.21 hrs, Volume= Primary = 1 cf Routed to Link AP1 : To Wetlands 2.00 hrs, Volume= Secondary = 0.00 cfs @ 0 cf Routed to Link AP1 : To Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.01' @ 12.21 hrs Surf.Area= 8,551 sf Storage= 112 cf

Plug-Flow detention time= 0.9 min calculated for 8,135 cf (100% of inflow) Center-of-Mass det. time= 0.7 min (867.2 - 866.5)

Volume	Invert	Avail.Sto	orage	Storage Description		
#1	247.00'	32,7	758 cf	Custom Stage Data	a (Irregular)Listed	below (Recalc)
Elevatio (fee		f.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
247.0	1		373.0	0	0	8,532
248.0	-	,	398.2	9,293	9,293	10,126
249.0		,	423.3	10,886	20,179	11,817
250.0)0 [~]	13,461	448.4	12,579	32,758	13,612
Device	Routing	Invert	-	et Devices		
#1	Discarded	247.00'	-	0 in/hr Exfiltration o		
#2	Primary	247.00'	,			
#3	Device 2	247.00'		-		to weir flow at low heads
#4	Device 2	248.20'	-	x 2.0" Horiz. Orifice		
				rows C= 0.600 in 24.0		l% open area)
#5	Secondary	249.00'	10.0 Head	ted to weir flow at low ' long x 10.0' breadt d (feet) 0.20 0.40 0. f. (English) 2.49 2.56	h Broad-Crested 60 0.80 1.00 1.2	0 1.40 1.60

Discarded OutFlow Max=1.65 cfs @ 12.21 hrs HW=247.01' (Free Discharge) **1=Exfiltration** (Controls 1.65 cfs)

Primary OutFlow Max=0.00 cfs @ 12.21 hrs HW=247.01' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 0.00 cfs of 0.00 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.39 fps) -4=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.00' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond SIS1: Stormtech

Inflow Area = 13,816 sf, 56.98% Impervious, Inflow Depth > 0.74" for 2-Inch event Inflow = 0.27 cfs @ 12.09 hrs, Volume= 857 cf 0.24 cfs @ 12.13 hrs, Volume= Outflow = 857 cf, Atten= 11%, Lag= 2.5 min 0.24 cfs @ 12.13 hrs, Volume= Discarded = 857 cf Secondary = 0.00 cfs @ 2.00 hrs, Volume= 0 cf Routed to Link AP1 : To Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.77' @ 12.13 hrs Surf.Area= 1,242 sf Storage= 10 cf

Plug-Flow detention time= 0.4 min calculated for 856 cf (100% of inflow) Center-of-Mass det. time= 0.3 min (843.4 - 843.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	247.75'	1,151 cf	20.50'W x 60.58'L x 3.50'H Field A
			4,346 cf Overall - 1,470 cf Embedded = 2,876 cf x 40.0% Voids
#2A	248.25'	1,470 cf	ADS_StormTech SC-740 +Cap x 32 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			32 Chambers in 4 Rows
#3	248.00'	35 cf	4.00'D x 2.75'H Vertical Cone/Cylinder-Impervious
		2,655 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	247.75'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 245.51' Phase-In= 0.01'
#2	Secondary	250.60'	2.0" x 2.0" Horiz. Orifice/Grate X 7.00 columns
			X 7 rows C= 0.600 in 24.0" x 24.0" Grate (34% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.24 cfs @ 12.13 hrs HW=247.77' (Free Discharge) **1=Exfiltration** (Controls 0.24 cfs)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.75' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Pond SIS1: Stormtech - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 58.58' Row Length +12.0" End Stone x 2 = 60.58' Base Length 4 Rows x 51.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.50' Base Width

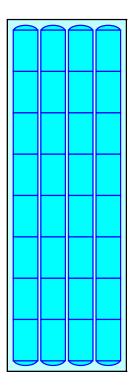
6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

32 Chambers x 45.9 cf = 1,470.1 cf Chamber Storage

4,346.4 cf Field - 1,470.1 cf Chambers = 2,876.3 cf Stone x 40.0% Voids = 1,150.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,620.6 cf = 0.060 afOverall Storage Efficiency = 60.3%Overall System Size = $60.58' \times 20.50' \times 3.50'$

32 Chambers 161.0 cy Field 106.5 cy Stone





Summary for Pond SIS2:

Inflow Area =	24,351 sf.	, 65.70% Impervious,	Inflow Depth > 0.83"	for 2-Inch event			
Inflow =	0.52 cfs @	12.09 hrs, Volume=	1,686 cf				
Outflow =	0.35 cfs @	12.19 hrs, Volume=	1,686 cf, Atter	n= 33%, Lag= 5.6 min			
Discarded =	0.35 cfs @	12.19 hrs, Volume=	1,686 cf				
Primary =		2.00 hrs, Volume=	0 cf				
Routed to Pond	Routed to Pond IB1 : Infiltration Basin #1						
Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.10' @ 12.19 hrs Surf.Area= 1,735 sf Storage= 72 cf							

Plug-Flow detention time= 0.9 min calculated for 1,685 cf (100% of inflow) Center-of-Mass det. time= 0.8 min (837.9 - 837.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	249.00'	1,603 cf	44.25'W x 39.22'L x 3.50'H Field A
			6,074 cf Overall - 2,067 cf Embedded = 4,006 cf x 40.0% Voids
#2A	249.50'	2,067 cf	ADS_StormTech SC-740 +Cap x 45 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			45 Chambers in 9 Rows
#3	250.80'	38 cf	4.00'D x 3.00'H Vertical Cone/Cylinder
		3,708 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	249.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01'
#2	Primary	253.50'	2.0" x 2.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 24.0" x 24.0" Grate (25% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.35 cfs @ 12.19 hrs HW=249.10' (Free Discharge) **1=Exfiltration** (Controls 0.35 cfs)

Primary OutFlow Max=0.00 cfs @ 2.00 hrs HW=249.00' TW=247.00' (Dynamic Tailwater) →2=Orifice/Grate (Controls 0.00 cfs)

Pond SIS2: - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

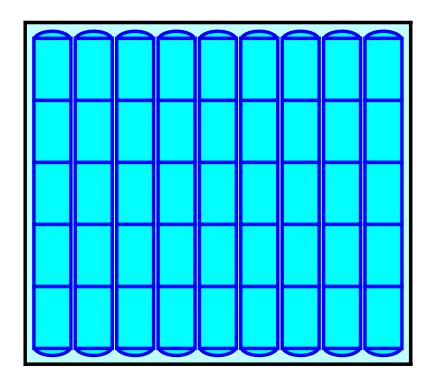
5 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 37.22' Row Length +12.0" End Stone x 2 = 39.22' Base Length 9 Rows x 51.0" Wide + 6.0" Spacing x 8 + 12.0" Side Stone x 2 = 44.25' Base Width 6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

45 Chambers x 45.9 cf = 2,067.3 cf Chamber Storage

6,073.7 cf Field - 2,067.3 cf Chambers = 4,006.4 cf Stone x 40.0% Voids = 1,602.6 cf Stone Storage

Chamber Storage + Stone Storage = 3,669.9 cf = 0.084 afOverall Storage Efficiency = 60.4%Overall System Size = $39.22' \times 44.25' \times 3.50'$

45 Chambers 225.0 cy Field 148.4 cy Stone





Summary for Link AP1: To Wetlands

Inflow Area	a =	236,171 sf, 4	42.45% Impervious,	Inflow Depth >	0.02"	for 2-Inch event
Inflow	=	0.05 cfs @ 12	2.10 hrs, Volume=	320 c	f	
Primary	=	0.05 cfs @ 12	2.10 hrs, Volume=	320 c	f, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP2: To Offsite

Inflow Are	a =	36,823 sf,	7.73% Impervious,	Inflow Depth > 0.10"	for 2-Inch event
Inflow	=	0.02 cfs @ 1	12.42 hrs, Volume=	311 cf	
Primary	=	0.02 cfs @ ´	12.42 hrs, Volume=	311 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP3: Abutter Depression

Inflow Are	a =	26,605 sf,	5.66% Impervious,	Inflow Depth = 0.00"	for 2-Inch event
Inflow	=	0.00 cfs @	2.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	2.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Time span=2.00-24.00 hrs, dt=0.02 hrs, 1101 points x 3 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method Page 56

Subcatchment A1: To Area Drain	Runoff Area=2,664 sf 0.00% Impervious Runoff Depth>0.47" Tc=6.0 min CN=61 Runoff=0.02 cfs 105 cf
Subcatchment A2: To Exterior	Runoff Area=1,761 sf 0.00% Impervious Runoff Depth>0.47" Tc=6.0 min CN=61 Runoff=0.01 cfs 69 cf
SubcatchmentA3: To Exterior	Runoff Area=3,301 sf 50.05% Impervious Runoff Depth>1.45" Tc=6.0 min CN=80 Runoff=0.13 cfs 400 cf
Subcatchment A4: To Roof Drain	Runoff Area=2,082 sf 100.00% Impervious Runoff Depth>3.03" Tc=6.0 min CN=98 Runoff=0.15 cfs 526 cf
SubcatchmentA5: To Abutter	Runoff Area=26,605 sf 5.66% Impervious Runoff Depth>0.55" Flow Length=212' Tc=11.8 min CN=63 Runoff=0.23 cfs 1,217 cf
Subcatchment A6: To Abutter	Runoff Area=36,823 sf 7.73% Impervious Runoff Depth>0.55" Tc=6.0 min CN=63 Runoff=0.40 cfs 1,688 cf
Subcatchment A7: To Infiltration Basin	Runoff Area=18,226 sf 0.00% Impervious Runoff Depth>0.47" Tc=6.0 min CN=61 Runoff=0.15 cfs 717 cf
Subcatchment A8: To Exterior	Runoff Area=30,601 sf 1.56% Impervious Runoff Depth>0.44" Tc=6.0 min CN=60 Runoff=0.22 cfs 1,109 cf
Subcatchment R1: To CB#1	Runoff Area=11,152 sf 70.59% Impervious Runoff Depth>1.97" Tc=6.0 min CN=87 Runoff=0.59 cfs 1,835 cf
Subcatchment R2: To CB-2	Runoff Area=22,269 sf 62.49% Impervious Runoff Depth>1.74" Tc=6.0 min CN=84 Runoff=1.04 cfs 3,227 cf
Subcatchment R3: To RGB	Runoff Area=10,217 sf 71.15% Impervious Runoff Depth>1.97" Tc=6.0 min CN=87 Runoff=0.54 cfs 1,681 cf
Subcatchment R4: To CB-4	Runoff Area=14,887 sf 66.43% Impervious Runoff Depth>1.89" Tc=6.0 min CN=86 Runoff=0.76 cfs 2,350 cf
SubcatchmentR5: To Foxhole	Runoff Area=33,967 sf 52.12% Impervious Runoff Depth>1.45" Flow Length=327' Tc=10.1 min CN=80 Runoff=1.14 cfs 4,112 cf
Subcatchment R6: To CB-6	Runoff Area=26,016 sf 47.47% Impervious Runoff Depth>1.38" Flow Length=248' Tc=16.0 min CN=79 Runoff=0.71 cfs 3,001 cf
Subcatchment R7: To CB-5	Runoff Area=31,901 sf 39.74% Impervious Runoff Depth>1.20" Flow Length=303' Tc=11.9 min CN=76 Runoff=0.82 cfs 3,184 cf
Subcatchment R8: To RGB 2	Runoff Area=17,230 sf 63.94% Impervious Runoff Depth>1.82" Tc=6.0 min CN=85 Runoff=0.84 cfs 2,607 cf

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Subcatchment R9: To RGB 1	Runoff Area=7,051 sf 91.07% Impervious Runoff Depth>2.71" Tc=6.0 min CN=95 Runoff=0.49 cfs 1,593 cf
SubcatchmentS3: To Swale	Runoff Area=10,077 sf 25.91% Impervious Runoff Depth>0.92" Tc=6.0 min CN=71 Runoff=0.23 cfs 772 cf
SubcatchmentS4: To Swale	Runoff Area=1,346 sf 29.12% Impervious Runoff Depth>0.97" Tc=6.0 min CN=72 Runoff=0.03 cfs 109 cf
SubcatchmentS8: To Swale	Runoff Area=5,239 sf 34.13% Impervious Runoff Depth>1.08" Tc=6.0 min CN=74 Runoff=0.15 cfs 473 cf
Reach SW3:	Avg. Flow Depth=0.24' Max Vel=1.14 fps Inflow=0.77 cfs 2,453 cf n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=0.60 cfs 2,436 cf
Reach SW8:	Avg. Flow Depth=0.48' Max Vel=0.29 fps Inflow=0.63 cfs 2,066 cf n=0.240 L=232.0' S=0.0102 '/' Capacity=4.93 cfs Outflow=0.41 cfs 2,045 cf
Pond AD:	Peak Elev=248.70' Inflow=0.02 cfs 105 cf 8.0" Round Culvert n=0.011 L=26.4' S=0.0049 '/' Outflow=0.02 cfs 105 cf
Pond CB1: CB#1	Peak Elev=249.42' Inflow=0.59 cfs 1,835 cf 12.0" Round Culvert n=0.011 L=93.3' S=0.0050 '/' Outflow=0.59 cfs 1,835 cf
Pond CB2:	Peak Elev=251.73' Inflow=1.04 cfs 3,227 cf 12.0" Round Culvert n=0.011 L=73.9' S=0.0100 '/' Outflow=1.04 cfs 3,227 cf
Pond CB4:	Peak Elev=248.27' Inflow=0.76 cfs 2,350 cf 12.0" Round Culvert n=0.011 L=14.5' S=0.0193 '/' Outflow=0.76 cfs 2,350 cf
Pond CB5:	Peak Elev=250.82' Inflow=0.82 cfs 3,184 cf 12.0" Round Culvert n=0.011 L=8.5' S=0.0118 '/' Outflow=0.82 cfs 3,184 cf
Pond CB6:	Peak Elev=248.81' Inflow=0.71 cfs 3,001 cf 12.0" Round Culvert n=0.011 L=6.3' S=0.0063 '/' Outflow=0.71 cfs 3,001 cf
Pond CB7:	Peak Elev=248.38' Inflow=0.63 cfs 2,545 cf 18.0" Round Culvert n=0.011 L=88.3' S=0.0057 '/' Outflow=0.63 cfs 2,545 cf
Pond DMH1:	Peak Elev=248.61' Inflow=0.61 cfs 1,940 cf Outflow=0.61 cfs 1,940 cf
Pond DMH3:	Peak Elev=247.83' Inflow=0.63 cfs 2,545 cf 18.0" Round Culvert n=0.011 L=68.2' S=0.0051 '/' Outflow=0.63 cfs 2,545 cf
Pond DMH4:	Peak Elev=247.37' Inflow=1.31 cfs 4,895 cf 24.0" Round Culvert n=0.011 L=34.6' S=0.0049 '/' Outflow=1.32 cfs 4,894 cf
Pond DMH5:	Peak Elev=252.17' Inflow=1.17 cfs 4,652 cf 15.0" Round Culvert n=0.011 L=116.8' S=0.0102 '/' Outflow=1.17 cfs 4,652 cf
Pond DMH6:	Peak Elev=250.34' Inflow=1.89 cfs 7,836 cf 18.0" Round Culvert n=0.011 L=160.8' S=0.0100 '/' Outflow=1.89 cfs 7,836 cf

19227 - PostDevelopmer Prepared by Howard Stein H HydroCAD® 10.10-6a s/n M2458		<i>-hr 2-yr Rainfall=3.27"</i> Printed 4/11/2022 Page 58
Pond DMH7:	Peak Elev=248.26' 24.0" Round Culvert n=0.011 L=111.5' S=0.0085 '/' (Inflow=2.48 cfs 10,837 cf Dutflow=2.48 cfs 10,837 cf
Pond EX: Existing Abutter De	Peak Elev=250.21' Storage=263 c Discarded=0.08 cfs 1,215 cf Primary=0.00 cfs 0 cf	
Pond HW2:	Peak Elev=253.52 15.0" Round Culvert n=0.013 L=14.6' S=0.0856 '/'	' Inflow=1.17 cfs 4,652 cf Outflow=1.17 cfs 4,652 cf
Pond IB1: Infiltration Basin # Discarded=2.02 cfs 19,852 cf	1 Peak Elev=247.35' Storage=3,113 cf ⁷ Primary=0.30 cfs 704 cf Secondary=0.00 cfs 0 cf 0	
Pond SIS1: Stormtech	Peak Elev=248.15' Storage=200 c Discarded=0.28 cfs 1,940 cf Secondary=0.00 cfs 0 cf	
Pond SIS2:	Peak Elev=249.65' Storage=562 c Discarded=0.44 cfs 3,753 cf Primary=0.00 cfs 0 cf	
Link AP1: To Wetlands		Inflow=0.48 cfs 2,283 cf Primary=0.48 cfs 2,283 cf
Link AP2: To Offsite		Inflow=0.40 cfs 1,688 cf Primary=0.40 cfs 1,688 cf
Link AP3: Abutter Depressio	n	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Total Runoff Area = 313,415 sf Runoff Volume = 30,775 cf Average Runoff Depth = 1.18" 64.11% Pervious = 200,945 sf 35.89% Impervious = 112,470 sf

Summary for Subcatchment A1: To Area Drain

Runoff = 0.02 cfs @ 12.12 hrs, Volume= 105 cf, Depth> 0.47" Routed to Pond AD :

A	rea (sf)	CN E	Description						
	2,664	61 >	61 >75% Grass cover, Good, HSG B						
	2,664	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment A2: To Exterior

Runoff = 0.01 cfs @ 12.12 hrs, Volume= Routed to Link AP1 : To Wetlands 69 cf, Depth> 0.47"

A	rea (sf)	CN E	Description						
	1,761	61 >	61 >75% Grass cover, Good, HSG B						
	1,761	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment A3: To Exterior

Runoff = 0.13 cfs @ 12.09 hrs, Volume= Routed to Link AP1 : To Wetlands 400 cf, Depth> 1.45"

49.95% Pervious Area				
50.05% Impervious Area				
-				

Summary for Subcatchment A4: To Roof Drain

Runoff = 0.15 cfs @ 12.08 hrs, Volume= 526 cf, Depth> 3.03" Routed to Pond SIS2 :

Area (sf)	CN I	Description		
2,082	98 I	Roofs, HSG	ВВ	
2,082		100.00% In	npervious A	Area
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description
6.0				Direct Entry,

Summary for Subcatchment A5: To Abutter

Runoff = 0.23 cfs @ 12.20 hrs, Volume= 1,217 cf, Depth> 0.55" Routed to Pond EX : Existing Abutter Depression

Α	rea (sf)	CN E	Description					
	23,897	61 >	75% Gras	s cover, Go	bod, HSG B			
	1,201	55 V	Voods, Go	od, HSG B				
	1,507	98 F	Roofs, HSG	6 B				
	26,605	63 V	Veighted A	verage				
	25,098	9	4.34% Per	vious Area				
	1,507	5	5.66% Impervious Area					
Тс	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.8	50	0.0120	0.12		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.27"			
5.0	162	0.0060	0.54		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
11.8	212	Total						

Summary for Subcatchment A6: To Abutter

Runoff = 0.40 cfs @ 12.11 hrs, Volume= 1 Routed to Link AP2 : To Offsite

1,688 cf, Depth> 0.55"

Ar	rea (sf)	CN	Description					
	1,281	98	Paved park	ing, HSG B	3			
	25,992	61	>75% Ġras	s cover, Go	ood, HSG B			
	7,984	55	Woods, Go	od, HSG B				
	1,566	98	Roofs, HSG B					
;	36,823	63	53 Weighted Average					
:	33,976		92.27% Pervious Area					
	2,847		7.73% Impervious Area					
_				- ··				
Тс	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment A7: To Infiltration Basin

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717 cf, Depth> 0.47" Runoff 0.15 cfs @ 12.12 hrs, Volume= = Routed to Pond IB1 : Infiltration Basin #1

Area (sf)	CN	Description						
18,226	61	61 >75% Grass cover, Good, HSG B						
18,226		100.00% Pervious Area						
Tc Length (min) (feet)	Slop (ft/f		Capacity (cfs)	Description				
6.0				Direct Entry,				

Summary for Subcatchment A8: To Exterior

Runoff = 0.22 cfs @ 12.13 hrs, Volume= Routed to Link AP1 : To Wetlands 1,109 cf, Depth> 0.44"

Α	rea (sf)	CN I	Description					
	476	98 I	Paved park	ing, HSG B	3			
	20,641	61 >	>75% Gras	s cover, Go	bod, HSG B			
	9,484	55 \	Noods, Go	od, HSG B				
	30,601	60 \	Neighted A	verage				
	30,125	ę	98.44% Per	vious Area	l			
	476		1.56% Impe	ervious Area	а			
_		~		• •	_			
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R1: To CB#1

Runoff = 0.59 cfs @ 12.09 hrs, Volume= 1,835 cf, Depth> 1.97" Routed to Pond CB1 : CB#1

Area (sf)	CN	I Description						
7,872	98	Paved park	ing, HSG B	3				
3,280	61	>75% Grass	s cover, Go	bod, HSG B				
11,152	87	Weighted A	verage					
3,280		29.41% Per	vious Area					
7,872		70.59% Impervious Area						
Tc Length (min) (feet)	Slop (ft/ft		Capacity (cfs)	Description				
6.0				Direct Entry,				

Summary for Subcatchment R2: To CB-2

Runoff = 1.04 cfs @ 12.09 hrs, Volume= 3,227 cf, Depth> 1.74" Routed to Pond CB2 :

Area	a (sf) CN	Description						
8	,993 98	Paved park						
8	,353 61	>75% Gras	s cover, Go	ood, HSG B				
4	,923 98	Roofs, HSC	βB					
22	,269 84	84 Weighted Average						
8	,353	37.51% Pervious Area						
13	,916	62.49% Impervious Area						
	ength Slo		Capacity	Description				
(min)	(feet) (ft/	(ft) (ft/sec)	(cfs)					
6.0				Direct Entry,				

Summary for Subcatchment R3: To RGB

Runoff = 0.54 cfs @ 12.09 hrs, Volume= 1,681 cf, Depth> 1.97" Routed to Reach SW3 :

A	rea (sf)	CN	Description						
	5,386	98	Paved park	ing, HSG B	В				
	2,948	61	>75% Gras	s cover, Go	ood, HSG B				
	1,883	98	Roofs, HSC	ЪВ					
	10,217	87	37 Weighted Average						
	2,948		28.85% Pei	vious Area	а				
	7,269		71.15% Impervious Area						
_									
Tc	Length	Slope		Capacity	1				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment R4: To CB-4

Runoff = 0.76 cfs @ 12.09 hrs, Volume= 2,350 cf, Depth> 1.89" Routed to Pond CB4 :

A	rea (sf)	CN	Description						
	7,280	98	Paved park	ing, HSG B	В				
	4,998	61	>75% Gras	s cover, Go	ood, HSG B				
	2,609	98	Roofs, HSC	βB					
	14,887	86	6 Weighted Average						
	4,998		33.57% Pei	vious Area	a				
	9,889		66.43% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	1				
6.0					Direct Entry,				

Summary for Subcatchment R5: To Foxhole

Runoff = 1.14 cfs @ 12.15 hrs, Volume=

4,112 cf, Depth> 1.45"

Α	vrea (sf)	CN [Description							
	11,765	98 F	98 Paved parking, HSG B							
	16,262	61 >	>75% Ġras	s cover, Go	bod, HSG B					
	5,940	98 F	Roofs, HSC	βB						
	33,967	80 \	Neighted A	verage						
	16,262	2	17.88% Pei	vious Area						
	17,705	Ę	52.12% Imp	pervious Ar	ea					
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
5.6	50	0.0200	0.15		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.27"					
0.5	68	0.0150	2.49		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
3.9	187	0.0130	0.80		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
0.1	22	0.0300	3.52		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
10.1	327	Total								

Summary for Subcatchment R6: To CB-6

Runoff = 0.71 cfs @ 12.23 hrs, Volume= 3,001 cf, Depth> 1.38" Routed to Pond CB6 :

	Α	rea (sf)	CN [Description							
		7,642	98 F	1 07							
		13,667	61 >	>75% Grass cover, Good, HSG B							
		4,707	98 F	Roofs, HSG B							
		26,016	79 \	Veighted A	verage						
	13,667 52.53% Pervious Area										
12,349 47.47% Impervious Area											
	Тс	Length	Slope	Velocity	Capacity	Description					
(m	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
11	1.9	50	0.0030	0.07		Sheet Flow,					
						Grass: Short n= 0.150 P2= 3.27"					
3	3.8	158	0.0100	0.70		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
().3	40	0.0130	2.31		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
16	6.0	248	Total								

Summary for Subcatchment R7: To CB-5

Runoff = 0.82 cfs @ 12.17 hrs, Volume= 3,184 cf, Depth> 1.20" Routed to Pond CB5 :

	A	rea (sf)	CN E	N Description							
		7,161	98 F	98 Paved parking, HSG B							
		19,224	61 >	>75% Grass cover, Good, HSG B							
		5,516	98 F	Roofs, HSG	БВ						
		31,901	76 V	Veighted A	verage						
	19,224 60.26% Pervious Area										
		12,677	3	9.74% Imp	pervious Ar	ea					
	Тс	Length	Slope	Velocity	Capacity	Description					
(<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	6.1	50	0.0160	0.14		Sheet Flow,					
						Grass: Short n= 0.150 P2= 3.27"					
	5.7	226	0.0090	0.66		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.1	27	0.0500	4.54		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	11.9	303	Total								

Summary for Subcatchment R8: To RGB 2

Runoff = 0.84 cfs @ 12.09 hrs, Volume= 2,607 cf, Depth> 1.82" Routed to Pond HW2 :

A	rea (sf)	CN I	Description						
	6,729	98	Paved park	ing, HSG B	В				
	6,213	61 :	>75% Gras	s cover, Go	ood, HSG B				
	4,288	98	Roofs, HSC	βB					
	17,230	85	85 Weighted Average						
	6,213	4	36.06% Pervious Area						
	11,017	(63.94% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	•				
6.0					Direct Entry,				

Summary for Subcatchment R9: To RGB 1

Runoff = 0.49 cfs @ 12.08 hrs, Volume= 1,593 cf, Depth> 2.71" Routed to Reach SW8 :

Α	rea (sf)	CN	Description				
	4,963	98	Paved park	ing, HSG E	В		
	630	61	>75% Gras	s cover, Go	lood, HSG B		
	1,458	98	Roofs, HSC	βB			
	7,051	95	Weighted A	verage			
	630		8.93% Pervious Area				
	6,421	91.07% Impervious Area					
_							
ŢĊ	Length	Slope	,	Capacity	· · · · · · · · · · · · · · · · · · ·		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment S3: To Swale

Runoff = 0.23 cfs @ 12.10 hrs, Volume= 772 cf, Depth> 0.92" Routed to Reach SW3 :

25.91% Impervious Area				
_				

Summary for Subcatchment S4: To Swale

Runoff = 0.03 cfs @ 12.10 hrs, Volume= 109 cf, Depth> 0.97" Routed to Pond CB7 :

Α	rea (sf)	CN	Description				
	392	98	Paved park	ing, HSG B	3		
	954	61	>75% Gras	s cover, Go	ood, HSG B		
	1,346	72	Weighted A	verage			
	954		70.88% Pervious Area				
	392		29.12% Impervious Area				
Тс	Length	Slope		Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment S8: To Swale

Runoff = 0.15 cfs @ 12.10 hrs, Volume= 473 cf, Depth> 1.08" Routed to Reach SW8 :

Ar	rea (sf)	CN	Description		
	1,788	98	Paved park	ing, HSG B	В
	3,451	61	>75% Gras	s cover, Go	ood, HSG B
	5,239	74	Weighted A	verage	
	3,451		65.87% Per	vious Area	a
	1,788		34.13% Imp	pervious Are	rea
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	•
6.0					Direct Entry,
(min)	3,451 5,239 3,451 1,788 Length	61 74 Slope	<u>>75% Grass</u> Weighted A 65.87% Per 34.13% Imp • Velocity	<u>s cover, Go</u> verage vious Area pervious Are Capacity	ood, HSG B a rea Description

Summary for Reach SW3:

Inflow Area = 20,294 sf, 48.68% Impervious, Inflow Depth > 1.45" for 2-yr event Inflow = 0.77 cfs @ 12.09 hrs, Volume= 2.453 cf 0.60 cfs @ 12.16 hrs, Volume= Outflow = 2,436 cf, Atten= 22%, Lag= 4.0 min Routed to Pond CB7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 1.14 fps, Min. Travel Time= 7.3 min Avg. Velocity = 0.40 fps, Avg. Travel Time= 21.1 min Peak Storage= 264 cf @ 12.16 hrs Average Depth at Peak Storage= 0.24', Surface Width= 2.93' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 28.59 cfs 1.50' x 1.50' deep channel, n= 0.041 Riprap, 2-inch Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 501.0' Slope= 0.0100 '/' Inlet Invert= 256.12', Outlet Invert= 251.10'

Summary for Reach SW8:

Inflow Area = 12,290 sf, 66.79% Impervious, Inflow Depth > 2.02" for 2-yr event Inflow = 0.63 cfs @ 12.09 hrs, Volume= 2.066 cf 0.41 cfs @ 12.18 hrs, Volume= Outflow = 2,045 cf, Atten= 36%, Lag= 5.8 min Routed to Pond HW2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 0.29 fps, Min. Travel Time= 13.5 min Avg. Velocity = 0.10 fps, Avg. Travel Time= 37.0 min Peak Storage= 328 cf @ 12.18 hrs Average Depth at Peak Storage= 0.48', Surface Width= 4.38' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 4.93 cfs 1.50' x 1.50' deep channel, n= 0.240 Sheet flow over Dense Grass Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 232.0' Slope= 0.0102 '/' Inlet Invert= 255.37', Outlet Invert= 253.00'

Summary for Pond AD:

Inflow Area = 2,664 sf, 0.00% Impervious, Inflow Depth > 0.47" for 2-yr event 0.02 cfs @ 12.12 hrs, Volume= Inflow = 105 cf 0.02 cfs @ 12.12 hrs, Volume= Outflow = 105 cf, Atten= 0%, Lag= 0.0 min 0.02 cfs @ 12.12 hrs, Volume= 105 cf Primary = Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.70' @ 12.11 hrs Flood Elev= 250.75' Device Routing Invert Outlet Devices #1 Primary 248.60' 8.0" Round Culvert L= 26.4' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.60' / 248.47' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.35 sf

Primary OutFlow Max=0.02 cfs @ 12.12 hrs HW=248.70' TW=248.60' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.02 cfs @ 0.99 fps)

Summary for Pond CB1: CB#1

Inflow Area = 11,152 sf, 70.59% Impervious, Inflow Depth > 1.97" for 2-yr event Inflow = 0.59 cfs @ 12.09 hrs, Volume= 1.835 cf 0.59 cfs @ 12.09 hrs, Volume= Outflow = 1,835 cf, Atten= 0%, Lag= 0.0 min 0.59 cfs @ 12.09 hrs, Volume= Primary = 1,835 cf Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.42' @ 12.09 hrs Flood Elev= 251.50' Device Routing Invert Outlet Devices #1 Primary 249.00' 12.0" Round Culvert L= 93.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.00' / 248.53' S= 0.0050 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=0.59 cfs @ 12.09 hrs HW=249.42' TW=248.61' (Dynamic Tailwater)

1=Culvert (Barrel Controls 0.59 cfs @ 2.78 fps)

Summary for Pond CB2:

Inflow Area = 22,269 sf, 62.49% Impervious, Inflow Depth > 1.74" for 2-yr event Inflow = 1.04 cfs @ 12.09 hrs, Volume= 3.227 cf 1.04 cfs @ 12.09 hrs, Volume= Outflow = 3,227 cf, Atten= 0%, Lag= 0.0 min 1.04 cfs @ 12.09 hrs, Volume= Primary = 3,227 cf Routed to Pond SIS2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.73' @ 12.09 hrs Flood Elev= 254.02' Device Routing Invert Outlet Devices #1 251.20' Primary 12.0" Round Culvert L= 73.9' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.20' / 250.46' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=1.03 cfs @ 12.09 hrs HW=251.73' TW=249.39' (Dynamic Tailwater)

1=Culvert (Inlet Controls 1.03 cfs @ 2.47 fps)

Summary for Pond CB4:

Inflow = 0.7 Outflow = 0.7	,887 sf, 66.43% Impervious, Inflow Depth > 1.89" for 2-yr event cfs @ 12.09 hrs, Volume= 2,350 cf cfs @ 12.09 hrs, Volume= 2,350 cf, Atten= 0%, Lag= 0.0 min cfs @ 12.09 hrs, Volume= 2,350 cf 4 :						
0, ,	Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.27' @ 12.09 hrs Flood Elev= 250.69'						
Device Routing	Invert Outlet Devices						
#1 Primary	247.83' 12.0" Round Culvert L= 14.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.83' / 247.55' S= 0.0193 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf						
Primary OutFlow Max=0.75 cfs @ 12.09 hrs HW=248.27' TW=247.25' (Dynamic Tailwater) ↓ 1=Culvert (Inlet Controls 0.75 cfs @ 2.26 fps)							

Summary for Pond CB5:

Inflow Area = 31,901 sf, 39.74% Impervious, Inflow Depth > 1.20" for 2-yr event Inflow = 0.82 cfs @ 12.17 hrs, Volume= 3.184 cf 0.82 cfs @ 12.17 hrs, Volume= Outflow = 3,184 cf, Atten= 0%, Lag= 0.0 min 0.82 cfs @ 12.17 hrs, Volume= Primary = 3,184 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.82' @ 12.17 hrs Flood Elev= 252.45' Device Routing Invert Outlet Devices #1 250.30' Primary 12.0" Round Culvert L= 8.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 250.30' / 250.20' S= 0.0118 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=0.82 cfs @ 12.17 hrs HW=250.82' TW=250.32' (Dynamic Tailwater)

Primary OutFlow Max=0.82 cts @ 12.17 hrs HW= 250.82° TW= 250.32° (Dyna **1=Culvert** (Barrel Controls 0.82 cfs @ 2.91 fps)

Summary for Pond CB6:

Inflow Area = 26,016 sf, 47.47% Impervious, Inflow Depth > 1.38" for 2-yr event 0.71 cfs @ 12.23 hrs. Volume= Inflow = 3.001 cf 0.71 cfs @ 12.23 hrs, Volume= Outflow = 3,001 cf, Atten= 0%, Lag= 0.0 min 0.71 cfs @ 12.23 hrs, Volume= Primary = 3,001 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.81' @ 12.23 hrs Flood Elev= 250.82' Device Routing Invert Outlet Devices #1 Primary 248.30' 12.0" Round Culvert L= 6.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.30' / 248.26' S= 0.0063 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=0.70 cfs @ 12.23 hrs HW=248.81' TW=248.23' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.70 cfs @ 2.54 fps)

Summary for Pond CB7:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 1.41" for 2-yr event 0.63 cfs @ 12.15 hrs, Volume= Inflow = 2.545 cf 0.63 cfs @ 12.15 hrs, Volume= Outflow = 2,545 cf, Atten= 0%, Lag= 0.0 min 0.63 cfs @ 12.15 hrs, Volume= Primary = 2,545 cf Routed to Pond DMH3 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.38' @ 12.15 hrs Flood Elev= 253.00' Device Routing **Outlet Devices** Invert #1 Primary 248.00' 18.0" Round Culvert L= 88.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.00' / 247.50' S= 0.0057 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=0.63 cfs @ 12.15 hrs HW=248.38' TW=247.83' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.63 cfs @ 2.71 fps)

Summary for Pond DMH1:

Inflow Area = 13,816 sf, 56.98% Impervious, Inflow Depth > 1.69" for 2-yr event Inflow = 0.61 cfs @ 12.09 hrs, Volume= 1,940 cf Outflow = 0.61 cfs @ 12.09 hrs, Volume= 1,940 cf, Atten= 0%, Lag= 0.0 min Primary = 0.61 cfs @ 12.09 hrs, Volume= 1,940 cf Routed to Pond SIS1 : Stormtech 1,940 cf					
Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.61' @ 12.09 hrs Flood Elev= 252.50'					
Device Routing Invert Outlet Devices					
#1 Primary 248.40' 12.0'' Round MANIFOLD L= 6.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.40' / 248.35' S= 0.0083 '/' Cc= 0.900					
 #2 Primary 248.28' 248.28' 248.28' 24.0" Round ISOLATOR L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.28' / 248.25' S= 0.0075 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf 					
Primary OutFlow Max=0.61 cfs @ 12.09 hrs HW=248.61' TW=247.95' (Dynamic Tailwater) 1=MANIFOLD (Barrel Controls 0.14 cfs @ 1.86 fps) 2=ISOLATOR (Barrel Controls 0.46 cfs @ 2.09 fps)					

Summary for Pond DMH3:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 1.41" for 2-yr event 0.63 cfs @ 12.15 hrs, Volume= Inflow = 2.545 cf 0.63 cfs @ 12.15 hrs, Volume= Outflow = 2,545 cf, Atten= 0%, Lag= 0.0 min 0.63 cfs @ 12.15 hrs, Volume= Primary = 2,545 cf Routed to Pond DMH4 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.83' @ 12.15 hrs Flood Elev= 251.98' Device Routing Invert Outlet Devices #1 247.45' Primary 18.0" Round Culvert L= 68.2' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.45' / 247.10' S= 0.0051 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=0.63 cfs @ 12.15 hrs HW=247.83' TW=247.29' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.63 cfs @ 2.67 fps)

Summary for Pond DMH4:

Inflow Area = 36,527 sf, 55.19% Impervious, Inflow Depth > 1.61" for 2-yr event Inflow = 1.31 cfs @ 12.11 hrs, Volume= 4.895 cf 1.32 cfs @ 12.11 hrs, Volume= Outflow = 4,894 cf, Atten= 0%, Lag= 0.0 min 1.32 cfs @ 12.11 hrs, Volume= Primary = 4,894 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.37' @ 12.45 hrs Flood Elev= 251.00' Device Routing Invert Outlet Devices #1 Primary 246.60' 24.0" Round Culvert L= 34.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 246.60' / 246.43' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf **Primary OutFlow** Max=1.31 cfs @ 12.11 hrs HW=247.27' TW=247.11' (Dynamic Tailwater)

1=Culvert (Outlet Controls 1.31 cfs @ 2.13 fps)

Summary for Pond DMH5:

Inflow Area = 29,520 sf, 65.13% Impervious, Inflow Depth > 1.89" for 2-yr event Inflow = 1.17 cfs @ 12.10 hrs, Volume= 4.652 cf 1.17 cfs @ 12.10 hrs, Volume= Outflow = 4,652 cf, Atten= 0%, Lag= 0.0 min 1.17 cfs @ 12.10 hrs, Volume= Primary = 4,652 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 252.17' @ 12.10 hrs Flood Elev= 254.00' Device Routing Invert Outlet Devices #1 251.65' Primary 15.0" Round Culvert L= 116.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.65' / 250.46' S= 0.0102 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.23 sf

Primary OutFlow Max=1.17 cfs @ 12.10 hrs HW=252.17' TW=250.33' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.17 cfs @ 2.45 fps)

Summary for Pond DMH6:

Inflow Area = 61,421 sf, 51.94% Impervious, Inflow Depth > 1.53" for 2-yr event Inflow = 1.89 cfs @ 12.13 hrs, Volume= 7.836 cf 1.89 cfs @ 12.13 hrs, Volume= Outflow = 7,836 cf, Atten= 0%, Lag= 0.0 min 1.89 cfs @ 12.13 hrs, Volume= Primary = 7,836 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.34' @ 12.13 hrs Flood Elev= 252.93' Device Routing Invert Outlet Devices #1 249.71' Primary 18.0" Round Culvert L= 160.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.71' / 248.10' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=1.89 cfs @ 12.13 hrs HW=250.34' TW=248.25' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.89 cfs @ 2.70 fps)

Summary for Pond DMH7:

Inflow Area = 87,437 sf, 50.61% Impervious, Inflow Depth > 1.49" for 2-yr event Inflow = 2.48 cfs @ 12.15 hrs, Volume= 10.837 cf 2.48 cfs @ 12.15 hrs, Volume= Outflow = 10,837 cf, Atten= 0%, Lag= 0.0 min 2.48 cfs @ 12.15 hrs, Volume= Primary = 10.837 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.26' @ 12.15 hrs Flood Elev= 251.25' Device Routing Invert Outlet Devices #1 247.60' Primary 24.0" Round Culvert L= 111.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.60' / 246.65' S= 0.0085 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf Primary OutFlow Max=2.47 cfs @ 12.15 hrs HW=248.26' TW=247.17' (Dynamic Tailwater)

1=Culvert (Inlet Controls 2.47 cfs @ 2.76 fps)

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	26,605 sf, 5.66% Impervious,	Inflow Depth > 0.55" for 2-yr event
Inflow =	0.23 cfs @ 12.20 hrs, Volume=	1,217 cf
Outflow =	0.08 cfs @ 12.73 hrs, Volume=	1,215 cf, Atten= 67%, Lag= 31.3 min
Discarded =	0.08 cfs @ 12.73 hrs, Volume=	1,215 cf
Primary =	0.00 cfs @ 2.00 hrs, Volume=	0 cf
Routed to Link	AP3 : Abutter Depression	

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.21' @ 12.73 hrs Surf.Area= 1,289 sf Storage= 263 cf

Plug-Flow detention time= 36.6 min calculated for 1,215 cf (100% of inflow) Center-of-Mass det. time= 36.0 min (941.3 - 905.3)

Volume	Invert	Avail.Sto	rage Stora	ge Description		
#1	249.80'	1,12	22 cf Cust	om Stage Data (P	rismatic)Listed below	/ (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4 250.6	t) 0 0 0 0 0	urf.Area (sq-ft) 164 562 1,261 2,144 3,127	Inc.Store (cubic-feet) 0 73 182 341 527	(cubic-feet) 0 73 255 595		
<u>Device</u> #1 #2	Routing Discarded Primary	<u>Invert</u> 249.80' 250.25'	Outlet Dev 2.410 in/h Conductivi 20.0' long Head (feet	ices r Exfiltration over ty to Groundwater x 15.0' breadth B) 0.20 0.40 0.60		1.60

Discarded OutFlow Max=0.08 cfs @ 12.73 hrs HW=250.21' (Free Discharge) **1=Exfiltration** (Controls 0.08 cfs)

Primary OutFlow Max=0.00 cfs @ 2.00 hrs HW=249.80' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Pond HW2:

[62] Hint: Exceeded Reach SW8 OUTLET depth by 0.10' @ 12.06 hrs

 Inflow Area =
 29,520 sf, 65.13% Impervious, Inflow Depth > 1.89" for 2-yr event

 Inflow =
 1.17 cfs @
 12.10 hrs, Volume=
 4,652 cf

 Outflow =
 1.17 cfs @
 12.10 hrs, Volume=
 4,652 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.17 cfs @
 12.10 hrs, Volume=
 4,652 cf

 Routed to Pond DMH5 :
 12.10 hrs, Volume=
 4,652 cf

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 253.52' @ 12.10 hrs Flood Elev= 254.00'

Device	Routing	Invert	Outlet Devices				
#1	Primary	253.00'	15.0" Round Culvert L= 14.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 253.00' / 251.75' S= 0.0856 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf				
Drimon	Drimony OutFlow May-1 17 of @ 12 10 bro LIW-252 52' TW-252 17' (Dynamia Tailyystor)						

Primary OutFlow Max=1.17 cfs @ 12.10 hrs HW=253.52' TW=252.17' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.17 cfs @ 2.45 fps)

Summary for Pond IB1: Infiltration Basin #1

[80] Warning: Exceeded Pond DMH4 by 0.31' @ 2.00 hrs (0.76 cfs 2,021 cf)

Inflow Area =	200,508 sf	, 48.93% Impervious,	Inflow Depth > 1.23" for 2-yr event			
Inflow =	5.02 cfs @	12.14 hrs, Volume=	20,561 cf			
Outflow =	2.32 cfs @	12.48 hrs, Volume=	20,556 cf, Atten= 54%, Lag= 20.7 min			
Discarded =	2.02 cfs @	12.48 hrs, Volume=	19,852 cf			
Primary =	0.30 cfs @	12.48 hrs, Volume=	704 cf			
Routed to Link	AP1 : To Wet	lands				
Secondary =	0.00 cfs @	2.00 hrs, Volume=	0 cf			
Routed to Link AP1 : To Wetlands						

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.35' @ 12.48 hrs Surf.Area= 9,063 sf Storage= 3,113 cf

Plug-Flow detention time= 7.7 min calculated for 20,537 cf (100% of inflow) Center-of-Mass det. time= 7.6 min (849.6 - 842.0)

Volume	Invert	Avail.Sto	orage	Storage Description		
#1	247.00'	32,7	′58 cf	Custom Stage Data	a (Irregular)Listed	below (Recalc)
Elevatio (fee		rf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
247.0 248.0 249.0 250.0)0)0	8,532 10,075 11,718	373.0 398.2 423.3 448.4	0 9,293 10,886 12,579	0 9,293 20,179 32,758	8,532 10,126 11,817 13,612
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	247.00'	-	0 in/hr Exfiltration o		
#2	Primary	247.00'	18.0 Inlet	ductivity to Groundwa " Round Culvert L= / Outlet Invert= 247.0 .012, Flow Area= 1.7	= 62.2' Ke= 0.500 00' / 246.00' S= 0.	
#3	Device 2	247.00'				l to weir flow at low heads
#4	Device 2	248.20'	-	x 2.0" Horiz. Orifice		-
#5	Secondary	249.00'	Limit 10.0 Head	rows C= 0.600 in 24.0 ted to weir flow at low ' long x 10.0' breadt d (feet) 0.20 0.40 0. f. (English) 2.49 2.56	heads h Broad-Crested 60 0.80 1.00 1.2	Rectangular Weir 0 1.40 1.60

Discarded OutFlow Max=2.02 cfs @ 12.48 hrs HW=247.35' (Free Discharge) **1=Exfiltration** (Controls 2.02 cfs)

Primary OutFlow Max=0.30 cfs @ 12.48 hrs HW=247.35' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 0.30 cfs of 0.64 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.30 cfs @ 2.03 fps) 4=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.00' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond SIS1: Stormtech

Inflow Area = 13,816 sf, 56.98% Impervious, Inflow Depth > 1.69" for 2-yr event Inflow = 0.61 cfs @ 12.09 hrs, Volume= 1.940 cf 0.28 cfs @ 12.29 hrs, Volume= 1,940 cf, Atten= 54%, Lag= 11.8 min Outflow = 0.28 cfs @ 12.29 hrs, Volume= Discarded = 1,940 cf Secondary = 0.00 cfs @ 2.00 hrs, Volume= 0 cf Routed to Link AP1 : To Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.15' @ 12.29 hrs Surf.Area= 1,242 sf Storage= 200 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 3.2 min (825.6 - 822.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	247.75'	1,151 cf	20.50'W x 60.58'L x 3.50'H Field A
			4,346 cf Overall - 1,470 cf Embedded = 2,876 cf x 40.0% Voids
#2A	248.25'	1,470 cf	ADS_StormTech SC-740 +Cap x 32 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			32 Chambers in 4 Rows
#3	248.00'	35 cf	4.00'D x 2.75'H Vertical Cone/Cylinder-Impervious
		2,655 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	247.75'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 245.51' Phase-In= 0.01'
#2	Secondary	250.60'	2.0" x 2.0" Horiz. Orifice/Grate X 7.00 columns
			X 7 rows C= 0.600 in 24.0" x 24.0" Grate (34% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.28 cfs @ 12.29 hrs HW=248.15' (Free Discharge) **1=Exfiltration** (Controls 0.28 cfs)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.75' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Pond SIS1: Stormtech - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 58.58' Row Length +12.0" End Stone x 2 = 60.58' Base Length 4 Rows x 51.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.50' Base Width

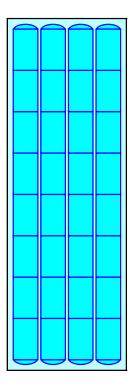
6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

32 Chambers x 45.9 cf = 1,470.1 cf Chamber Storage

4,346.4 cf Field - 1,470.1 cf Chambers = 2,876.3 cf Stone x 40.0% Voids = 1,150.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,620.6 cf = 0.060 afOverall Storage Efficiency = 60.3%Overall System Size = $60.58' \times 20.50' \times 3.50'$

32 Chambers 161.0 cy Field 106.5 cy Stone





Summary for Pond SIS2:

Inflow Area =	24,351 sf,	65.70% Impervious,	Inflow Depth > 1.85"	for 2-yr event
Inflow =	1.19 cfs @	12.09 hrs, Volume=	3,754 cf	
Outflow =	0.44 cfs @	12.37 hrs, Volume=	3,753 cf, Atter	n= 63%, Lag= 16.7 min
Discarded =	0.44 cfs @	12.37 hrs, Volume=	3,753 cf	
Primary =	0.00 cfs @	2.00 hrs, Volume=	0 cf	
Routed to Pond	I IB1 : Infiltrati	on Basin #1		

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.65' @ 12.37 hrs Surf.Area= 1,735 sf Storage= 562 cf

Plug-Flow detention time= 6.4 min calculated for 3,750 cf (100% of inflow) Center-of-Mass det. time= 6.3 min (824.1 - 817.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	249.00'	1,603 cf	44.25'W x 39.22'L x 3.50'H Field A
			6,074 cf Overall - 2,067 cf Embedded = 4,006 cf x 40.0% Voids
#2A	249.50'	2,067 cf	ADS_StormTech SC-740 +Cap x 45 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			45 Chambers in 9 Rows
#3	250.80'	38 cf	4.00'D x 3.00'H Vertical Cone/Cylinder
		3,708 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	249.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01'
#2	Primary	253.50'	2.0" x 2.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 24.0" x 24.0" Grate (25% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.44 cfs @ 12.37 hrs HW=249.65' (Free Discharge) **1=Exfiltration** (Controls 0.44 cfs)

Primary OutFlow Max=0.00 cfs @ 2.00 hrs HW=249.00' TW=247.00' (Dynamic Tailwater)

Pond SIS2: - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

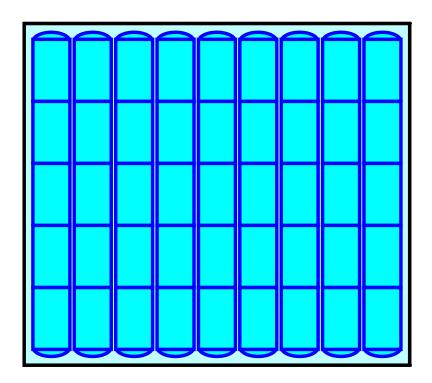
5 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 37.22' Row Length +12.0" End Stone x 2 = 39.22' Base Length 9 Rows x 51.0" Wide + 6.0" Spacing x 8 + 12.0" Side Stone x 2 = 44.25' Base Width 6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

45 Chambers x 45.9 cf = 2,067.3 cf Chamber Storage

6,073.7 cf Field - 2,067.3 cf Chambers = 4,006.4 cf Stone x 40.0% Voids = 1,602.6 cf Stone Storage

Chamber Storage + Stone Storage = 3,669.9 cf = 0.084 afOverall Storage Efficiency = 60.4%Overall System Size = $39.22' \times 44.25' \times 3.50'$

45 Chambers 225.0 cy Field 148.4 cy Stone





Summary for Link AP1: To Wetlands

Inflow Area	a =	236,171 sf	, 42.45% Impervious,	Inflow Depth >	0.12"	for 2-yr event
Inflow	=	0.48 cfs @	12.37 hrs, Volume=	2,283 0	of	
Primary	=	0.48 cfs @	12.37 hrs, Volume=	2,283 c	of, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP2: To Offsite

Inflow Are	a =	36,823 sf,	7.73% Impervious,	Inflow Depth >	0.55"	for 2-yr event
Inflow	=	0.40 cfs @	12.11 hrs, Volume=	1,688 c	f	
Primary	=	0.40 cfs @	12.11 hrs, Volume=	1,688 c	f, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP3: Abutter Depression

Inflow Area	a =	26,605 sf,	5.66% Impervious,	Inflow Depth = 0.00"	for 2-yr event
Inflow	=	0.00 cfs @	2.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	2.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Time span=2.00-24.00 hrs, dt=0.02 hrs, 1101 points x 3 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment A1: To Area Drain	Runoff Area=2,664 sf 0.00% Impervious Runoff Depth>1.32" Tc=6.0 min CN=61 Runoff=0.09 cfs 293 cf
Subcatchment A2: To Exterior	Runoff Area=1,761 sf 0.00% Impervious Runoff Depth>1.32" Tc=6.0 min CN=61 Runoff=0.06 cfs 194 cf
SubcatchmentA3: To Exterior	Runoff Area=3,301 sf 50.05% Impervious Runoff Depth>2.82" Tc=6.0 min CN=80 Runoff=0.25 cfs 776 cf
Subcatchment A4: To Roof Drain	Runoff Area=2,082 sf 100.00% Impervious Runoff Depth>4.67" Tc=6.0 min CN=98 Runoff=0.23 cfs 810 cf
Subcatchment A5: To Abutter	Runoff Area=26,605 sf 5.66% Impervious Runoff Depth>1.45" Flow Length=212' Tc=11.8 min CN=63 Runoff=0.79 cfs 3,223 cf
Subcatchment A6: To Abutter	Runoff Area=36,823 sf 7.73% Impervious Runoff Depth>1.46" Tc=6.0 min CN=63 Runoff=1.34 cfs 4,469 cf
Subcatchment A7: To Infiltration Basin	Runoff Area=18,226 sf 0.00% Impervious Runoff Depth>1.32" Tc=6.0 min CN=61 Runoff=0.59 cfs 2,004 cf
Subcatchment A8: To Exterior	Runoff Area=30,601 sf 1.56% Impervious Runoff Depth>1.25" Tc=6.0 min CN=60 Runoff=0.92 cfs 3,194 cf
Subcatchment R1: To CB#1	Runoff Area=11,152 sf 70.59% Impervious Runoff Depth>3.49" Tc=6.0 min CN=87 Runoff=1.03 cfs 3,242 cf
Subcatchment R2: To CB-2	Runoff Area=22,269 sf 62.49% Impervious Runoff Depth>3.19" Tc=6.0 min CN=84 Runoff=1.90 cfs 5,928 cf
Subcatchment R3: To RGB	Runoff Area=10,217 sf 71.15% Impervious Runoff Depth>3.49" Tc=6.0 min CN=87 Runoff=0.94 cfs 2,970 cf
Subcatchment R4: To CB-4	Runoff Area=14,887 sf 66.43% Impervious Runoff Depth>3.39" Tc=6.0 min CN=86 Runoff=1.34 cfs 4,205 cf
Subcatchment R5: To Foxhole	Runoff Area=33,967 sf 52.12% Impervious Runoff Depth>2.82" Flow Length=327' Tc=10.1 min CN=80 Runoff=2.25 cfs 7,976 cf
Subcatchment R6: To CB-6	Runoff Area=26,016 sf 47.47% Impervious Runoff Depth>2.72" Flow Length=248' Tc=16.0 min CN=79 Runoff=1.41 cfs 5,906 cf
Subcatchment R7: To CB-5	Runoff Area=31,901 sf 39.74% Impervious Runoff Depth>2.46" Flow Length=303' Tc=11.9 min CN=76 Runoff=1.74 cfs 6,550 cf
Subcatchment R8: To RGB 2	Runoff Area=17,230 sf 63.94% Impervious Runoff Depth>3.29" Tc=6.0 min CN=85 Runoff=1.51 cfs 4,725 cf

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Subcatchment R9: To RGB 1	Runoff Area=7,051 sf 91.07% Impervious Runoff Depth>4.34" Tc=6.0 min CN=95 Runoff=0.76 cfs 2,548 cf
SubcatchmentS3: To Swale	Runoff Area=10,077 sf 25.91% Impervious Runoff Depth>2.05" Tc=6.0 min CN=71 Runoff=0.55 cfs 1,725 cf
SubcatchmentS4: To Swale	Runoff Area=1,346 sf 29.12% Impervious Runoff Depth>2.13" Tc=6.0 min CN=72 Runoff=0.08 cfs 239 cf
SubcatchmentS8: To Swale	Runoff Area=5,239 sf 34.13% Impervious Runoff Depth>2.30" Tc=6.0 min CN=74 Runoff=0.32 cfs 1,003 cf
Reach SW3:	Avg. Flow Depth=0.35' Max Vel=1.39 fps Inflow=1.49 cfs 4,695 cf n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=1.23 cfs 4,670 cf
Reach SW8:	Avg. Flow Depth=0.64' Max Vel=0.34 fps Inflow=1.08 cfs 3,551 cf n=0.240 L=232.0' S=0.0102 '/' Capacity=4.93 cfs Outflow=0.74 cfs 3,524 cf
Pond AD:	Peak Elev=248.81' Inflow=0.09 cfs 293 cf 8.0" Round Culvert n=0.011 L=26.4' S=0.0049 '/' Outflow=0.09 cfs 293 cf
Pond CB1: CB#1	Peak Elev=249.57' Inflow=1.03 cfs 3,242 cf 12.0" Round Culvert n=0.011 L=93.3' S=0.0050 '/' Outflow=1.03 cfs 3,242 cf
Pond CB2:	Peak Elev=251.96' Inflow=1.90 cfs 5,928 cf 12.0" Round Culvert n=0.011 L=73.9' S=0.0100 '/' Outflow=1.90 cfs 5,928 cf
Pond CB4:	Peak Elev=248.44' Inflow=1.34 cfs 4,205 cf 12.0" Round Culvert n=0.011 L=14.5' S=0.0193 '/' Outflow=1.34 cfs 4,205 cf
Pond CB5:	Peak Elev=251.12' Inflow=1.74 cfs 6,550 cf 12.0" Round Culvert n=0.011 L=8.5' S=0.0118 '/' Outflow=1.74 cfs 6,550 cf
Pond CB6:	Peak Elev=249.07' Inflow=1.41 cfs 5,906 cf 12.0" Round Culvert n=0.011 L=6.3' S=0.0063 '/' Outflow=1.41 cfs 5,906 cf
Pond CB7:	Peak Elev=248.57' Inflow=1.29 cfs 4,909 cf 18.0" Round Culvert n=0.011 L=88.3' S=0.0057 '/' Outflow=1.29 cfs 4,909 cf
Pond DMH1:	Peak Elev=248.72' Inflow=1.11 cfs 3,535 cf Outflow=1.11 cfs 3,535 cf
Pond DMH3:	Peak Elev=248.05' Inflow=1.29 cfs 4,909 cf 18.0" Round Culvert n=0.011 L=68.2' S=0.0051 '/' Outflow=1.29 cfs 4,909 cf
Pond DMH4:	Peak Elev=247.99' Inflow=2.52 cfs 9,114 cf 24.0" Round Culvert n=0.011 L=34.6' S=0.0049 '/' Outflow=2.52 cfs 9,114 cf
Pond DMH5:	Peak Elev=252.37' Inflow=2.13 cfs 8,249 cf 15.0" Round Culvert n=0.011 L=116.8' S=0.0102 '/' Outflow=2.13 cfs 8,249 cf
Pond DMH6:	Peak Elev=250.63' Inflow=3.69 cfs 14,799 cf 18.0" Round Culvert n=0.011 L=160.8' S=0.0100 '/' Outflow=3.69 cfs 14,799 cf

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	Soz © 2020 Hydrochd Soliware Solutions LLC Page 107
Pond DMH7:	Peak Elev=248.57' Inflow=4.88 cfs 20,705 cf 24.0" Round Culvert n=0.011 L=111.5' S=0.0085 '/' Outflow=4.88 cfs 20,705 cf
Pond EX: Existing Abutter	DepressionPeak Elev=250.30' Storage=407 cfInflow=0.79 cfs3,223 cfDiscarded=0.10 cfs2,270 cfPrimary=0.64 cfs934 cfOutflow=0.75 cfs3,204 cf
Pond HW2:	Peak Elev=253.72' Inflow=2.13 cfs 8,249 cf 15.0" Round Culvert n=0.013 L=14.6' S=0.0856 '/' Outflow=2.13 cfs 8,249 cf
Pond IB1: Infiltration Basin Discarded=2.75 cfs 35,901 c	#1 Peak Elev=247.98' Storage=9,094 cf Inflow=10.09 cfs 39,799 cf Primary=0.81 cfs 3,890 cf Secondary=0.00 cfs 0 cf Outflow=3.56 cfs 39,791 cf
Pond SIS1: Stormtech	Peak Elev=248.67' Storage=681 cf Inflow=1.11 cfs 3,535 cf Discarded=0.34 cfs 3,535 cf Secondary=0.00 cfs 0 cf Outflow=0.34 cfs 3,535 cf
Pond SIS2:	Peak Elev=250.38' Storage=1,561 cf Inflow=2.13 cfs 6,738 cf Discarded=0.56 cfs 6,737 cf Primary=0.00 cfs 0 cf Outflow=0.56 cfs 6,737 cf
Link AP1: To Wetlands	Inflow=1.61 cfs 8,054 cf Primary=1.61 cfs 8,054 cf
Link AP2: To Offsite	Inflow=1.34 cfs 4,469 cf Primary=1.34 cfs 4,469 cf
Link AP3: Abutter Depress	on Inflow=0.64 cfs 934 cf Primary=0.64 cfs 934 cf

Total Runoff Area = 313,415 sf Runoff Volume = 61,979 cf Average Runoff Depth = 2.37" 64.11% Pervious = 200,945 sf 35.89% Impervious = 112,470 sf

Summary for Subcatchment A1: To Area Drain

Runoff = 0.09 cfs @ 12.10 hrs, Volume= 293 cf, Depth> 1.32" Routed to Pond AD :

Area (sf)	CN I	Description						
2,664	61 :	1 >75% Grass cover, Good, HSG B						
2,664		100.00% Pervious Area						
Tc Length (min) (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0				Direct Entry,				

Summary for Subcatchment A2: To Exterior

Runoff = 0.06 cfs @ 12.10 hrs, Volume= Routed to Link AP1 : To Wetlands 194 cf, Depth> 1.32"

A	rea (sf)	CN E	Description						
	1,761	61 >	51 >75% Grass cover, Good, HSG B						
	1,761	100.00% Pervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment A3: To Exterior

Runoff = 0.25 cfs @ 12.09 hrs, Volume= Routed to Link AP1 : To Wetlands 776 cf, Depth> 2.82"

Α	rea (sf)	(sf) CN Description				
	1,649	61	>75% Grass cover, Good, HSG B			
	1,652	98	Roofs, HSG B			
	3,301	80	30 Weighted Average			
	1,649	1,649 49.95% Pervious Area				
	1,652		50.05% Impervious Area			
_				. .		
Тс	Length	Slope		Capacity	1	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	
					-	

Summary for Subcatchment A4: To Roof Drain

Runoff = 0.23 cfs @ 12.08 hrs, Volume= 810 cf, Depth> 4.67" Routed to Pond SIS2 :

Area (sf)	CN	Description						
2,082	98	98 Roofs, HSG B						
2,082		100.00% In	npervious A	vrea				
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
6.0				Direct Entry,				

Summary for Subcatchment A5: To Abutter

Runoff = 0.79 cfs @ 12.18 hrs, Volume= 3,223 cf, Depth> 1.45" Routed to Pond EX : Existing Abutter Depression

Α	rea (sf)	CN E	CN Description						
	23,897	61 >	75% Gras	s cover, Go	bod, HSG B				
	1,201	55 V	Voods, Go	od, HSG B					
	1,507	98 F	Roofs, HSG	6 B					
	26,605	63 V	Veighted A	verage					
	25,098	9	4.34% Per	vious Area					
	1,507	5	.66% Impe	ervious Area	a				
Тс	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.8	50	0.0120	0.12		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.27"				
5.0	162	0.0060	0.54		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
11.8	212	Total							

Summary for Subcatchment A6: To Abutter

Runoff = 1.34 cfs @ 12.10 hrs, Volume= 4,469 cf, Depth> 1.46" Routed to Link AP2 : To Offsite

A	rea (sf)	CN	Description							
	1,281	98	Paved park	ing, HSG B	3					
	25,992	61	>75% Gras	s cover, Go	ood, HSG B					
	7,984	55	Noods, Go	od, HSG B						
	1,566	98	Roofs, HSG	БB						
	36,823	63	Neighted A	verage						
	33,976	1	92.27% Pervious Area							
	2,847		7.73% Impe	ervious Are	a					
Tc	Length	Slope		Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment A7: To Infiltration Basin

Runoff = 0.59 cfs @ 12.10 hrs, Volume= 2,004 cf, Depth> 1.32" Routed to Pond IB1 : Infiltration Basin #1

Area (sf)	CN	V Description						
18,226	61	61 >75% Grass cover, Good, HSG B						
18,226		100.00% Pe	ervious Are	a				
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
6.0				Direct Entry,				

Summary for Subcatchment A8: To Exterior

Runoff = 0.92 cfs @ 12.10 hrs, Volume= 3,194 cf, Depth> 1.25" Routed to Link AP1 : To Wetlands

A	rea (sf)	CN [Description					
	476	98 F	Paved park	ing, HSG B	3			
	20,641	61 >	>75% Ġras	s cover, Go	ood, HSG B			
	9,484	55 \	Noods, Go	od, HSG B				
	30,601	60 \	60 Weighted Average					
	30,125	ę	98.44% Per	vious Area	3			
	476		l.56% Impe	ervious Area	a			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment R1: To CB#1

Runoff = 1.03 cfs @ 12.09 hrs, Volume= 3,242 cf, Depth> 3.49" Routed to Pond CB1 : CB#1

A	rea (sf)	CN I	Description						
	7,872	98	Paved park	ing, HSG B	3				
	3,280	61 :	>75% Gras	s cover, Go	ood, HSG B				
	11,152	87	Neighted A	verage					
	3,280		29.41% Per	vious Area	3				
	7,872	-	70.59% Imp	pervious Are	rea				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)		(cfs)	Description				
6.0	((10/10)	(13,000)	(010)	Direct Entry,				
0.0					Dirott Elitif,				

Summary for Subcatchment R2: To CB-2

Runoff = 1.90 cfs @ 12.09 hrs, Volume= 5,928 cf, Depth> 3.19" Routed to Pond CB2 :

A	rea (sf)	CN I	Description					
	8,993	98	Paved park	ing, HSG B	3			
	8,353	61 :	>75% Gras	s cover, Go	ood, HSG B			
	4,923	98	Roofs, HSC	βB				
	22,269	84	Weighted Average					
	8,353	4	37.51% Pei	vious Area	a			
	13,916	(62.49% Imp	pervious Are	rea			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)		(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R3: To RGB

Runoff = 0.94 cfs @ 12.09 hrs, Volume= 2,970 cf, Depth> 3.49" Routed to Reach SW3 :

Α	rea (sf)	CN	Description							
	5,386	98	Paved parking, HSG B							
	2,948	61	>75% Gras	s cover, Go	ood, HSG B					
	1,883	98	Roofs, HSC	B						
	10,217	87	Weighted Average							
	2,948		28.85% Pei	vious Area	а					
	7,269		71.15% Imp	pervious Ar	rea					
_										
ŢĊ	Length	Slope		Capacity	1					
(min)	(feet)	(ft/ft)	t) (ft/sec) (cfs)							
6.0					Direct Entry,					

Summary for Subcatchment R4: To CB-4

Runoff = 1.34 cfs @ 12.09 hrs, Volume= 4,205 cf, Depth> 3.39" Routed to Pond CB4 :

A	rea (sf)	CN	Description							
	7,280	98	Paved parking, HSG B							
	4,998	61	>75% Gras	s cover, Go	ood, HSG B					
	2,609	98	Roofs, HSC	βB						
	14,887	86	Weighted Average							
	4,998		33.57% Pei	vious Area	a					
	9,889		66.43% Imp	pervious Ar	rea					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	1					
6.0					Direct Entry,					

Summary for Subcatchment R5: To Foxhole

Runoff = 2.25 cfs @ 12.14 hrs, Volume= 7,976 cf, Depth> 2.82" Routed to Pond IB1 : Infiltration Basin #1

Α	rea (sf)	CN [Description							
	11,765	98 F	98 Paved parking, HSG B							
	16,262	61 >	>75% Ġras	s cover, Go	bod, HSG B					
	5,940	98 F	Roofs, HSC	βB						
	33,967	80 \	Neighted A	verage						
	16,262	2	17.88% Pei	vious Area						
	17,705	5	52.12% Imp	pervious Ar	ea					
Tc	Length	Slope		Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
5.6	50	0.0200	0.15		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.27"					
0.5	68	0.0150	2.49		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
3.9	187	0.0130	0.80		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
0.1	22	0.0300	3.52		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
10.1	327	Total								

Summary for Subcatchment R6: To CB-6

Runoff = 1.41 cfs @ 12.22 hrs, Volume= 5,906 cf, Depth> 2.72" Routed to Pond CB6 :

Α	rea (sf)	CN E	Description						
	7,642			ing, HSG B					
	13,667	61 >	75% Gras	s cover, Go	ood, HSG B				
	4,707	98 F	Roofs, HSG	6 B					
	26,016	79 V	Veighted A	verage					
	13,667	5	2.53% Per	vious Area					
	12,349	4	7.47% Imp	pervious Are	ea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
11.9	50	0.0030	0.07		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.27"				
3.8	158	0.0100	0.70		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.3	40	0.0130	2.31		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
16.0	248	Total							

Summary for Subcatchment R7: To CB-5

Runoff = 1.74 cfs @ 12.17 hrs, Volume= 6,550 cf, Depth> 2.46" Routed to Pond CB5 :

	A	rea (sf)	CN [CN Description							
		7,161	98 F	Paved park	ing, HSG B	}					
		19,224	61 >	•75% Ġras	s cover, Go	bod, HSG B					
		5,516	98 F	Roofs, HSC	βB						
		31,901	76 V	Veighted A	verage						
		19,224	6	0.26% Pei	vious Area						
		12,677	3	89.74% Imp	pervious Ar	ea					
	Тс	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	6.1	50	0.0160	0.14		Sheet Flow,					
						Grass: Short n= 0.150 P2= 3.27"					
	5.7	226	0.0090	0.66		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.1	27	0.0500	4.54		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	11.9	303	Total								

Summary for Subcatchment R8: To RGB 2

Runoff = 1.51 cfs @ 12.09 hrs, Volume= 4,725 cf, Depth> 3.29" Routed to Pond HW2 :

A	rea (sf)	CN I	Description					
	6,729	98	Paved park	ing, HSG B	3			
	6,213	61 :	>75% Gras	s cover, Go	ood, HSG B			
	4,288	98	Roofs, HSG	БВ				
	17,230	85	Neighted A	verage				
	6,213	4	36.06% Per	vious Area	a			
	11,017	(63.94% Imp	pervious Are	rea			
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R9: To RGB 1

Runoff = 0.76 cfs @ 12.08 hrs, Volume= 2,548 cf, Depth> 4.34" Routed to Reach SW8 :

Α	rea (sf)	CN	Description					
	4,963		Paved park					
	630	61	>75% Gras	s cover, Go	od, HSG B			
	1,458	98	Roofs, HSC	βB				
	7,051	95	Weighted A	verage				
	630		8.93% Perv	ious Area				
	6,421		91.07% Impervious Area					
т.	المربع مرالم	01.000	Mala alta	0	Description			
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment S3: To Swale

Runoff = 0.55 cfs @ 12.09 hrs, Volume= 1,725 cf, Depth> 2.05" Routed to Reach SW3 :

A	rea (sf)	CN [Description				
	2,611	98 F	Paved park	ing, HSG B	В		
	7,466	61 >	>75% Gras	s cover, Go	ood, HSG B		
	10,077	71 \	Veighted A	verage			
	7,466	7	74.09% Per	vious Area	a		
	2,611	2	25.91% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)			
6.0					Direct Entry,		

Summary for Subcatchment S4: To Swale

Runoff = 0.08 cfs @ 12.09 hrs, Volume= 239 cf, Depth> 2.13" Routed to Pond CB7 :

A	rea (sf)	CN	Description					
	392	98	Paved park	ing, HSG B	3			
	954	61	>75% Gras	s cover, Go	bod, HSG B			
	1,346	72	Weighted Average					
	954		70.88% Per	vious Area	3			
	392		29.12% Imp	pervious Are	ea			
Тс	Length	Slope		Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment S8: To Swale

Runoff = 0.32 cfs @ 12.09 hrs, Volume= 1,003 cf, Depth> 2.30" Routed to Reach SW8 :

ea (sf)	CN I	Description				
1,788	98 I	Paved park	ing, HSG B	3		
3,451	61 >	>75% Ġras	s cover, Go	bod, HSG B		
5,239	74 \	Weighted Average				
3,451	6	65.87% Per	vious Area	l		
1,788	:	84.13% Imp	pervious Ar	ea		
Length (feet)		Velocity (ft/sec)	Capacity (cfs)	Description		
				Direct Entry,		
	1,788 3,451 5,239 3,451 1,788 Length	1,788 98 F 3,451 61 > 5,239 74 V 3,451 6 1,788 3	1,788 98 Paved park 3,451 61 >75% Gras 5,239 74 Weighted A 3,451 65.87% Per 1,788 34.13% Imp Length Slope Velocity	1,78898Paved parking, HSG E3,45161>75% Grass cover, Go5,23974Weighted Average3,45165.87% Pervious Area1,78834.13% Impervious ArLengthSlopeVelocityCapacity		

Summary for Reach SW3:

Inflow Area = 20,294 sf, 48.68% Impervious, Inflow Depth > 2.78" for 10-yr event Inflow = 1.49 cfs @ 12.09 hrs, Volume= 4.695 cf 1.23 cfs @ 12.14 hrs, Volume= Outflow = 4,670 cf, Atten= 18%, Lag= 3.4 min Routed to Pond CB7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 1.39 fps, Min. Travel Time= 6.0 min Avg. Velocity = 0.47 fps, Avg. Travel Time= 17.9 min Peak Storage= 441 cf @ 12.14 hrs Average Depth at Peak Storage= 0.35', Surface Width= 3.58' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 28.59 cfs 1.50' x 1.50' deep channel, n= 0.041 Riprap, 2-inch Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 501.0' Slope= 0.0100 '/' Inlet Invert= 256.12', Outlet Invert= 251.10'

Summary for Reach SW8:

Inflow Area = 12,290 sf, 66.79% Impervious, Inflow Depth > 3.47" for 10-yr event Inflow = 1.08 cfs @ 12.09 hrs, Volume= 3.551 cf 0.74 cfs @ 12.17 hrs, Volume= Outflow = 3,524 cf, Atten= 32%, Lag= 5.1 min Routed to Pond HW2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 0.34 fps, Min. Travel Time= 11.5 min Avg. Velocity = 0.12 fps, Avg. Travel Time= 31.7 min Peak Storage= 508 cf @ 12.17 hrs Average Depth at Peak Storage= 0.64', Surface Width= 5.34' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 4.93 cfs 1.50' x 1.50' deep channel, n= 0.240 Sheet flow over Dense Grass Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 232.0' Slope= 0.0102 '/' Inlet Invert= 255.37', Outlet Invert= 253.00'

Summary for Pond AD:

Inflow Area = 2,664 sf, 0.00% Impervious, Inflow Depth > 1.32" for 10-yr event 0.09 cfs @ 12.10 hrs. Volume= Inflow = 293 cf 0.09 cfs @ 12.10 hrs, Volume= Outflow = 293 cf, Atten= 0%, Lag= 0.0 min 0.09 cfs @ 12.10 hrs, Volume= Primary = 293 cf Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.81' @ 12.09 hrs Flood Elev= 250.75' Device Routing Invert Outlet Devices #1 Primary 248.60' 8.0" Round Culvert L= 26.4' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.60' / 248.47' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.35 sf Primary OutFlow Max=0.09 cfs @ 12.10 hrs HW=248.81' TW=248.72' (Dynamic Tailwater)

T=Culvert(Outlet Controls 0.09 cfs @ 1.32 fps)

Summary for Pond CB1: CB#1

Inflow Area = 11,152 sf, 70.59% Impervious, Inflow Depth > 3.49" for 10-yr event Inflow = 1.03 cfs @ 12.09 hrs, Volume= 3.242 cf 1.03 cfs @ 12.09 hrs, Volume= Outflow = 3,242 cf, Atten= 0%, Lag= 0.0 min 1.03 cfs @ 12.09 hrs, Volume= Primary = 3,242 cf Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.57' @ 12.09 hrs Flood Elev= 251.50' Device Routing Invert Outlet Devices #1 249.00' Primary 12.0" Round Culvert L= 93.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.00' / 248.53' S= 0.0050 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=1.02 cfs @ 12.09 hrs HW=249.57' TW=248.72' (Dynamic Tailwater)

1=Culvert (Barrel Controls 1.02 cfs @ 3.18 fps)

Summary for Pond CB2:

Inflow Area = 22,269 sf, 62.49% Impervious, Inflow Depth > 3.19" for 10-yr event Inflow = 1.90 cfs @ 12.09 hrs, Volume= 5.928 cf 1.90 cfs @ 12.09 hrs, Volume= Outflow = 5,928 cf, Atten= 0%, Lag= 0.0 min 1.90 cfs @ 12.09 hrs, Volume= Primary = 5,928 cf Routed to Pond SIS2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.96' @ 12.09 hrs Flood Elev= 254.02' Device Routing Invert Outlet Devices #1 251.20' Primary 12.0" Round Culvert L= 73.9' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.20' / 250.46' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=1.89 cfs @ 12.09 hrs HW=251.96' TW=249.82' (Dynamic Tailwater)

1=Culvert (Inlet Controls 1.89 cfs @ 2.96 fps)

Summary for Pond CB4:

Inflow Area = Inflow = Outflow = Primary = Routed to Pond	14,887 sf, 66.43% Impervious, Inflow Depth > 3.39" for 10-yr event 1.34 cfs @ 12.09 hrs, Volume= 4,205 cf 1.34 cfs @ 12.09 hrs, Volume= 4,205 cf, Atten= 0%, Lag= 0.0 min 1.34 cfs @ 12.09 hrs, Volume= 4,205 cf, Atten= 0%, Lag= 0.0 min 1.34 cfs @ 12.09 hrs, Volume= 4,205 cf 1.34 cfs @ 12.09 hrs, Volume= 4,205 cf						
Peak Elev= 248.44	Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.44' @ 12.09 hrs Flood Elev= 250.69'						
Device Routing	Invert Outlet Devices						
#1Primary247.83' 12.0'' Round Culvert L= 14.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.83' / 247.55' S= 0.0193 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf							
Primary OutFlow Max=1.33 cfs @ 12.09 hrs HW=248.44' TW=247.54' (Dynamic Tailwater) ☐1=Culvert (Inlet Controls 1.33 cfs @ 2.66 fps)							

Summary for Pond CB5:

Inflow Area = 31,901 sf, 39.74% Impervious, Inflow Depth > 2.46" for 10-yr event Inflow = 1.74 cfs @ 12.17 hrs, Volume= 6.550 cf 1.74 cfs @ 12.17 hrs, Volume= Outflow = 6,550 cf, Atten= 0%, Lag= 0.0 min 1.74 cfs @ 12.17 hrs, Volume= Primary = 6,550 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.12' @ 12.17 hrs Flood Elev= 252.45' Device Routing Invert Outlet Devices #1 250.30' Primary 12.0" Round Culvert L= 8.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 250.30' / 250.20' S= 0.0118 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.73 cfs @ 12.17 hrs HW=251.12' TW=250.60' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 1.73 cfs @ 3.43 fps)

Summary for Pond CB6:

Inflow Area = 26,016 sf, 47.47% Impervious, Inflow Depth > 2.72" for 10-yr event 1.41 cfs @ 12.22 hrs. Volume= Inflow = 5.906 cf 1.41 cfs @ 12.22 hrs, Volume= Outflow = 5,906 cf, Atten= 0%, Lag= 0.0 min 1.41 cfs @ 12.22 hrs, Volume= Primary = 5,906 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.07' @ 12.22 hrs Flood Elev= 250.82' Device Routing Invert Outlet Devices #1 248.30' Primary 12.0" Round Culvert L= 6.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.30' / 248.26' S= 0.0063 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=1.41 cfs @ 12.22 hrs HW=249.07' TW=248.57' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 1.41 cfs @ 3.03 fps)

Summary for Pond CB7:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 2.72" for 10-yr event 1.29 cfs @ 12.14 hrs. Volume= Inflow = 4.909 cf 1.29 cfs @ 12.14 hrs, Volume= Outflow = 4,909 cf, Atten= 0%, Lag= 0.0 min 1.29 cfs @ 12.14 hrs, Volume= Primary = 4,909 cf Routed to Pond DMH3 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.57' @ 12.15 hrs Flood Elev= 253.00' Device Routing **Outlet Devices** Invert #1 248.00' Primary 18.0" Round Culvert L= 88.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.00' / 247.50' S= 0.0057 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf Primary OutFlow Max=1.27 cfs @ 12.14 hrs HW=248.57' TW=248.04' (Dynamic Tailwater)

1=Culvert (Outlet Controls 1.27 cfs @ 3.09 fps)

Summary for Pond DMH1:

Outflow Primary	= = =	1.11 cfs @ 12 1.11 cfs @ 12	6.98% Impervious, Inflow Depth > 3.07" for 10-yr event 2.09 hrs, Volume= 3,535 cf 2.09 hrs, Volume= 3,535 cf, Atten= 0%, Lag= 0.0 min 2.09 hrs, Volume= 3,535 cf ch				
Peak Ĕl Flood E	Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.72' @ 12.09 hrs Flood Elev= 252.50'						
Device	Routing	Invert	Outlet Devices				
#1	Primary Primary		12.0" Round MANIFOLD L= 6.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.40' / 248.35' S= 0.0083 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.79 sf 24.0" Round ISOLATOR				
	ŗ		L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.28' / 248.25' S= 0.0075 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf 12.00 brs HW=248.72' TW=248.23' (Dynamic Tailwater)				

Primary OutFlow Max=1.11 cfs @ 12.09 hrs HW=248.72' TW=248.33' (Dynamic Tailwater) **1=MANIFOLD** (Barrel Controls 0.31 cfs @ 2.19 fps)

2=ISOLATOR (Barrel Controls 0.79 cfs @ 2.37 fps)

Summary for Pond DMH3:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 2.72" for 10-yr event 1.29 cfs @ 12.14 hrs, Volume= Inflow = 4.909 cf 1.29 cfs @ 12.14 hrs, Volume= 4,909 cf, Atten= 0%, Lag= 0.0 min Outflow = 1.29 cfs @ 12.14 hrs, Volume= Primary = 4,909 cf Routed to Pond DMH4 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.05' @ 12.42 hrs Flood Elev= 251.98' Device Routing Invert Outlet Devices #1 247.45' Primary 18.0" Round Culvert L= 68.2' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.45' / 247.10' S= 0.0051 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf Primary OutFlow Max=1.29 cfs @ 12.14 hrs HW=248.04' TW=247.64' (Dynamic Tailwater)

1=Culvert (Outlet Controls 1.29 cfs @ 2.93 fps)

Summary for Pond DMH4:

Inflow Area = 36,527 sf, 55.19% Impervious, Inflow Depth > 2.99" for 10-yr event Inflow = 2.52 cfs @ 12.11 hrs, Volume= 9.114 cf 2.52 cfs @ 12.11 hrs, Volume= Outflow = 9,114 cf, Atten= 0%, Lag= 0.0 min 2.52 cfs @ 12.11 hrs, Volume= Primary = 9,114 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.99' @ 12.52 hrs Flood Elev= 251.00' Device Routing Invert Outlet Devices #1 Primary 246.60' 24.0" Round Culvert L= 34.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 246.60' / 246.43' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf Primary OutFlow Max=2.51 cfs @ 12.11 hrs HW=247.59' TW=247.42' (Dynamic Tailwater)

1=Culvert (Outlet Controls 2.51 cfs @ 2.38 fps)

Summary for Pond DMH5:

Inflow Area = 29,520 sf, 65.13% Impervious, Inflow Depth > 3.35" for 10-yr event Inflow = 2.13 cfs @ 12.10 hrs, Volume= 8.249 cf 2.13 cfs @ 12.10 hrs, Volume= Outflow = 8,249 cf, Atten= 0%, Lag= 0.0 min 2.13 cfs @ 12.10 hrs, Volume= Primary = 8,249 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 252.37' @ 12.10 hrs Flood Elev= 254.00' Device Routing Invert Outlet Devices #1 251.65' Primary 15.0" Round Culvert L= 116.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.65' / 250.46' S= 0.0102 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.23 sf

Primary OutFlow Max=2.13 cfs @ 12.10 hrs HW=252.37' TW=250.61' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.13 cfs @ 2.89 fps)

Summary for Pond DMH6:

Inflow Area = 61,421 sf, 51.94% Impervious, Inflow Depth > 2.89" for 10-yr event Inflow = 3.69 cfs @ 12.13 hrs, Volume= 14.799 cf 3.69 cfs @ 12.13 hrs, Volume= 14,799 cf, Atten= 0%, Lag= 0.0 min Outflow = 3.69 cfs @ 12.13 hrs, Volume= Primary = 14,799 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.63' @ 12.13 hrs Flood Elev= 252.93' Device Routing Invert Outlet Devices #1 249.71' Primary 18.0" Round Culvert L= 160.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.71' / 248.10' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=3.68 cfs @ 12.13 hrs HW=250.62' TW=248.54' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.68 cfs @ 3.26 fps)

Summary for Pond DMH7:

Inflow Area = 87,437 sf, 50.61% Impervious, Inflow Depth > 2.84" for 10-yr event Inflow = 4.88 cfs @ 12.15 hrs, Volume= 20.705 cf 4.88 cfs @ 12.15 hrs, Volume= 20,705 cf, Atten= 0%, Lag= 0.0 min Outflow = 4.88 cfs @ 12.15 hrs, Volume= Primary = 20,705 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.57' @ 12.19 hrs Flood Elev= 251.25' Device Routing Invert Outlet Devices #1 247.60' Primary 24.0" Round Culvert L= 111.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.60' / 246.65' S= 0.0085 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf Primary OutFlow Max=4.87 cfs @ 12.15 hrs HW=248.56' TW=247.54' (Dynamic Tailwater)

1=Culvert (Outlet Controls 4.87 cfs @ 4.80 fps)

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	26,605 sf,	5.66% Impervious,	Inflow Depth > 1.45" for 10-yr event
Inflow =	0.79 cfs @	12.18 hrs, Volume=	3,223 cf
Outflow =	0.75 cfs @	12.23 hrs, Volume=	3,204 cf, Atten= 6%, Lag= 3.0 min
Discarded =	0.10 cfs @	12.23 hrs, Volume=	2,270 cf
Primary =	0.64 cfs @	12.23 hrs, Volume=	934 cf
Routed to Link	AP3 : Abutter	Depression	

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.30' @ 12.23 hrs Surf.Area= 1,713 sf Storage= 407 cf

Plug-Flow detention time= 34.9 min calculated for 3,202 cf (99% of inflow) Center-of-Mass det. time= 31.6 min (902.7 - 871.1)

Volume	Invert	Avail.Sto	rage Stora	age Description		
#1	249.80'	1,12	22 cf Cust	om Stage Data (P	rismatic)Listed belov	v (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4	30 90 20	ırf.Area (sq-ft) 164 562 1,261 2,144	Inc.Store (cubic-feet) 0 73 182 341	(cubic-feet) 0 73 255		
250.6	60	3,127	527	1,122		
Device	Routing	Invert	Outlet Dev	ices		
#1	Discarded	249.80'	-	r Exfiltration over		
#2	Primary	250.25'	Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01' 20.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63			

Discarded OutFlow Max=0.10 cfs @ 12.23 hrs HW=250.30' (Free Discharge) **1=Exfiltration** (Controls 0.10 cfs)

Primary OutFlow Max=0.64 cfs @ 12.23 hrs HW=250.30' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.64 cfs @ 0.61 fps)

Summary for Pond HW2:

[62] Hint: Exceeded Reach SW8 OUTLET depth by 0.15' @ 12.06 hrs

 Inflow Area =
 29,520 sf, 65.13% Impervious, Inflow Depth > 3.35" for 10-yr event

 Inflow =
 2.13 cfs @
 12.10 hrs, Volume=
 8,249 cf

 Outflow =
 2.13 cfs @
 12.10 hrs, Volume=
 8,249 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 2.13 cfs @
 12.10 hrs, Volume=
 8,249 cf, Atten= 0%, Lag= 0.0 min

 Routed to Pond DMH5 :
 12.10 hrs, Volume=
 8,249 cf

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 253.72' @ 12.10 hrs Flood Elev= 254.00'

Device	Routing	Invert	Outlet Devices				
#1	Primary	253.00'	15.0" Round Culvert L= 14.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 253.00' / 251.75' S= 0.0856 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf				
D eine and O (F) and M (a) 40 afr (O 40 km (b) W (-050 70) T) W (-050 07) (D) m and T (b) m the state m)							

Primary OutFlow Max=2.13 cfs @ 12.10 hrs HW=253.72' TW=252.37' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.13 cfs @ 2.89 fps)

Summary for Pond IB1: Infiltration Basin #1

[80] Warning: Exceeded Pond DMH4 by 0.31' @ 2.00 hrs (0.76 cfs 1,415 cf)

Inflow Area =	200,508 sf,	, 48.93% Impervious,	Inflow Depth > 2.38" for 10-yr event				
Inflow =	10.09 cfs @	12.13 hrs, Volume=	39,799 cf				
Outflow =	3.56 cfs @	12.53 hrs, Volume=	39,791 cf, Atten= 65%, Lag= 24.3 min				
Discarded =	2.75 cfs @	12.53 hrs, Volume=	35,901 cf				
Primary =	0.81 cfs @	12.53 hrs, Volume=	3,890 cf				
Routed to Link	AP1 : To Wet	lands					
Secondary =	0.00 cfs @	2.00 hrs, Volume=	0 cf				
Routed to Link AP1 : To Wetlands							

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 247.98' @ 12.53 hrs Surf.Area= 10,043 sf Storage= 9,094 cf

Plug-Flow detention time= 18.0 min calculated for 39,755 cf (100% of inflow) Center-of-Mass det. time= 17.8 min (842.4 - 824.6)

Volume	Invert	Avail.Sto	orage	Storage Description			
#1	247.00'	32,7	'58 cf	Custom Stage Data	a (Irregular)Listed	below (Recalc)	
Elevatio (fee		rf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
247.0 248.0	00	10,075	373.0 398.2	0 9,293	0 9,293	8,532 10,126	
249.0 250.0		,	423.3 448.4	10,886 12,579	20,179 32,758	11,817 13,612	
Device	Routing	Invert	Outle	et Devices			
#1	Discarded	247.00'	-	0 in/hr Exfiltration of		1.02' Dhace In= 0.01'	
#2	Primary	247.00'	18.0 Inlet	Conductivity to Groundwater Elevation = 244.93' Phase-In= 0.0 18.0" Round Culvert L= 62.2' Ke= 0.500 Inlet / Outlet Invert= 247.00' / 246.00' S= 0.0161 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf			
#3	Device 2	247.00'	6.0"	Vert. Orifice/Grate	C= 0.600 Limited	I to weir flow at low heads	
#4	Device 2	248.20'	-	x 2.0" Horiz. Orifice rows C= 0.600 in 24.0		-	
#5	Secondary	249.00'	Limit 10.0 Head	ted to weir flow at low ' long x 10.0' breadt d (feet) 0.20 0.40 0. f. (English) 2.49 2.56	Rectangular Weir 0 1.40 1.60		

Discarded OutFlow Max=2.75 cfs @ 12.53 hrs HW=247.98' (Free Discharge) **1=Exfiltration** (Controls 2.75 cfs)

Primary OutFlow Max=0.81 cfs @ 12.53 hrs HW=247.98' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 0.81 cfs of 4.12 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.81 cfs @ 4.11 fps) 4=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.00' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond SIS1: Stormtech

Inflow Area = 13,816 sf, 56.98% Impervious, Inflow Depth > 3.07" for 10-yr event Inflow = 1.11 cfs @ 12.09 hrs, Volume= 3.535 cf 0.34 cfs @ 12.42 hrs, Volume= Outflow = 3,535 cf, Atten= 70%, Lag= 20.2 min 0.34 cfs @ 12.42 hrs, Volume= Discarded = 3.535 cf 0.00 cfs @ 2.00 hrs, Volume= Secondary = 0 cf Routed to Link AP1 : To Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.67' @ 12.42 hrs Surf.Area= 1,242 sf Storage= 681 cf

Plug-Flow detention time= 10.9 min calculated for 3,535 cf (100% of inflow) Center-of-Mass det. time= 10.9 min (818.0 - 807.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	247.75'	1,151 cf	20.50'W x 60.58'L x 3.50'H Field A
			4,346 cf Overall - 1,470 cf Embedded = 2,876 cf x 40.0% Voids
#2A	248.25'	1,470 cf	ADS_StormTech SC-740 +Cap x 32 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			32 Chambers in 4 Rows
#3	248.00'	35 cf	4.00'D x 2.75'H Vertical Cone/Cylinder-Impervious
		2,655 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	247.75'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 245.51' Phase-In= 0.01'
#2	Secondary	250.60'	2.0" x 2.0" Horiz. Orifice/Grate X 7.00 columns
			X 7 rows C= 0.600 in 24.0" x 24.0" Grate (34% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.34 cfs @ 12.42 hrs HW=248.67' (Free Discharge) **1=Exfiltration** (Controls 0.34 cfs)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.75' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Pond SIS1: Stormtech - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 58.58' Row Length +12.0" End Stone x 2 = 60.58' Base Length 4 Rows x 51.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.50' Base Width

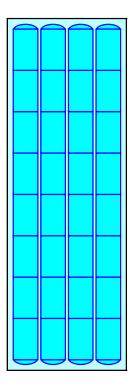
6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

32 Chambers x 45.9 cf = 1,470.1 cf Chamber Storage

4,346.4 cf Field - 1,470.1 cf Chambers = 2,876.3 cf Stone x 40.0% Voids = 1,150.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,620.6 cf = 0.060 afOverall Storage Efficiency = 60.3%Overall System Size = $60.58' \times 20.50' \times 3.50'$

32 Chambers 161.0 cy Field 106.5 cy Stone





Summary for Pond SIS2:

Inflow Area =	24,351 sf, 65.	.70% Impervious,	Inflow Depth > 3.32"	for 10-yr event
Inflow =	2.13 cfs @ 12.0	09 hrs, Volume=	6,738 cf	
Outflow =	0.56 cfs @ 12.4	46 hrs, Volume=	6,737 cf, Atter	n= 74%, Lag= 22.4 min
Discarded =	0.56 cfs @ 12.4	16 hrs, Volume=	6,737 cf	
Primary =	0.00 cfs @ 2.0	00 hrs, Volume=	0 cf	
Routed to Pond	I IB1 : Infiltration B	Basin #1		

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.38' @ 12.46 hrs Surf.Area= 1,735 sf Storage= 1,561 cf

Plug-Flow detention time= 16.9 min calculated for 6,737 cf (100% of inflow) Center-of-Mass det. time= 16.9 min (820.1 - 803.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	249.00'	1,603 cf	44.25'W x 39.22'L x 3.50'H Field A
			6,074 cf Overall - 2,067 cf Embedded = 4,006 cf x 40.0% Voids
#2A	249.50'	2,067 cf	ADS_StormTech SC-740 +Cap x 45 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			45 Chambers in 9 Rows
#3	250.80'	38 cf	4.00'D x 3.00'H Vertical Cone/Cylinder
		3,708 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	249.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01'
#2	Primary	253.50'	2.0" x 2.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 24.0" x 24.0" Grate (25% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.56 cfs @ 12.46 hrs HW=250.38' (Free Discharge) **1=Exfiltration** (Controls 0.56 cfs)

Primary OutFlow Max=0.00 cfs @ 2.00 hrs HW=249.00' TW=247.00' (Dynamic Tailwater)

Pond SIS2: - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

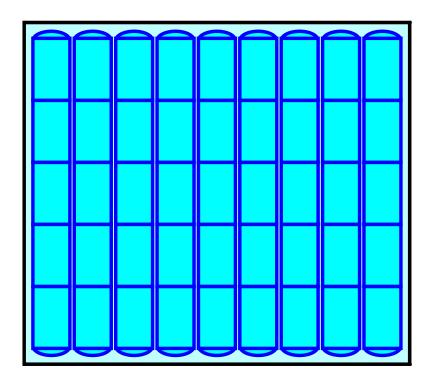
5 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 37.22' Row Length +12.0" End Stone x 2 = 39.22' Base Length 9 Rows x 51.0" Wide + 6.0" Spacing x 8 + 12.0" Side Stone x 2 = 44.25' Base Width 6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

45 Chambers x 45.9 cf = 2,067.3 cf Chamber Storage

6,073.7 cf Field - 2,067.3 cf Chambers = 4,006.4 cf Stone x 40.0% Voids = 1,602.6 cf Stone Storage

Chamber Storage + Stone Storage = 3,669.9 cf = 0.084 afOverall Storage Efficiency = 60.4%Overall System Size = $39.22' \times 44.25' \times 3.50'$

45 Chambers 225.0 cy Field 148.4 cy Stone





Summary for Link AP1: To Wetlands

Inflow Are	a =	236,171 sf, 42.45% Impervious,	Inflow Depth > 0.41" for 10-yr event
Inflow	=	1.61 cfs @ 12.12 hrs, Volume=	8,054 cf
Primary	=	1.61 cfs @ 12.12 hrs, Volume=	8,054 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP2: To Offsite

Inflow Are	a =	36,823 sf,	7.73% Impervious,	Inflow Depth > 1	.46" for 10-yr event
Inflow	=	1.34 cfs @	12.10 hrs, Volume=	4,469 cf	
Primary	=	1.34 cfs @	12.10 hrs, Volume=	4,469 cf,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP3: Abutter Depression

Inflow Area	a =	26,605 sf,	5.66% Impervious,	Inflow Depth = 0.42"	for 10-yr event
Inflow	=	0.64 cfs @ 1	12.23 hrs, Volume=	934 cf	
Primary	=	0.64 cfs @ 1	12.23 hrs, Volume=	934 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Time span=2.00-24.00 hrs, dt=0.02 hrs, 1101 points x 3 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment A1: To Area Drain	Runoff Area=2,664 sf 0.00% Impervious Runoff Depth>3.01" Tc=6.0 min CN=61 Runoff=0.21 cfs 667 cf
Subcatchment A2: To Exterior	Runoff Area=1,761 sf 0.00% Impervious Runoff Depth>3.01" Tc=6.0 min CN=61 Runoff=0.14 cfs 441 cf
Subcatchment A3: To Exterior	Runoff Area=3,301 sf 50.05% Impervious Runoff Depth>5.08" Tc=6.0 min CN=80 Runoff=0.45 cfs 1,397 cf
Subcatchment A4: To Roof Drain	Runoff Area=2,082 sf 100.00% Impervious Runoff Depth>7.14" Tc=6.0 min CN=98 Runoff=0.35 cfs 1,239 cf
SubcatchmentA5: To Abutter	Runoff Area=26,605 sf 5.66% Impervious Runoff Depth>3.21" Flow Length=212' Tc=11.8 min CN=63 Runoff=1.88 cfs 7,117 cf
SubcatchmentA6: To Abutter	Runoff Area=36,823 sf 7.73% Impervious Runoff Depth>3.21" Tc=6.0 min CN=63 Runoff=3.15 cfs 9,864 cf
SubcatchmentA7: To Infiltration Basin	Runoff Area=18,226 sf 0.00% Impervious Runoff Depth>3.01" Tc=6.0 min CN=61 Runoff=1.45 cfs 4,564 cf
SubcatchmentA8: To Exterior	Runoff Area=30,601 sf 1.56% Impervious Runoff Depth>2.90" Tc=6.0 min CN=60 Runoff=2.33 cfs 7,398 cf
Subcatchment R1: To CB#1	Runoff Area=11,152 sf 70.59% Impervious Runoff Depth>5.88" Tc=6.0 min CN=87 Runoff=1.69 cfs 5,465 cf
Subcatchment R2: To CB-2	Runoff Area=22,269 sf 62.49% Impervious Runoff Depth>5.53" Tc=6.0 min CN=84 Runoff=3.23 cfs 10,271 cf
Subcatchment R3: To RGB	Runoff Area=10,217 sf 71.15% Impervious Runoff Depth>5.88" Tc=6.0 min CN=87 Runoff=1.55 cfs 5,007 cf
Subcatchment R4: To CB-4	Runoff Area=14,887 sf 66.43% Impervious Runoff Depth>5.77" Tc=6.0 min CN=86 Runoff=2.23 cfs 7,152 cf
Subcatchment R5: To Foxhole	Runoff Area=33,967 sf 52.12% Impervious Runoff Depth>5.07" Flow Length=327' Tc=10.1 min CN=80 Runoff=4.00 cfs 14,365 cf
Subcatchment R6: To CB-6	Runoff Area=26,016 sf 47.47% Impervious Runoff Depth>4.96" Flow Length=248' Tc=16.0 min CN=79 Runoff=2.55 cfs 10,745 cf
Subcatchment R7: To CB-5	Runoff Area=31,901 sf 39.74% Impervious Runoff Depth>4.62" Flow Length=303' Tc=11.9 min CN=76 Runoff=3.27 cfs 12,290 cf
Subcatchment R8: To RGB 2	Runoff Area=17,230 sf 63.94% Impervious Runoff Depth>5.65" Tc=6.0 min CN=85 Runoff=2.54 cfs 8,112 cf

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Subcatchment R9: To RGB	Runoff Area=7,051 sf 91.07% Impervious Runoff Depth>6.82" Tc=6.0 min CN=95 Runoff=1.16 cfs 4,005 cf
SubcatchmentS3: To Swale	Runoff Area=10,077 sf 25.91% Impervious Runoff Depth>4.08" Tc=6.0 min CN=71 Runoff=1.11 cfs 3,422 cf
SubcatchmentS4: To Swale	Runoff Area=1,346 sf 29.12% Impervious Runoff Depth>4.18" Tc=6.0 min CN=72 Runoff=0.15 cfs 469 cf
SubcatchmentS8: To Swale	Runoff Area=5,239 sf 34.13% Impervious Runoff Depth>4.41" Tc=6.0 min CN=74 Runoff=0.62 cfs 1,923 cf
Reach SW3:	Avg. Flow Depth=0.47' Max Vel=1.65 fps Inflow=2.65 cfs 8,429 cf n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=2.26 cfs 8,395 cf
Reach SW8:	Avg. Flow Depth=0.83' Max Vel=0.39 fps Inflow=1.78 cfs 5,929 cf n=0.240 L=232.0' S=0.0102 '/' Capacity=4.93 cfs Outflow=1.28 cfs 5,892 cf
Pond AD:	Peak Elev=249.63' Inflow=0.21 cfs 667 cf 8.0" Round Culvert n=0.011 L=26.4' S=0.0049 '/' Outflow=0.21 cfs 667 cf
Pond CB1: CB#1	Peak Elev=249.78' Inflow=1.69 cfs 5,465 cf 12.0" Round Culvert n=0.011 L=93.3' S=0.0050 '/' Outflow=1.69 cfs 5,465 cf
Pond CB2:	Peak Elev=252.43' Inflow=3.23 cfs 10,271 cf 12.0" Round Culvert n=0.011 L=73.9' S=0.0100 '/' Outflow=3.23 cfs 10,271 cf
Pond CB4:	Peak Elev=248.68' Inflow=2.23 cfs 7,152 cf 12.0" Round Culvert n=0.011 L=14.5' S=0.0193 '/' Outflow=2.23 cfs 7,152 cf
Pond CB5:	Peak Elev=251.77' Inflow=3.27 cfs 12,290 cf 12.0" Round Culvert n=0.011 L=8.5' S=0.0118 '/' Outflow=3.27 cfs 12,290 cf
Pond CB6:	Peak Elev=249.59' Inflow=2.55 cfs 10,745 cf 12.0" Round Culvert n=0.011 L=6.3' S=0.0063 '/' Outflow=2.55 cfs 10,745 cf
Pond CB7:	Peak Elev=248.89' Inflow=2.39 cfs 8,865 cf 18.0" Round Culvert n=0.011 L=88.3' S=0.0057 '/' Outflow=2.39 cfs 8,865 cf
Pond DMH1:	Peak Elev=249.63' Inflow=1.90 cfs 6,132 cf Outflow=1.90 cfs 6,132 cf
Pond DMH3:	Peak Elev=248.66' Inflow=2.39 cfs 8,865 cf 18.0" Round Culvert n=0.011 L=68.2' S=0.0051 '/' Outflow=2.39 cfs 8,865 cf
Pond DMH4:	Peak Elev=248.61' Inflow=4.47 cfs 16,017 cf 24.0" Round Culvert n=0.011 L=34.6' S=0.0049 '/' Outflow=4.47 cfs 16,017 cf
Pond DMH5:	Peak Elev=252.66' Inflow=3.63 cfs 14,004 cf 15.0" Round Culvert n=0.011 L=116.8' S=0.0102 '/' Outflow=3.63 cfs 14,004 cf
Pond DMH6:	Peak Elev=251.06' Inflow=6.60 cfs 26,294 cf 18.0" Round Culvert n=0.011 L=160.8' S=0.0100 '/' Outflow=6.60 cfs 26,294 cf

19227 - PostDevelopi Prepared by Howard Ste <u>HydroCAD® 10.10-6a s/n M</u>		Type III 24-hr 50-yr Rainfall=7.42" Printed 4/11/2022 LC Page 156
Pond DMH7:	Peak E 24.0" Round Culvert n=0.011 L=111.5' S	lev=249.14' Inflow=8.78 cfs 37,039 cf =0.0085 '/' Outflow=8.78 cfs 37,039 cf
Pond EX: Existing Abutte	Peak Elev=250.35' Store Discarded=0.12 cfs 3,324 cf Primary=1.73 c	orage=496 cf Inflow=1.88 cfs 7,117 cf fs 3,718 cf Outflow=1.85 cfs 7,042 cf
Pond HW2:	Peak E 15.0" Round Culvert n=0.013 L=14.6' S	lev=254.01' Inflow=3.63 cfs 14,004 cf =0.0856 '/' Outflow=3.63 cfs 14,004 cf
Pond IB1: Infiltration Bas Discarded=3.52 cfs 56,110 cf	in #1 Peak Elev=248.60' Storage Primary=6.48 cfs 15,862 cf Secondary=0.00	=15,583 cf Inflow=18.37 cfs 71,985 cf cfs 0 cf Outflow=10.00 cfs 71,973 cf
Pond SIS1: Stormtech	Peak Elev=249.63' Stor Discarded=0.44 cfs 6,132 cf Secondary=0.	age=1,588 cf Inflow=1.90 cfs 6,132 cf .00 cfs 0 cf Outflow=0.44 cfs 6,132 cf
Pond SIS2:	Peak Elev=251.84' Stora Discarded=0.81 cfs 11,510 cf Primary=0.0	ge=3,221 cf Inflow=3.57 cfs 11,511 cf 0 cfs 0 cf Outflow=0.81 cfs 11,510 cf
Link AP1: To Wetlands		Inflow=7.71 cfs 25,098 cf Primary=7.71 cfs 25,098 cf
Link AP2: To Offsite		Inflow=3.15 cfs 9,864 cf Primary=3.15 cfs 9,864 cf
Link AP3: Abutter Depres	ssion	Inflow=1.73 cfs 3,718 cf Primary=1.73 cfs 3,718 cf

Total Runoff Area = 313,415 sf Runoff Volume = 115,917 cf Average Runoff Depth = 4.44" 64.11% Pervious = 200,945 sf 35.89% Impervious = 112,470 sf

Summary for Subcatchment A1: To Area Drain

Runoff = 0.21 cfs @ 12.09 hrs, Volume= 667 cf, Depth> 3.01" Routed to Pond AD :

A	rea (sf)	CN E	escription					
	2,664	61 >	61 >75% Grass cover, Good, HSG B					
	2,664	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment A2: To Exterior

Runoff = 0.14 cfs @ 12.09 hrs, Volume= 441 cf, Depth> 3.01" Routed to Link AP1 : To Wetlands

A	rea (sf)	CN E	Description						
	1,761	61 >	>75% Grass cover, Good, HSG B						
	1,761	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment A3: To Exterior

Runoff = 0.45 cfs @ 12.09 hrs, Volume= 1,397 cf, Depth> 5.08" Routed to Link AP1 : To Wetlands

Description						
Weighted Average						
49.95% Pervious Area						
-						

Summary for Subcatchment A4: To Roof Drain

Runoff = 0.35 cfs @ 12.08 hrs, Volume= 1,239 cf, Depth> 7.14" Routed to Pond SIS2 :

Area (sf)	CN I	Description					
2,082	98 I	Roofs, HSG	ВВ				
2,082		100.00% Impervious Area					
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
6.0				Direct Entry,			

Summary for Subcatchment A5: To Abutter

Runoff = 1.88 cfs @ 12.17 hrs, Volume= 7,117 cf, Depth> 3.21" Routed to Pond EX : Existing Abutter Depression

Α	rea (sf)	CN E	N Description						
	23,897	61 >	75% Gras	s cover, Go	bod, HSG B				
	1,201	55 V	Voods, Go	od, HSG B					
	1,507	98 F	Roofs, HSG	6 B					
	26,605	63 V	Veighted A	verage					
	25,098	9	4.34% Per	vious Area					
	1,507	5	.66% Impe	ervious Area	a				
Тс	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.8	50	0.0120	0.12		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.27"				
5.0	162	0.0060	0.54		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
11.8	212	Total							

Summary for Subcatchment A6: To Abutter

Runoff = 3.15 cfs @ 12.09 hrs, Volume= 9,864 cf, Depth> 3.21" Routed to Link AP2 : To Offsite

Area (sf)	CN	Description						
1,281	98	Paved park	ing, HSG B	3				
25,992	61	>75% Gras	s cover, Go	bod, HSG B				
7,984	55	Woods, Go	od, HSG B					
1,566	98	Roofs, HSG	БВ					
36,823	63	Weighted A	Weighted Average					
33,976		92.27% Pervious Area						
2,847		7.73% Impe	ervious Are	a				
			- ··					
Tc Length			Capacity	Description				
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)					
6.0				Direct Entry,				

Summary for Subcatchment A7: To Infiltration Basin

Runoff = 1.45 cfs @ 12.09 hrs, Volume= 4,564 cf, Depth> 3.01" Routed to Pond IB1 : Infiltration Basin #1

Are	ea (sf)	CN E	N Description						
1	8,226	61 >	31 >75% Grass cover, Good, HSG B						
1	8,226	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment A8: To Exterior

Runoff = 2.33 cfs @ 12.09 hrs, Volume= 7,398 cf, Depth> 2.90" Routed to Link AP1 : To Wetlands

A	rea (sf)	CN	Description					
	476	98	Paved park	ing, HSG B	3			
	20,641	61	>75% Gras	s cover, Go	ood, HSG B			
	9,484	55	Woods, Go	od, HSG B				
	30,601	60	Weighted A	verage				
	30,125		98.44% Per	vious Area	3			
	476		1.56% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment R1: To CB#1

Runoff = 1.69 cfs @ 12.08 hrs, Volume= 5,465 cf, Depth> 5.88" Routed to Pond CB1 : CB#1

Α	rea (sf)	CN	Description					
	7,872	98	Paved park	ing, HSG B	3			
	3,280	61	>75% Gras	s cover, Go	ood, HSG B			
	11,152	87	Weighted A	verage				
	3,280		29.41% Per	vious Area	3			
	7,872		70.59% Imp	pervious Are	rea			
Тс	Length	Slope		Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R2: To CB-2

Runoff = 3.23 cfs @ 12.09 hrs, Volume= 10,271 cf, Depth> 5.53" Routed to Pond CB2 :

A	rea (sf)	CN I	Description					
	8,993	98	Paved park	ing, HSG B	3			
	8,353	61 :	>75% Gras	s cover, Go	ood, HSG B			
	4,923	98	Roofs, HSC	βB				
	22,269	84	Weighted Average					
	8,353	4	37.51% Pei	vious Area	a			
	13,916	(62.49% Imp	pervious Are	rea			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)		(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R3: To RGB

Runoff = 1.55 cfs @ 12.08 hrs, Volume= 5,007 cf, Depth> 5.88" Routed to Reach SW3 :

Α	rea (sf)	CN	Description					
	5,386	98	Paved park	ing, HSG B	В			
	2,948	61	>75% Gras	s cover, Go	ood, HSG B			
	1,883	98	Roofs, HSC	B				
	10,217	87	Weighted Average					
	2,948		28.85% Pei	vious Area	а			
	7,269		71.15% Imp	pervious Ar	rea			
_								
ŢĊ	Length	Slope		Capacity	1			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R4: To CB-4

Runoff = 2.23 cfs @ 12.09 hrs, Volume= 7,152 cf, Depth> 5.77" Routed to Pond CB4 :

A	rea (sf)	CN	Description					
	7,280	98	Paved park	ing, HSG B	В			
	4,998	61	>75% Gras	s cover, Go	ood, HSG B			
	2,609	98	Roofs, HSO	βB				
	14,887	86	Weighted A	verage				
	4,998		33.57% Pei	vious Area	a			
	9,889		66.43% Imp	pervious Ar	rea			
_								
Tc	Length	Slope		Capacity	1			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R5: To Foxhole

Runoff = 4.00 cfs @ 12.14 hrs, Volume= 14,365 cf, Depth> 5.07" Routed to Pond IB1 : Infiltration Basin #1

A	rea (sf)	CN I	Description						
	11,765	98 I	Paved parking, HSG B						
	16,262	61 🔅	>75% Ġras	s cover, Go	bod, HSG B				
	5,940	98 I	Roofs, HSC	βB					
	33,967	80 V	Weighted A	verage					
	16,262	4	47.88% Pei	rvious Area					
	17,705	ę	52.12% Imp	pervious Ar	ea				
Tc	Length	Slope		Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.6	50	0.0200	0.15		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.27"				
0.5	68	0.0150	2.49		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
3.9	187	0.0130	0.80		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.1	22	0.0300	3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
10.1	327	Total							

Summary for Subcatchment R6: To CB-6

Runoff = 2.55 cfs @ 12.22 hrs, Volume= 10,745 cf, Depth> 4.96" Routed to Pond CB6 :

	A	rea (sf)	CN E	Description				
	7,642 98 Paved parking, HSG B							
	13,667 61 >75% Grass cover, Good, HSG B							
	4,707 98 Roofs, HSG B							
	26,016 79 Weighted Average							
		13,667	5	52.53% Per	vious Area			
		12,349	4	7.47% Imp	pervious Ar	ea		
	_				_			
	ŢĊ	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	11.9	50	0.0030	0.07		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.27"		
	3.8	158	0.0100	0.70		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.3	40	0.0130	2.31		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	16.0	248	Total					

Summary for Subcatchment R7: To CB-5

Runoff = 3.27 cfs @ 12.16 hrs, Volume= 12,290 cf, Depth> 4.62" Routed to Pond CB5 :

	A	rea (sf)	CN E	Description				
	7,161 98 Paved parking, HSG B							
	19,224 61 >75% Grass cover, Good, HSG B							
	5,516 98 Roofs, HSG B							
	31,901 76 Weighted Average							
		19,224	6	0.26% Per	vious Area			
		12,677	3	9.74% Imp	pervious Ar	ea		
	Тс	Length	Slope	Velocity	Capacity	Description		
(<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.1	50	0.0160	0.14		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.27"		
	5.7	226	0.0090	0.66		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.1	27	0.0500	4.54		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	11.9	303	Total					

Summary for Subcatchment R8: To RGB 2

Runoff = 2.54 cfs @ 12.09 hrs, Volume= 8,112 cf, Depth> 5.65" Routed to Pond HW2 :

A	rea (sf)	CN I	Description						
	6,729	98 I	Paved parking, HSG B						
	6,213	61 ;	>75% Gras	s cover, Go	ood, HSG B				
	4,288	98 I	Roofs, HSC	БВ					
	17,230	230 85 Weighted Average							
	6,213 36.06% Pervious Area								
	11,017	,017 63.94% Impervious Area							
т	المراجع والمراجع	Class	Valasita.	0	Description				
Tc	Length	Slope		Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment R9: To RGB 1

Runoff = 1.16 cfs @ 12.08 hrs, Volume= 4,005 cf, Depth> 6.82" Routed to Reach SW8 :

rea (sf)	CN	Description						
4,963		Paved parking, HSG B						
630	61	>75% Gras	s cover, Go	ood, HSG B				
1,458	98	Roofs, HSO	βB					
7,051 95 Weighted Average								
630	30 8.93% Pervious Area							
6,421	9	91.07% Impervious Area						
Length				Description				
(feet)	(ft/ft)	(ft/sec)	(cfs)					
				Direct Entry,				
	4,963 630 1,458 7,051 630 6,421	4,963 98 630 61 1,458 98 7,051 95 630 6 6,421 9 Length Slope	4,963 98 Paved park 630 61 >75% Gras 1,458 98 Roofs, HSG 7,051 95 Weighted A 630 8.93% Perv 6,421 91.07% Imp Length Slope Velocity	4,96398Paved parking, HSG B63061>75% Grass cover, Go1,45898Roofs, HSG B7,05195Weighted Average6308.93% Pervious Area6,42191.07% Impervious AreaLengthSlopeVelocityCapacity	4,96398Paved parking, HSG B63061>75% Grass cover, Good, HSG B1,45898Roofs, HSG B7,05195Weighted Average6308.93% Pervious Area6,42191.07% Impervious AreaLengthSlopeVelocity(feet)(ft/ft)(ft/sec)(cfs)			

Summary for Subcatchment S3: To Swale

Runoff = 1.11 cfs @ 12.09 hrs, Volume= 3,422 cf, Depth> 4.08" Routed to Reach SW3 :

Α	rea (sf)	CN I	Description						
	2,611	98 I	Paved parking, HSG B						
	7,466	61 3	>75% Ġras	s cover, Go	ood, HSG B				
	10,077	71 \	Weighted Average						
	7,466	-	74.09% Pervious Area						
	2,611	2	25.91% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment S4: To Swale

Runoff = 0.15 cfs @ 12.09 hrs, Volume= 469 cf, Depth> 4.18" Routed to Pond CB7 :

A	rea (sf)	CN	Description					
	392	98	Paved parking, HSG B					
	954	61	>75% Grass cover, Good, HSG B					
	1,346	72	Weighted Average					
	954		70.88% Pervious Area					
	392		29.12% Impervious Area					
Тс	Length	Slope		Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment S8: To Swale

Runoff = 0.62 cfs @ 12.09 hrs, Volume= 1,923 cf, Depth> 4.41" Routed to Reach SW8 :

Ar	rea (sf)	sf) CN	Description					
	1,788	38 98	Paved parking, HSG B					
	3,451	51 61	>75% Grass cover, Good, HSG B					
	5,239	39 74	Weighted A	verage				
	3,451	51	65.87% Pervious Area					
	1,788	38	34.13% Impervious Area					
Tc (min)	Length (feet)			Capacity (cfs)				
6.0					Direct Entry,			
(min)	3,451 5,239 3,451 1,788 Length	5 <u>1 61</u> 39 74 51 38 gth Slop	>75% Grass Weighted A 65.87% Per 34.13% Imp e Velocity	<u>s cover, Go</u> verage vious Area pervious Are Capacity	ood, HSG B rea Description			

Summary for Reach SW3:

Inflow Area = 20,294 sf, 48.68% Impervious, Inflow Depth > 4.98" for 50-yr event Inflow = 2.65 cfs @ 12.09 hrs, Volume= 8.429 cf 2.26 cfs @ 12.14 hrs, Volume= Outflow = 8,395 cf, Atten= 15%, Lag= 3.0 min Routed to Pond CB7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 1.65 fps, Min. Travel Time= 5.1 min Avg. Velocity = 0.54 fps, Avg. Travel Time= 15.4 min Peak Storage= 688 cf @ 12.14 hrs Average Depth at Peak Storage= 0.47', Surface Width= 4.33' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 28.59 cfs 1.50' x 1.50' deep channel, n= 0.041 Riprap, 2-inch Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 501.0' Slope= 0.0100 '/' Inlet Invert= 256.12', Outlet Invert= 251.10'

Summary for Reach SW8:

12,290 sf, 66.79% Impervious, Inflow Depth > 5.79" for 50-yr event Inflow Area = Inflow = 1.78 cfs @ 12.09 hrs, Volume= 5.929 cf 1.28 cfs @ 12.16 hrs, Volume= Outflow = 5,892 cf, Atten= 28%, Lag= 4.6 min Routed to Pond HW2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 0.39 fps, Min. Travel Time= 9.9 min Avg. Velocity = 0.14 fps, Avg. Travel Time= 26.9 min Peak Storage= 765 cf @ 12.16 hrs Average Depth at Peak Storage= 0.83', Surface Width= 6.46' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 4.93 cfs 1.50' x 1.50' deep channel, n= 0.240 Sheet flow over Dense Grass Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 232.0' Slope= 0.0102 '/' Inlet Invert= 255.37', Outlet Invert= 253.00'

Summary for Pond AD:

Inflow Area = 2,664 sf, 0.00% Impervious, Inflow Depth > 3.01" for 50-yr event 0.21 cfs @ 12.09 hrs, Volume= Inflow = 667 cf 0.21 cfs @ 12.09 hrs, Volume= Outflow = 667 cf, Atten= 0%, Lag= 0.0 min 0.21 cfs @ 12.09 hrs, Volume= 667 cf Primary = Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.63' @ 12.49 hrs Flood Elev= 250.75' Device Routing Invert Outlet Devices #1 Primary 248.60' 8.0" Round Culvert L= 26.4' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.60' / 248.47' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.35 sf Primary OutFlow Max=0.21 cfs @ 12.09 hrs HW=249.02' TW=248.95' (Dynamic Tailwater)

1=Culvert (Outlet Controls 0.21 cfs @ 1.31 fps)

Summary for Pond CB1: CB#1

Inflow Area = 11,152 sf, 70.59% Impervious, Inflow Depth > 5.88" for 50-yr event Inflow = 1.69 cfs @ 12.08 hrs, Volume= 5.465 cf 1.69 cfs @ 12.08 hrs, Volume= Outflow = 5,465 cf, Atten= 0%, Lag= 0.0 min 1.69 cfs @ 12.08 hrs, Volume= Primary = 5,465 cf Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.78' @ 12.08 hrs Flood Elev= 251.50' Device Routing Invert Outlet Devices #1 Primary 249.00' 12.0" Round Culvert L= 93.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.00' / 248.53' S= 0.0050 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=1.68 cfs @ 12.08 hrs HW=249.77' TW=248.92' (Dynamic Tailwater)

1=Culvert (Barrel Controls 1.68 cfs @ 3.56 fps)

Summary for Pond CB2:

Inflow Area = 22,269 sf, 62.49% Impervious, Inflow Depth > 5.53" for 50-yr event Inflow = 3.23 cfs @ 12.09 hrs, Volume= 10.271 cf 3.23 cfs @ 12.09 hrs, Volume= Outflow = 10,271 cf, Atten= 0%, Lag= 0.0 min 3.23 cfs @ 12.09 hrs, Volume= Primary = 10,271 cf Routed to Pond SIS2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 252.43' @ 12.09 hrs Flood Elev= 254.02' Device Routing Invert Outlet Devices #1 251.20' Primary 12.0" Round Culvert L= 73.9' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.20' / 250.46' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=3.21 cfs @ 12.09 hrs HW=252.42' TW=250.51' (Dynamic Tailwater)

1=Culvert (Inlet Controls 3.21 cfs @ 4.08 fps)

Summary for Pond CB4:

Inflow Area = 14,887 sf, 66.43% Impervious, Inflow Depth > 5.77" for 50-yr event Inflow = 2.23 cfs @ 12.09 hrs, Volume= 7.152 cf 2.23 cfs @ 12.09 hrs, Volume= Outflow = 7,152 cf, Atten= 0%, Lag= 0.0 min 2.23 cfs @ 12.09 hrs, Volume= Primary = 7,152 cf Routed to Pond DMH4 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.68' @ 12.32 hrs Flood Elev= 250.69' Device Routing Invert Outlet Devices #1 247.83' Primary 12.0" Round Culvert L= 14.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.83' / 247.55' S= 0.0193 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf Primary OutFlow Max=2.21 cfs @ 12.09 hrs HW=248.67' TW=248.04' (Dynamic Tailwater)

1=Culvert(Inlet Controls 2.21 cfs @ 3.13 fps)

Summary for Pond CB5:

Inflow Area = 31,901 sf, 39.74% Impervious, Inflow Depth > 4.62" for 50-yr event Inflow = 3.27 cfs @ 12.16 hrs, Volume= 12.290 cf 3.27 cfs @ 12.16 hrs, Volume= Outflow = 12,290 cf, Atten= 0%, Lag= 0.0 min 3.27 cfs @ 12.16 hrs, Volume= Primary = 12,290 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.77' @ 12.15 hrs Flood Elev= 252.45' Device Routing Invert Outlet Devices #1 250.30' Primary 12.0" Round Culvert L= 8.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 250.30' / 250.20' S= 0.0118 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=3.26 cfs @ 12.16 hrs HW=251.76' TW=251.01' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.26 cfs @ 4.15 fps)

Summary for Pond CB6:

Inflow Area = 26,016 sf, 47.47% Impervious, Inflow Depth > 4.96" for 50-yr event 2.55 cfs @ 12.22 hrs, Volume= Inflow = 10.745 cf 2.55 cfs @ 12.22 hrs, Volume= 10,745 cf, Atten= 0%, Lag= 0.0 min Outflow = 2.55 cfs @ 12.22 hrs, Volume= Primary = 10,745 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.59' @ 12.21 hrs Flood Elev= 250.82' Device Routing Invert Outlet Devices #1 248.30' Primary 12.0" Round Culvert L= 6.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.30' / 248.26' S= 0.0063 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=2.55 cfs @ 12.22 hrs HW=249.59' TW=249.14' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.55 cfs @ 3.24 fps)

Summary for Pond CB7:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 4.92" for 50-yr event 2.39 cfs @ 12.13 hrs, Volume= Inflow = 8.865 cf 2.39 cfs @ 12.13 hrs, Volume= Outflow = 8,865 cf, Atten= 0%, Lag= 0.0 min 2.39 cfs @ 12.13 hrs, Volume= Primary = 8,865 cf Routed to Pond DMH3 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.89' @ 12.16 hrs Flood Elev= 253.00' Device Routing **Outlet Devices** Invert #1 248.00' Primary 18.0" Round Culvert L= 88.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.00' / 247.50' S= 0.0057 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf Primary OutFlow Max=2.25 cfs @ 12.13 hrs HW=248.88' TW=248.48' (Dynamic Tailwater)

1=Culvert (Outlet Controls 2.25 cfs @ 3.01 fps)

Summary for Pond DMH1:

Inflow Outflow Primary	Inflow Area = 13,816 sf, 56.98% Impervious, Inflow Depth > 5.33" for 50-yr event Inflow = 1.90 cfs @ 12.09 hrs, Volume= 6,132 cf Outflow = 1.90 cfs @ 12.09 hrs, Volume= 6,132 cf, Atten= 0%, Lag= 0.0 min Primary = 1.90 cfs @ 12.09 hrs, Volume= 6,132 cf, Atten= 0%, Lag= 0.0 min Primary = 1.90 cfs @ 12.09 hrs, Volume= 6,132 cf Routed to Pond SIS1 : Stormtech 500 hrs, Volume= 500 hrs, Volume=					
Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.63' @ 12.49 hrs Flood Elev= 252.50'						
Device	Routing	Invert	Outlet Devices			
#1	Primary	248.40'	12.0" Round MANIFOLD L= 6.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.40' / 248.35' S= 0.0083 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.79 sf			
#2	Primary	248.28'	24.0" Round ISOLATOR L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.28' / 248.25' S= 0.0075 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf			
Primary OutFlow Max=1.91 cfs @ 12.09 brs $HW=248.93'$ TW=248.81' (Dynamic Tailwater)						

Primary OutFlow Max=1.91 cfs @ 12.09 hrs HW=248.93' TW=248.81' (Dynamic Tailwater) **1=MANIFOLD** (Outlet Controls 0.62 cfs @ 2.14 fps)

2=ISOLATOR (Outlet Controls 1.29 cfs @ 2.19 fps)

Summary for Pond DMH3:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 4.92" for 50-yr event 2.39 cfs @ 12.13 hrs. Volume= Inflow = 8.865 cf 2.39 cfs @ 12.13 hrs, Volume= Outflow = 8,865 cf, Atten= 0%, Lag= 0.0 min 2.39 cfs @ 12.13 hrs, Volume= Primary = 8,865 cf Routed to Pond DMH4 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.66' @ 12.33 hrs Flood Elev= 251.98' Device Routing Invert Outlet Devices #1 247.45' Primary 18.0" Round Culvert L= 68.2' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.45' / 247.10' S= 0.0051 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=2.39 cfs @ 12.13 hrs HW=248.48' TW=248.24' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 2.39 cfs @ 2.59 fps)

Summary for Pond DMH4:

Inflow Area = 36,527 sf, 55.19% Impervious, Inflow Depth > 5.26" for 50-yr event Inflow = 4.47 cfs @ 12.11 hrs, Volume= 16.017 cf 4.47 cfs @ 12.11 hrs, Volume= 16,017 cf, Atten= 0%, Lag= 0.0 min Outflow = 4.47 cfs @ 12.11 hrs, Volume= Primary = 16.017 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.61' @ 12.36 hrs Flood Elev= 251.00' Device Routing Invert Outlet Devices #1 Primary 246.60' 24.0" Round Culvert L= 34.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 246.60' / 246.43' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf Primary OutFlow Max=4.46 cfs @ 12.11 hrs HW=248.13' TW=247.97' (Dynamic Tailwater)

1=Culvert (Outlet Controls 4.46 cfs @ 2.39 fps)

Summary for Pond DMH5:

Inflow Area = 29,520 sf, 65.13% Impervious, Inflow Depth > 5.69" for 50-yr event Inflow = 3.63 cfs @ 12.10 hrs, Volume= 14.004 cf 3.63 cfs @ 12.10 hrs, Volume= 14,004 cf, Atten= 0%, Lag= 0.0 min Outflow = 3.63 cfs @ 12.10 hrs, Volume= Primary = 14,004 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 252.66' @ 12.10 hrs Flood Elev= 254.00' Device Routing Invert Outlet Devices #1 251.65' Primary 15.0" Round Culvert L= 116.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.65' / 250.46' S= 0.0102 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.23 sf

Primary OutFlow Max=3.62 cfs @ 12.10 hrs HW=252.66' TW=251.02' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.62 cfs @ 3.42 fps)

Summary for Pond DMH6:

Inflow Area = 61,421 sf, 51.94% Impervious, Inflow Depth > 5.14" for 50-yr event Inflow = 6.60 cfs @ 12.13 hrs, Volume= 26.294 cf 6.60 cfs @ 12.13 hrs, Volume= 26,294 cf, Atten= 0%, Lag= 0.0 min Outflow = 6.60 cfs @ 12.13 hrs, Volume= Primary = 26,294 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.06' @ 12.13 hrs Flood Elev= 252.93' Device Routing Invert Outlet Devices #1 249.71' Primary 18.0" Round Culvert L= 160.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.71' / 248.10' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=6.58 cfs @ 12.13 hrs HW=251.05' TW=249.04' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 6.58 cfs @ 3.94 fps)

Summary for Pond DMH7:

Inflow Area = 87,437 sf, 50.61% Impervious, Inflow Depth > 5.08" for 50-yr event Inflow = 8.78 cfs @ 12.15 hrs, Volume= 37.039 cf 8.78 cfs @ 12.15 hrs, Volume= 37,039 cf, Atten= 0%, Lag= 0.0 min Outflow = 8.78 cfs @ 12.15 hrs, Volume= Primary = 37.039 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.14' @ 12.21 hrs Flood Elev= 251.25' Device Routing Invert Outlet Devices #1 247.60' Primary 24.0" Round Culvert L= 111.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.60' / 246.65' S= 0.0085 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf Primary OutFlow Max=8.77 cfs @ 12.15 hrs HW=249.08' TW=248.18' (Dynamic Tailwater)

1=Culvert (Outlet Controls 8.77 cfs @ 4.89 fps)

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	26,605 sf,	5.66% Impervious,	Inflow Depth > 3.21" for 50-yr event
Inflow =	1.88 cfs @	12.17 hrs, Volume=	7,117 cf
Outflow =	1.85 cfs @	12.19 hrs, Volume=	7,042 cf, Atten= 2%, Lag= 1.3 min
Discarded =	0.12 cfs @	12.19 hrs, Volume=	3,324 cf
Primary =	1.73 cfs @	12.19 hrs, Volume=	3,718 cf
Routed to Link	AP3 : Abutter	Depression	

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.35' @ 12.19 hrs Surf.Area= 1,929 sf Storage= 496 cf

Plug-Flow detention time= 25.7 min calculated for 7,042 cf (99% of inflow) Center-of-Mass det. time= 19.5 min (866.8 - 847.2)

Volume	Invert	Avail.Stor	rage Stor	age Description		
#1	249.80'	1,12	22 cf Cus	tom Stage Data (P	rismatic)Listed belov	v (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4 250.6	20 20 20	urf.Area (sq-ft) 164 562 1,261 2,144 3,127	Inc.Stor (cubic-fee 7 18 34 52	t) (cubic-feet) 0 0 3 73 2 255 1 595		
Device #1	Routing Discarded	Invert 249.80'	Outlet De 2.410 in/l Conductiv	vices nr Exfiltration over <i>v</i> ity to Groundwater	Elevation = 247.00'	
#2	Primary	250.25'	Head (fee	et) 0.20 0.40 0.60	Broad-Crested Recta 0.80 1.00 1.20 1.40 .70 2.64 2.63 2.64	0 1.60

Discarded OutFlow Max=0.12 cfs @ 12.19 hrs HW=250.35' (Free Discharge) **1=Exfiltration** (Controls 0.12 cfs)

Primary OutFlow Max=1.72 cfs @ 12.19 hrs HW=250.35' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 1.72 cfs @ 0.85 fps)

Summary for Pond HW2:

[58] Hint: Peaked 0.01' above defined flood level [62] Hint: Exceeded Reach SW8 OUTLET depth by 0.25' @ 12.08 hrs

 Inflow Area =
 29,520 sf, 65.13% Impervious, Inflow Depth > 5.69" for 50-yr event

 Inflow =
 3.63 cfs @
 12.10 hrs, Volume=
 14,004 cf

 Outflow =
 3.63 cfs @
 12.10 hrs, Volume=
 14,004 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 3.63 cfs @
 12.10 hrs, Volume=
 14,004 cf

 Routed to Pond DMH5 :
 12.10 hrs, Volume=
 14,004 cf

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 254.01' @ 12.10 hrs Flood Elev= 254.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	253.00'	15.0" Round Culvert L= 14.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 253.00' / 251.75' S= 0.0856 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.62 cfs @ 12.10 hrs HW=254.01' TW=252.66' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.62 cfs @ 3.42 fps)

Summary for Pond IB1: Infiltration Basin #1

[80] Warning: Exceeded Pond DMH4 by 0.31' @ 2.00 hrs (0.76 cfs 859 cf)

Inflow Area =	200,508 sf,	48.93% Impervious,	Inflow Depth > 4.31" for 50-yr event	
Inflow =	18.37 cfs @	12.13 hrs, Volume=	71,985 cf	
Outflow =	10.00 cfs @	12.37 hrs, Volume=	71,973 cf, Atten= 46%, Lag= 14.5 min	
Discarded =	3.52 cfs @	12.37 hrs, Volume=	56,110 cf	
Primary =	6.48 cfs @	12.37 hrs, Volume=	15,862 cf	
Routed to Link	AP1 : To Wetla	ands		
Secondary =	0.00 cfs @	2.00 hrs, Volume=	0 cf	
Routed to Link AP1 : To Wetlands				

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.60' @ 12.37 hrs Surf.Area= 11,039 sf Storage= 15,583 cf

Plug-Flow detention time= 21.1 min calculated for 71,907 cf (100% of inflow) Center-of-Mass det. time= 20.9 min (830.0 - 809.1)

Volume	Invert	Avail.Sto	orage	Storage Description			
#1	247.00'	32,7	'58 cf	cf Custom Stage Data (Irregular)Listed below (Recalc)			
Elevatio (fee		rf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
247.0 248.0	00	10,075	373.0 398.2	0 9,293	0 9,293	8,532 10,126	
249.0 250.0		,	423.3 448.4	10,886 12,579	20,179 32,758	11,817 13,612	
Device	Routing	Invert	Outle	et Devices			
#1	Discarded	247.00'	-	0 in/hr Exfiltration of		1.02' Dhace In= 0.01'	
#2	Primary	247.00'	18.0 Inlet	ductivity to Groundwa " Round Culvert L= / Outlet Invert= 247.0 .012, Flow Area= 1.7	= 62.2' Ke= 0.500 00' / 246.00' S= 0.		
#3	Device 2	247.00'	6.0"	Vert. Orifice/Grate	C= 0.600 Limited	I to weir flow at low heads	
#4	Device 2	248.20'	-	x 2.0" Horiz. Orifice rows C= 0.600 in 24.0		-	
#5	Secondary	249.00'	Limit 10.0 Head	ted to weir flow at low ' long x 10.0' breadt d (feet) 0.20 0.40 0. f. (English) 2.49 2.56	heads t h Broad-Crested 60 0.80 1.00 1.2	Rectangular Weir 0 1.40 1.60	

Discarded OutFlow Max=3.52 cfs @ 12.37 hrs HW=248.60' (Free Discharge) **1=Exfiltration** (Controls 3.52 cfs)

Primary OutFlow Max=6.48 cfs @ 12.37 hrs HW=248.60' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 6.48 cfs of 7.82 cfs potential flow) -3=Orifice/Grate (Orifice Controls 1.10 cfs @ 5.59 fps) -4=Orifice/Grate (Orifice Controls 5.38 cfs @ 3.03 fps)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.00' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond SIS1: Stormtech

Inflow Area = 13,816 sf, 56.98% Impervious, Inflow Depth > 5.33" for 50-yr event Inflow = 1.90 cfs @ 12.09 hrs, Volume= 6.132 cf 0.44 cfs @ 12.49 hrs, Volume= Outflow = 6,132 cf, Atten= 77%, Lag= 24.2 min 0.44 cfs @ 12.49 hrs, Volume= Discarded = 6,132 cf 0.00 cfs @ 2.00 hrs, Volume= Secondary = 0 cf Routed to Link AP1 : To Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.63' @ 12.49 hrs Surf.Area= 1,242 sf Storage= 1,588 cf

Plug-Flow detention time= 23.7 min calculated for 6,127 cf (100% of inflow) Center-of-Mass det. time= 23.7 min (817.1 - 793.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	247.75'	1,151 cf	20.50'W x 60.58'L x 3.50'H Field A
			4,346 cf Overall - 1,470 cf Embedded = 2,876 cf x 40.0% Voids
#2A	248.25'	1,470 cf	ADS_StormTech SC-740 +Cap x 32 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			32 Chambers in 4 Rows
#3	248.00'	35 cf	4.00'D x 2.75'H Vertical Cone/Cylinder-Impervious
		2,655 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	247.75'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 245.51' Phase-In= 0.01'
#2	Secondary	250.60'	2.0" x 2.0" Horiz. Orifice/Grate X 7.00 columns
			X 7 rows C= 0.600 in 24.0" x 24.0" Grate (34% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.44 cfs @ 12.49 hrs HW=249.63' (Free Discharge) **1=Exfiltration** (Controls 0.44 cfs)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.75' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Pond SIS1: Stormtech - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 58.58' Row Length +12.0" End Stone x 2 = 60.58' Base Length 4 Rows x 51.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.50' Base Width

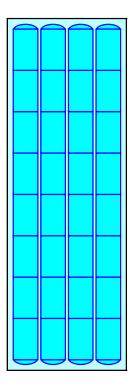
6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

32 Chambers x 45.9 cf = 1,470.1 cf Chamber Storage

4,346.4 cf Field - 1,470.1 cf Chambers = 2,876.3 cf Stone x 40.0% Voids = 1,150.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,620.6 cf = 0.060 afOverall Storage Efficiency = 60.3%Overall System Size = $60.58' \times 20.50' \times 3.50'$

32 Chambers 161.0 cy Field 106.5 cy Stone





Summary for Pond SIS2:

Inflow Area =	24,351 sf, 65.70%	Impervious,	Inflow Depth > 5.67"	for 50-yr event
Inflow =	3.57 cfs @ 12.09 hr	s, Volume=	11,511 cf	-
Outflow =	0.81 cfs @ 12.49 hr	s, Volume=	11,510 cf, Atter	n= 77%, Lag= 24.3 min
Discarded =	0.81 cfs @ 12.49 hr	s, Volume=	11,510 cf	
Primary =	0.00 cfs @ 2.00 hr	s, Volume=	0 cf	
Routed to Pond	I IB1 : Infiltration Basin	#1		

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.84' @ 12.49 hrs Surf.Area= 1,748 sf Storage= 3,221 cf

Plug-Flow detention time= 29.8 min calculated for 11,499 cf (100% of inflow) Center-of-Mass det. time= 29.7 min (819.4 - 789.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	249.00'	1,603 cf	44.25'W x 39.22'L x 3.50'H Field A
			6,074 cf Overall - 2,067 cf Embedded = 4,006 cf x 40.0% Voids
#2A	249.50'	2,067 cf	ADS_StormTech SC-740 +Cap x 45 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			45 Chambers in 9 Rows
#3	250.80'	38 cf	4.00'D x 3.00'H Vertical Cone/Cylinder
		3,708 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	249.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01'
#2	Primary	253.50'	2.0" x 2.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 24.0" x 24.0" Grate (25% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.81 cfs @ 12.49 hrs HW=251.84' (Free Discharge) **1=Exfiltration** (Controls 0.81 cfs)

Primary OutFlow Max=0.00 cfs @ 2.00 hrs HW=249.00' TW=247.00' (Dynamic Tailwater)

Pond SIS2: - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

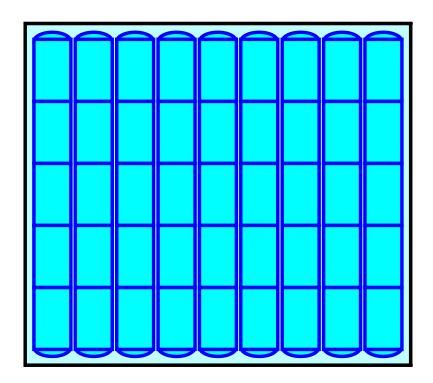
5 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 37.22' Row Length +12.0" End Stone x 2 = 39.22' Base Length 9 Rows x 51.0" Wide + 6.0" Spacing x 8 + 12.0" Side Stone x 2 = 44.25' Base Width 6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

45 Chambers x 45.9 cf = 2,067.3 cf Chamber Storage

6,073.7 cf Field - 2,067.3 cf Chambers = 4,006.4 cf Stone x 40.0% Voids = 1,602.6 cf Stone Storage

Chamber Storage + Stone Storage = 3,669.9 cf = 0.084 afOverall Storage Efficiency = 60.4%Overall System Size = $39.22' \times 44.25' \times 3.50'$

45 Chambers 225.0 cy Field 148.4 cy Stone





Summary for Link AP1: To Wetlands

Inflow Area	a =	236,171 sf, 42.45% Impervious, Infl	ow Depth > 1.28" for 50-yr event
Inflow	=	7.71 cfs @ 12.33 hrs, Volume=	25,098 cf
Primary	=	7.71 cfs @ 12.33 hrs, Volume=	25,098 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP2: To Offsite

Inflow Area	a =	36,823 sf,	7.73% Impervious,	Inflow Depth >	3.21"	for 50-yr event
Inflow	=	3.15 cfs @ 1	12.09 hrs, Volume=	9,864 c	f	
Primary	=	3.15 cfs @	12.09 hrs, Volume=	9,864 c	f, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP3: Abutter Depression

Inflow Area	a =	26,605 sf,	5.66% Impervious,	Inflow Depth = 1.68"	for 50-yr event
Inflow	=	1.73 cfs @ 1	2.19 hrs, Volume=	3,718 cf	
Primary	=	1.73 cfs @ 1	2.19 hrs, Volume=	3,718 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Time span=2.00-24.00 hrs, dt=0.02 hrs, 1101 points x 3 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment A1: To Area Drain	Runoff Area=2,664 sf 0.00% Impervious Runoff Depth>4.11" Tc=6.0 min CN=61 Runoff=0.29 cfs 912 cf
Subcatchment A2: To Exterior	Runoff Area=1,761 sf 0.00% Impervious Runoff Depth>4.11" Tc=6.0 min CN=61 Runoff=0.19 cfs 603 cf
SubcatchmentA3: To Exterior	Runoff Area=3,301 sf 50.05% Impervious Runoff Depth>6.43" Tc=6.0 min CN=80 Runoff=0.56 cfs 1,769 cf
SubcatchmentA4: To Roof Drain	Runoff Area=2,082 sf 100.00% Impervious Runoff Depth>8.56" Tc=6.0 min CN=98 Runoff=0.42 cfs 1,486 cf
Subcatchment A5: To Abutter	Runoff Area=26,605 sf 5.66% Impervious Runoff Depth>4.35" Flow Length=212' Tc=11.8 min CN=63 Runoff=2.57 cfs 9,635 cf
Subcatchment A6: To Abutter	Runoff Area=36,823 sf 7.73% Impervious Runoff Depth>4.35" Tc=6.0 min CN=63 Runoff=4.30 cfs 13,352 cf
Subcatchment A7: To Infiltration Basin	Runoff Area=18,226 sf 0.00% Impervious Runoff Depth>4.11" Tc=6.0 min CN=61 Runoff=2.00 cfs 6,239 cf
Subcatchment A8: To Exterior	Runoff Area=30,601 sf 1.56% Impervious Runoff Depth>3.99" Tc=6.0 min CN=60 Runoff=3.25 cfs 10,166 cf
Subcatchment R1: To CB#1	Runoff Area=11,152 sf 70.59% Impervious Runoff Depth>7.28" Tc=6.0 min CN=87 Runoff=2.07 cfs 6,768 cf
Subcatchment R2: To CB-2	Runoff Area=22,269 sf 62.49% Impervious Runoff Depth>6.92" Tc=6.0 min CN=84 Runoff=3.99 cfs 12,837 cf
Subcatchment R3: To RGB	Runoff Area=10,217 sf 71.15% Impervious Runoff Depth>7.28" Tc=6.0 min CN=87 Runoff=1.90 cfs 6,201 cf
Subcatchment R4: To CB-4	Runoff Area=14,887 sf 66.43% Impervious Runoff Depth>7.16" Tc=6.0 min CN=86 Runoff=2.73 cfs 8,884 cf
Subcatchment R5: To Foxhole	Runoff Area=33,967 sf 52.12% Impervious Runoff Depth>6.42" Flow Length=327' Tc=10.1 min CN=80 Runoff=5.02 cfs 18,186 cf
Subcatchment R6: To CB-6	Runoff Area=26,016 sf 47.47% Impervious Runoff Depth>6.30" Flow Length=248' Tc=16.0 min CN=79 Runoff=3.22 cfs 13,650 cf
Subcatchment R7: To CB-5	Runoff Area=31,901 sf 39.74% Impervious Runoff Depth>5.93" Flow Length=303' Tc=11.9 min CN=76 Runoff=4.17 cfs 15,775 cf
Subcatchment R8: To RGB 2	Runoff Area=17,230 sf 63.94% Impervious Runoff Depth>7.04" Tc=6.0 min CN=85 Runoff=3.12 cfs 10,107 cf

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Subcatchment R9: To RGB	1 Runoff Area=7,051 sf 91.07% Impervious Runoff Depth>8.25" Tc=6.0 min CN=95 Runoff=1.39 cfs 4,845 cf
SubcatchmentS3: To Swale	Runoff Area=10,077 sf 25.91% Impervious Runoff Depth>5.33" Tc=6.0 min CN=71 Runoff=1.44 cfs 4,475 cf
SubcatchmentS4: To Swale	Runoff Area=1,346 sf 29.12% Impervious Runoff Depth>5.45" Tc=6.0 min CN=72 Runoff=0.20 cfs 611 cf
SubcatchmentS8: To Swale	Runoff Area=5,239 sf 34.13% Impervious Runoff Depth>5.70" Tc=6.0 min CN=74 Runoff=0.80 cfs 2,487 cf
Reach SW3:	Avg. Flow Depth=0.53' Max Vel=1.76 fps Inflow=3.34 cfs 10,675 cf n=0.041 L=501.0' S=0.0100 '/' Capacity=28.59 cfs Outflow=2.88 cfs 10,637 cf
Reach SW8:	Avg. Flow Depth=0.92' Max Vel=0.41 fps Inflow=2.19 cfs 7,331 cf n=0.240 L=232.0' S=0.0102 '/' Capacity=4.93 cfs Outflow=1.61 cfs 7,290 cf
Pond AD:	Peak Elev=250.30' Inflow=0.29 cfs 912 cf 8.0" Round Culvert n=0.011 L=26.4' S=0.0049 '/' Outflow=0.29 cfs 912 cf
Pond CB1: CB#1	Peak Elev=250.32' Inflow=2.07 cfs 6,768 cf 12.0" Round Culvert n=0.011 L=93.3' S=0.0050 '/' Outflow=2.07 cfs 6,768 cf
Pond CB2:	Peak Elev=254.00' Inflow=3.99 cfs 12,837 cf 12.0" Round Culvert n=0.011 L=73.9' S=0.0100 '/' Outflow=3.99 cfs 12,837 cf
Pond CB4:	Peak Elev=249.05' Inflow=2.73 cfs 8,884 cf 12.0" Round Culvert n=0.011 L=14.5' S=0.0193 '/' Outflow=2.73 cfs 8,884 cf
Pond CB5:	Peak Elev=252.58' Inflow=4.17 cfs 15,775 cf 12.0" Round Culvert n=0.011 L=8.5' S=0.0118 '/' Outflow=4.17 cfs 15,775 cf
Pond CB6:	Peak Elev=250.18' Inflow=3.22 cfs 13,650 cf 12.0" Round Culvert n=0.011 L=6.3' S=0.0063 '/' Outflow=3.22 cfs 13,650 cf
Pond CB7:	Peak Elev=249.13' Inflow=3.06 cfs 11,248 cf 18.0" Round Culvert n=0.011 L=88.3' S=0.0057 '/' Outflow=3.06 cfs 11,248 cf
Pond DMH1:	Peak Elev=250.30' Inflow=2.36 cfs 7,680 cf Outflow=2.36 cfs 7,680 cf
Pond DMH3:	Peak Elev=249.02' Inflow=3.06 cfs 11,248 cf 18.0" Round Culvert n=0.011 L=68.2' S=0.0051 '/' Outflow=3.06 cfs 11,248 cf
Pond DMH4:	Peak Elev=248.98' Inflow=5.61 cfs 20,132 cf 24.0" Round Culvert n=0.011 L=34.6' S=0.0049 '/' Outflow=5.61 cfs 20,132 cf
Pond DMH5:	Peak Elev=252.85' Inflow=4.50 cfs 17,397 cf 15.0" Round Culvert n=0.011 L=116.8' S=0.0102 '/' Outflow=4.50 cfs 17,397 cf
Pond DMH6:	Peak Elev=251.41' Inflow=8.31 cfs 33,172 cf 18.0" Round Culvert n=0.011 L=160.8' S=0.0100 '/' Outflow=8.31 cfs 33,172 cf

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Pond DMH7:	Peak Elev=249.46' Inflow=11.07 cfs 24.0" Round Culvert n=0.011 L=111.5' S=0.0085 '/' Outflow=11.07 cfs	,
Pond EX: Existing Abut	ter DepressionPeak Elev=250.38' Storage=546 cfInflow=2.57 cfsDiscarded=0.12 cfs3,825 cfPrimary=2.41 cfs5,701 cfOutflow=2.53 cfs	
Pond HW2:	Peak Elev=254.20' Inflow=4.50 cfs 15.0" Round Culvert n=0.013 L=14.6' S=0.0856 '/' Outflow=4.50 cfs	,
Pond IB1: Infiltration Ba Discarded=3.99 cfs 66,331 d	rsin #1 Peak Elev=248.96' Storage=19,697 cf Inflow=23.28 cfs cf Primary=8.69 cfs 25,405 cf Secondary=0.00 cfs 0 cf Outflow=12.68 cfs	
Pond SIS1: Stormtech	Peak Elev=250.30' Storage=2,134 cf Inflow=2.36 cfs Discarded=0.51 cfs 7,680 cf Secondary=0.00 cfs 0 cf Outflow=0.51 cfs	
Pond SIS2:	Peak Elev=253.64' Storage=3,706 cf Inflow=4.40 cfs Discarded=1.11 cfs 13,952 cf Primary=1.38 cfs 371 cf Outflow=2.49 cfs	
Link AP1: To Wetlands	Inflow=10.34 cfs Primary=10.34 cfs	
Link AP2: To Offsite	Inflow=4.30 cfs Primary=4.30 cfs	,
Link AP3: Abutter Depre	ession Inflow=2.41 cfs Primary=2.41 cfs	,

Total Runoff Area = 313,415 sf Runoff Volume = 148,988 cf Average Runoff Depth = 5.70" 64.11% Pervious = 200,945 sf 35.89% Impervious = 112,470 sf

Summary for Subcatchment A1: To Area Drain

Runoff = 0.29 cfs @ 12.09 hrs, Volume= 912 cf, Depth> 4.11" Routed to Pond AD :

Area (sf)	CN I	Description							
2,664	61 :	51 >75% Grass cover, Good, HSG B							
2,664		100.00% Pervious Area							
Tc Length (min) (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
6.0				Direct Entry,					

Summary for Subcatchment A2: To Exterior

Runoff = 0.19 cfs @ 12.09 hrs, Volume= Routed to Link AP1 : To Wetlands 603 cf, Depth> 4.11"

Are	a (sf)	CN D	escription							
	1,761	61 >	61 >75% Grass cover, Good, HSG B							
	1,761	100.00% Pervious Area								
Tc L (min)	ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
6.0					Direct Entry,					

Summary for Subcatchment A3: To Exterior

Runoff = 0.56 cfs @ 12.09 hrs, Volume= Routed to Link AP1 : To Wetlands 1,769 cf, Depth> 6.43"

50.05% Impervious Area				
-				

Summary for Subcatchment A4: To Roof Drain

0.42 cfs @ 12.08 hrs, Volume= 1,486 cf, Depth> 8.56" Runoff = Routed to Pond SIS2 :

Area (sf)	CN	Description		
2,082	98	Roofs, HSG	βB	
2,082		100.00% In	npervious A	vrea
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description
6.0				Direct Entry,

Summary for Subcatchment A5: To Abutter

Runoff = 2.57 cfs @ 12.17 hrs, Volume= 9,635 cf, Depth> 4.35" Routed to Pond EX : Existing Abutter Depression

Α	rea (sf)	CN E	Description					
	23,897	61 >	75% Gras	s cover, Go	bod, HSG B			
	1,201	55 V	Voods, Go	od, HSG B				
	1,507	98 F	Roofs, HSG	БВ				
	26,605	63 V	Veighted A	verage				
	25,098	g	4.34% Per	vious Area				
	1,507	5	.66% Impe	ervious Are	а			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.8	50	0.0120	0.12		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.27"			
5.0	162	0.0060	0.54		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
11.8	212	Total						

Summary for Subcatchment A6: To Abutter

Runoff = 4.30 cfs @ 12.09 hrs, Volume= 13,352 cf, Depth> 4.35" Routed to Link AP2 : To Offsite

A	rea (sf)	CN	Description					
	1,281	98	Paved park	ing, HSG B	3			
	25,992	61	>75% Gras	s cover, Go	ood, HSG B			
	7,984	55	Noods, Go	od, HSG B				
	1,566	98	Roofs, HSG	БB				
	36,823	63	Weighted Average					
	33,976	1	92.27% Pervious Area					
	2,847		7.73% Impervious Area					
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment A7: To Infiltration Basin

Runoff = 2.00 cfs @ 12.09 hrs, Volume= 6,239 cf, Depth> 4.11" Routed to Pond IB1 : Infiltration Basin #1

Area (sf)	CN	N Description						
18,226	61	61 >75% Grass cover, Good, HSG B						
18,226		100.00% Pervious Area						
Tc Length (min) (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
6.0				Direct Entry,				

Summary for Subcatchment A8: To Exterior

Runoff = 3.25 cfs @ 12.09 hrs, Volume= 10,166 cf, Depth> 3.99" Routed to Link AP1 : To Wetlands

Α	rea (sf)	CN I	Description						
	476	98	Paved parking, HSG B						
	20,641	61 :	>75% Gras	s cover, Go	ood, HSG B				
	9,484	55	Noods, Go	od, HSG B	3				
	30,601	60	60 Weighted Average						
	30,125 98.44% Pervious Area								
	476	1.56% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)					
6.0					Direct Entry,				

Summary for Subcatchment R1: To CB#1

Runoff = 2.07 cfs @ 12.08 hrs, Volume= 6,768 cf, Depth> 7.28" Routed to Pond CB1 : CB#1

A	rea (sf)	CN	Description				
	7,872	98	Paved park	ing, HSG B	3		
	3,280	61	>75% Ġras	s cover, Go	ood, HSG B		
	11,152	87	Neighted A	verage			
	3,280		29.41% Pervious Area				
	7,872		70.59% Impervious Area				
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		
					•		

Summary for Subcatchment R2: To CB-2

Runoff = 3.99 cfs @ 12.08 hrs, Volume= 12,837 cf, Depth> 6.92" Routed to Pond CB2 :

A	rea (sf)	CN I	Description					
	8,993	98	Paved park	ing, HSG B	3			
	8,353	61 :	>75% Gras	s cover, Go	ood, HSG B			
	4,923	98	Roofs, HSC	БВ				
	22,269	84	84 Weighted Average					
	8,353	37.51% Pervious Area						
	13,916	62.49% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)		(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment R3: To RGB

Runoff = 1.90 cfs @ 12.08 hrs, Volume= 6,201 cf, Depth> 7.28" Routed to Reach SW3 :

Α	rea (sf)	CN	Description						
	5,386	98	Paved parking, HSG B						
	2,948	61	>75% Gras	s cover, Go	ood, HSG B				
	1,883	98	Roofs, HSC	B					
	10,217	87	Weighted Average						
	2,948		28.85% Pervious Area						
	7,269		71.15% Impervious Area						
_									
ŢĊ	Length	Slope		Capacity	1				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment R4: To CB-4

Runoff = 2.73 cfs @ 12.08 hrs, Volume= 8,884 cf, Depth> 7.16" Routed to Pond CB4 :

A	rea (sf)	CN	Description						
	7,280	98	Paved parking, HSG B						
	4,998	61	>75% Gras	s cover, Go	ood, HSG B				
	2,609	98	Roofs, HSC	БВ					
	14,887	86	S Weighted Average						
	4,998		33.57% Pervious Area						
	9,889		66.43% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)					
6.0					Direct Entry,				

Summary for Subcatchment R5: To Foxhole

Runoff = 5.02 cfs @ 12.14 hrs, Volume= 18,186 cf, Depth> 6.42" Routed to Pond IB1 : Infiltration Basin #1

A	rea (sf)	CN E	Description						
	11,765	98 Paved parking, HSG B							
	16,262	61 >	•75% Ġras	s cover, Go	bod, HSG B				
	5,940	98 F	98 Roofs, HSG B						
	33,967	80 Weighted Average							
	16,262	4	7.88% Pe	vious Area					
	17,705	5	52.12% Imp	pervious Ar	ea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.6	50	0.0200	0.15		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.27"				
0.5	68	0.0150	2.49		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
3.9	187	0.0130	0.80		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
0.1	22	0.0300	3.52		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
10.1	327	Total							

Summary for Subcatchment R6: To CB-6

Runoff = 3.22 cfs @ 12.21 hrs, Volume= 13,650 cf, Depth> 6.30" Routed to Pond CB6 :

	A	rea (sf)	CN E	Description				
		7,642	98 F	98 Paved parking, HSG B				
		13,667	61 >	•75% Gras	s cover, Go	ood, HSG B		
		4,707	98 F	Roofs, HSC	ЪВ			
		26,016	79 V	Veighted A	verage			
		13,667	5	52.53% Per	vious Area			
		12,349	4	7.47% Imp	pervious Ar	ea		
	Тс	Length	Slope	Velocity	Capacity	Description		
(r	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	11.9	50	0.0030	0.07		Sheet Flow,		
						Grass: Short n= 0.150 P2= 3.27"		
	3.8	158	0.0100	0.70		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.3	40	0.0130	2.31		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
1	16.0	248	Total					

Summary for Subcatchment R7: To CB-5

Runoff = 4.17 cfs @ 12.16 hrs, Volume= 15,775 cf, Depth> 5.93" Routed to Pond CB5 :

	A	rea (sf)	CN E	Description					
		7,161	98 F	98 Paved parking, HSG B					
		19,224	61 >	75% Gras	s cover, Go	bod, HSG B			
		5,516	98 F	Roofs, HSG	БВ				
		31,901	76 V	Veighted A	verage				
		19,224	6	0.26% Per	vious Area				
		12,677	3	9.74% Imp	pervious Ar	ea			
	Тс	Length	Slope	Velocity	Capacity	Description			
(<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.1	50	0.0160	0.14		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.27"			
	5.7	226	0.0090	0.66		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.1	27	0.0500	4.54		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	11.9	303	Total						

Summary for Subcatchment R8: To RGB 2

Runoff = 3.12 cfs @ 12.08 hrs, Volume= 10,107 cf, Depth> 7.04" Routed to Pond HW2 :

A	rea (sf)	CN I	CN Description				
	6,729	98	Paved park	ing, HSG B	3		
	6,213	61 :	>75% Gras	s cover, Go	ood, HSG B		
	4,288	98	B Roofs, HSG B				
	17,230	,230 85 Weighted Average					
	6,213 36.06% Pervious Area						
	11,017 63.94% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)			
6.0					Direct Entry,		

Summary for Subcatchment R9: To RGB 1

Runoff = 1.39 cfs @ 12.08 hrs, Volume= 4,845 cf, Depth> 8.25" Routed to Reach SW8 :

A	rea (sf)	CN	N Description				
	4,963	98	Paved park	ing, HSG B			
	630	61	>75% Gras	s cover, Go	od, HSG B		
	1,458	98	Roofs, HSG B				
	7,051	1 95 Weighted Average					
	630	8.93% Pervious Area					
	6,421	91.07% Impervious Area					
_							
ŢĊ	Length	Slope		Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment S3: To Swale

Runoff = 1.44 cfs @ 12.09 hrs, Volume= 4,475 cf, Depth> 5.33" Routed to Reach SW3 :

CN	Description				
98	Paved park	ing, HSG B	3		
61	>75% Ġras	s cover, Go	bod, HSG B		
71	71 Weighted Average				
	74.09% Pervious Area				
	25.91% Impervious Area				
		Capacity (cfs)	Description		
			Direct Entry,		
	98 61 71 Slop	98 Paved park 61 >75% Grass 71 Weighted A 74.09% Per 25.91% Imp Slope Velocity	 98 Paved parking, HSG E 61 >75% Grass cover, Go 71 Weighted Average 74.09% Pervious Area 25.91% Impervious Ar Slope Velocity Capacity 		

Summary for Subcatchment S4: To Swale

Runoff = 0.20 cfs @ 12.09 hrs, Volume= 611 cf, Depth> 5.45" Routed to Pond CB7 :

A	rea (sf)	CN	Description			
	392	98	Paved park	ing, HSG B	3	
	954	61	>75% Ġras	s cover, Go	ood, HSG B	
	1,346	72	Weighted A	verage		
	954		70.88% Pei	vious Area	a	
	392		29.12% Impervious Area			
_				• •	-	
Tc	Length	Slope		Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	
					-	

Summary for Subcatchment S8: To Swale

Runoff = 0.80 cfs @ 12.09 hrs, Volume= 2,487 cf, Depth> 5.70" Routed to Reach SW8 :

Α	rea (sf)	CN	Description				
	1,788	98	Paved park	ing, HSG B	3		
	3,451	61	>75% Grass cover, Good, HSG B				
	5,239	74	Weighted A	verage			
	3,451		65.87% Per	vious Area	1		
	1,788	;	34.13% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Reach SW3:

20,294 sf, 48.68% Impervious, Inflow Depth > 6.31" for 100-yr event Inflow Area = Inflow = 3.34 cfs @ 12.09 hrs, Volume= 10.675 cf 2.88 cfs @ 12.13 hrs, Volume= Outflow = 10,637 cf, Atten= 14%, Lag= 2.9 min Routed to Pond CB7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 1.76 fps, Min. Travel Time= 4.7 min Avg. Velocity = 0.58 fps, Avg. Travel Time= 14.4 min Peak Storage= 821 cf @ 12.13 hrs Average Depth at Peak Storage= 0.53', Surface Width= 4.68' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 28.59 cfs 1.50' x 1.50' deep channel, n= 0.041 Riprap, 2-inch Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 501.0' Slope= 0.0100 '/' Inlet Invert= 256.12', Outlet Invert= 251.10'

Summary for Reach SW8:

[82] Warning: Early inflow requires earlier time span

 Inflow Area =
 12,290 sf, 66.79% Impervious, Inflow Depth > 7.16" for 100-yr event

 Inflow =
 2.19 cfs @
 12.08 hrs, Volume=
 7,331 cf

 Outflow =
 1.61 cfs @
 12.16 hrs, Volume=
 7,290 cf, Atten= 27%, Lag= 4.4 min

 Routed to Pond HW2 :
 1
 1
 1

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Max. Velocity= 0.41 fps, Min. Travel Time= 9.4 min Avg. Velocity = 0.15 fps, Avg. Travel Time= 25.1 min

Peak Storage= 904 cf @ 12.16 hrs Average Depth at Peak Storage= 0.92' , Surface Width= 7.00' Bank-Full Depth= 1.50' Flow Area= 9.0 sf, Capacity= 4.93 cfs

1.50' x 1.50' deep channel, n= 0.240 Sheet flow over Dense Grass Side Slope Z-value= 3.0 '/' Top Width= 10.50' Length= 232.0' Slope= 0.0102 '/' Inlet Invert= 255.37', Outlet Invert= 253.00'

Summary for Pond AD:

Inflow Area = 2,664 sf, 0.00% Impervious, Inflow Depth > 4.11" for 100-yr event 0.29 cfs @ 12.09 hrs, Volume= Inflow = 912 cf 0.29 cfs @ 12.09 hrs, Volume= Outflow = 912 cf, Atten= 0%, Lag= 0.0 min 0.29 cfs @ 12.09 hrs, Volume= Primary = 912 cf Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.30' @ 12.50 hrs Flood Elev= 250.75' Device Routing Invert Outlet Devices #1 Primary 248.60' 8.0" Round Culvert L= 26.4' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.60' / 248.47' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.35 sf Primary OutFlow Max=0.30 cfs @ 12.09 hrs HW=249.26' TW=249.23' (Dynamic Tailwater)

1=Culvert (Outlet Controls 0.30 cfs @ 1.05 fps)

Summary for Pond CB1: CB#1

Inflow Area = 11,152 sf, 70.59% Impervious, Inflow Depth > 7.28" for 100-yr event Inflow = 2.07 cfs @ 12.08 hrs, Volume= 6.768 cf 2.07 cfs @ 12.08 hrs, Volume= Outflow = 6,768 cf, Atten= 0%, Lag= 0.0 min 2.07 cfs @ 12.08 hrs, Volume= Primary = 6,768 cf Routed to Pond DMH1 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.32' @ 12.49 hrs Flood Elev= 251.50' Device Routing Invert Outlet Devices #1 249.00' Primary 12.0" Round Culvert L= 93.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.00' / 248.53' S= 0.0050 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=2.08 cfs @ 12.08 hrs HW=249.89' TW=249.19' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 2.08 cfs @ 3.73 fps)

Summary for Pond CB2:

Inflow Area = 22,269 sf, 62.49% Impervious, Inflow Depth > 6.92" for 100-yr event 3.99 cfs @ 12.08 hrs. Volume= Inflow 12.837 cf = 3.99 cfs @ 12.08 hrs, Volume= Outflow = 12,837 cf, Atten= 0%, Lag= 0.0 min 3.99 cfs @ 12.08 hrs, Volume= Primary = 12.837 cf Routed to Pond SIS2 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 254.00' @ 12.27 hrs Flood Elev= 254.02' Device Routing Invert Outlet Devices #1 251.20' Primary 12.0" Round Culvert L= 73.9' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.20' / 250.46' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=3.97 cfs @ 12.08 hrs HW=252.80' TW=250.99' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.97 cfs @ 5.05 fps)

Summary for Pond CB4:

Inflow Area = 14,887 sf, 66.43% Impervious, Inflow Depth > 7.16" for 100-yr event Inflow = 2.73 cfs @ 12.08 hrs, Volume= 8.884 cf 2.73 cfs @ 12.08 hrs, Volume= Outflow = 8,884 cf, Atten= 0%, Lag= 0.0 min 2.73 cfs @ 12.08 hrs, Volume= Primary = 8,884 cf Routed to Pond DMH4 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.05' @ 12.33 hrs Flood Elev= 250.69' Device Routing Invert Outlet Devices #1 247.83' Primary 12.0" Round Culvert L= 14.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.83' / 247.55' S= 0.0193 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=2.72 cfs @ 12.08 hrs HW=248.88' TW=248.36' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 2.72 cfs @ 3.46 fps)

Summary for Pond CB5:

[58] Hint: Peaked 0.13' above defined flood level

 Inflow Area =
 31,901 sf, 39.74% Impervious, Inflow Depth > 5.93" for 100-yr event

 Inflow =
 4.17 cfs @
 12.16 hrs, Volume=
 15,775 cf

 Outflow =
 4.17 cfs @
 12.16 hrs, Volume=
 15,775 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 4.17 cfs @
 12.16 hrs, Volume=
 15,775 cf

 Routed to Pond DMH6 :
 100-yr event

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 252.58' @ 12.15 hrs Flood Elev= 252.45'

Device	Routing	Invert	Outlet Devices			
#1	Primary	250.30'	12.0" Round Culvert L= 8.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 250.30' / 250.20' S= 0.0118 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf			
Primary OutFlow Mayad 17 of @ 12.16 bro LIM-252.56' TM-251.24' (Dynamia Taihyatar)						

Primary OutFlow Max=4.17 cfs @ 12.16 hrs HW=252.56' TW=251.34' (Dynamic Tailwater) -1=Culvert (Inlet Controls 4.17 cfs @ 5.30 fps)

Summary for Pond CB6:

Inflow Area = 26,016 sf, 47.47% Impervious, Inflow Depth > 6.30" for 100-yr event 3.22 cfs @ 12.21 hrs, Volume= Inflow = 13.650 cf 3.22 cfs @ 12.21 hrs, Volume= Outflow = 13,650 cf, Atten= 0%, Lag= 0.0 min 3.22 cfs @ 12.21 hrs, Volume= Primary = 13.650 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.18' @ 12.21 hrs Flood Elev= 250.82' Device Routing Invert Outlet Devices #1 Primary 248.30' 12.0" Round Culvert L= 6.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.30' / 248.26' S= 0.0063 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=3.21 cfs @ 12.21 hrs HW=250.18' TW=249.45' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 3.21 cfs @ 4.09 fps)

Summary for Pond CB7:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 6.24" for 100-yr event 3.06 cfs @ 12.13 hrs. Volume= Inflow = 11.248 cf 3.06 cfs @ 12.13 hrs, Volume= Outflow = 11,248 cf, Atten= 0%, Lag= 0.0 min 3.06 cfs @ 12.13 hrs, Volume= Primary = 11,248 cf Routed to Pond DMH3 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.13' @ 12.18 hrs Flood Elev= 253.00' Device Routing **Outlet Devices** Invert #1 Primary 248.00' 18.0" Round Culvert L= 88.3' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.00' / 247.50' S= 0.0057 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=2.75 cfs @ 12.13 hrs HW=249.10' TW=248.79' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 2.75 cfs @ 2.77 fps)

Summary for Pond DMH1:

Inflow Area = 13,816 sf, 56.98% Impervious, Inflow Depth > 6.67" for 100-yr event Inflow = 2.36 cfs @ 12.09 hrs, Volume= 7,680 cf Outflow = 2.36 cfs @ 12.09 hrs, Volume= 7,680 cf, Atten= 0%, Lag= 0.0 min Primary = 2.36 cfs @ 12.09 hrs, Volume= 7,680 cf Routed to Pond SIS1 : Stormtech 7,680 cf						
Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.30' @ 12.50 hrs Flood Elev= 252.50'						
Device Routing Invert Outlet Devices						
#1 Primary 248.40' 12.0" Round MANIFOLD L= 6.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.40' / 248.35' S= 0.0083 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.79 sf						
 #2 Primary 248.28' 24.0" Round ISOLATOR L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 248.28' / 248.25' S= 0.0075 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 3.14 sf 						
Primary OutFlow Max=2.37 cfs @ 12.09 hrs HW=249.19' TW=249.12' (Dynamic Tailwater) -1=MANIFOLD (Outlet Controls 0.79 cfs @ 1.64 fps) -2=ISOLATOR (Outlet Controls 1.58 cfs @ 1.67 fps)						

Summary for Pond DMH3:

Inflow Area = 21,640 sf, 47.47% Impervious, Inflow Depth > 6.24" for 100-yr event 3.06 cfs @ 12.13 hrs. Volume= Inflow = 11.248 cf 3.06 cfs @ 12.13 hrs, Volume= Outflow = 11,248 cf, Atten= 0%, Lag= 0.0 min 3.06 cfs @ 12.13 hrs, Volume= Primary = 11,248 cf Routed to Pond DMH4 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.02' @ 12.34 hrs Flood Elev= 251.98' Device Routing Invert Outlet Devices #1 247.45' Primary 18.0" Round Culvert L= 68.2' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.45' / 247.10' S= 0.0051 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=3.05 cfs @ 12.13 hrs HW=248.79' TW=248.59' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 3.05 cfs @ 2.42 fps)

Summary for Pond DMH4:

Inflow Area = 36,527 sf, 55.19% Impervious, Inflow Depth > 6.61" for 100-yr event Inflow = 5.61 cfs @ 12.11 hrs, Volume= 20.132 cf 5.61 cfs @ 12.11 hrs, Volume= Outflow = 20,132 cf, Atten= 0%, Lag= 0.0 min 5.61 cfs @ 12.11 hrs, Volume= Primary = 20,132 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.98' @ 12.36 hrs Flood Elev= 251.00' Device Routing Invert Outlet Devices #1 Primary 246.60' 24.0" Round Culvert L= 34.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 246.60' / 246.43' S= 0.0049 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf Primary OutFlow Max=5.60 cfs @ 12.11 hrs HW=248.47' TW=248.32' (Dynamic Tailwater)

1=Culvert (Outlet Controls 5.60 cfs @ 2.37 fps)

Summary for Pond DMH5:

Inflow Area = 29,520 sf, 65.13% Impervious, Inflow Depth > 7.07" for 100-yr event Inflow = 4.50 cfs @ 12.10 hrs, Volume= 17.397 cf 4.50 cfs @ 12.10 hrs, Volume= Outflow = 17,397 cf, Atten= 0%, Lag= 0.0 min 4.50 cfs @ 12.10 hrs, Volume= Primary = 17,397 cf Routed to Pond DMH6 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 252.85' @ 12.10 hrs Flood Elev= 254.00' Device Routing Invert Outlet Devices #1 251.65' Primary 15.0" Round Culvert L= 116.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 251.65' / 250.46' S= 0.0102 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.23 sf

Primary OutFlow Max=4.49 cfs @ 12.10 hrs HW=252.84' TW=251.36' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 4.49 cfs @ 3.72 fps)

Summary for Pond DMH6:

Inflow Area = 61,421 sf, 51.94% Impervious, Inflow Depth > 6.48" for 100-yr event Inflow = 8.31 cfs @ 12.13 hrs, Volume= 33.172 cf 8.31 cfs @ 12.13 hrs, Volume= Outflow = 33,172 cf, Atten= 0%, Lag= 0.0 min 8.31 cfs @ 12.13 hrs, Volume= Primary = 33,172 cf Routed to Pond DMH7 : Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 251.41' @ 12.13 hrs Flood Elev= 252.93' Device Routing Invert Outlet Devices #1 249.71' Primary 18.0" Round Culvert L= 160.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 249.71' / 248.10' S= 0.0100 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.77 sf

Primary OutFlow Max=8.28 cfs @ 12.13 hrs HW=251.41' TW=249.35' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 8.28 cfs @ 4.69 fps)

Summary for Pond DMH7:

Inflow Area = 87,437 sf, 50.61% Impervious, Inflow Depth > 6.43" for 100-yr event Inflow = 11.07 cfs @ 12.15 hrs, Volume= 46.822 cf 11.07 cfs @ 12.15 hrs, Volume= Outflow = 46,822 cf, Atten= 0%, Lag= 0.0 min 11.07 cfs @ 12.15 hrs, Volume= Primary = 46.822 cf Routed to Pond IB1 : Infiltration Basin #1 Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 249.46' @ 12.21 hrs Flood Elev= 251.25' Device Routing Invert Outlet Devices #1 247.60' Primary 24.0" Round Culvert L= 111.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 247.60' / 246.65' S= 0.0085 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf Primary OutFlow Max=11.05 cfs @ 12.15 hrs HW=249.40' TW=248.54' (Dynamic Tailwater)

1=Culvert (Outlet Controls 11.05 cfs @ 4.90 fps)

Summary for Pond EX: Existing Abutter Depression

Inflow Area =	26,605 sf,	5.66% Impervious,	Inflow Depth > 4.35"	for 100-yr event
Inflow =	2.57 cfs @	12.17 hrs, Volume=	9,635 cf	
Outflow =	2.53 cfs @	12.19 hrs, Volume=	9,526 cf, Atter	n= 1%, Lag= 1.2 min
Discarded =	0.12 cfs @	12.19 hrs, Volume=	3,825 cf	
Primary =	2.41 cfs @	12.19 hrs, Volume=	5,701 cf	
Routed to Link	AP3 : Abutter	Depression		

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.38' @ 12.19 hrs Surf.Area= 2,039 sf Storage= 546 cf

Plug-Flow detention time= 22.5 min calculated for 9,517 cf (99% of inflow) Center-of-Mass det. time= 16.0 min (854.5 - 838.5)

Volume	Invert	Avail.Sto	rage Storag	e Description		
#1	249.80'	1,12	22 cf Custo	m Stage Data (Pi	rismatic)Listed below	v (Recalc)
Elevatio (fee 249.8 250.0 250.2 250.4 250.6	t) 0 0 0 0 0	urf.Area (sq-ft) 164 562 1,261 2,144 3,127	Inc.Store (cubic-feet) 0 73 182 341 527	Cum.Store (cubic-feet) 0 73 255 595 1,122		
<u>Device</u> #1 #2	Routing Discarded Primary	<u>Invert</u> 249.80' 250.25'	Outlet Devic 2.410 in/hr Conductivity 20.0' long 2 Head (feet)	Exfiltration over to Groundwater x 15.0' breadth B 0.20 0.40 0.60	Surface area Elevation = 247.00' road-Crested Recta 0.80 1.00 1.20 1.40 70 2.64 2.63 2.64	n gular Weir D 1.60

Discarded OutFlow Max=0.12 cfs @ 12.19 hrs HW=250.38' (Free Discharge) **1=Exfiltration** (Controls 0.12 cfs)

Primary OutFlow Max=2.40 cfs @ 12.19 hrs HW=250.38' TW=0.00' (Dynamic Tailwater) **2=Broad-Crested Rectangular Weir** (Weir Controls 2.40 cfs @ 0.95 fps)

Summary for Pond HW2:

[58] Hint: Peaked 0.20' above defined flood level [62] Hint: Exceeded Reach SW8 OUTLET depth by 0.34' @ 12.08 hrs

Inflow Area = 29,520 sf, 65.13% Impervious, Inflow Depth > 7.07" for 100-yr event Inflow = 4.50 cfs @ 12.10 hrs, Volume= 17,397 cf = 4.50 cfs @ 12.10 hrs, Volume= 17,397 cf, Atten= 0%, Lag= 0.0 min Outflow = 4.50 cfs @ 12.10 hrs, Volume= Primary 17,397 cf Routed to Pond DMH5 :

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 254.20' @ 12.10 hrs Flood Elev= 254.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	253.00'	15.0" Round Culvert L= 14.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 253.00' / 251.75' S= 0.0856 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=4.49 cfs @ 12.10 hrs HW=254.19' TW=252.84' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 4.49 cfs @ 3.72 fps)

Summary for Pond IB1: Infiltration Basin #1

[80] Warning: Exceeded Pond DMH4 by 0.31' @ 2.00 hrs (0.76 cfs 661 cf)

Inflow Area =	200,508 sf,	, 48.93% Impervious,	Inflow Depth > 5.49" for 100-yr event
Inflow =	23.28 cfs @	12.13 hrs, Volume=	91,751 cf
Outflow =	12.68 cfs @	12.37 hrs, Volume=	91,736 cf, Atten= 46%, Lag= 14.8 min
Discarded =	3.99 cfs @	12.37 hrs, Volume=	66,331 cf
Primary =	8.69 cfs @	12.37 hrs, Volume=	25,405 cf
Routed to Link	AP1 : To Wet	lands	
Secondary =	0.00 cfs @	2.00 hrs, Volume=	0 cf
Routed to Link	AP1 : To Wet	lands	

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 248.96' @ 12.37 hrs Surf.Area= 11,648 sf Storage= 19,697 cf

Plug-Flow detention time= 21.1 min calculated for 91,736 cf (100% of inflow) Center-of-Mass det. time= 21.0 min (823.6 - 802.6)

Volume	Invert	Avail.Sto	orage	Storage Description		
#1	247.00'	32,7	′58 cf	Custom Stage Data	a (Irregular)Listed	below (Recalc)
Elevatio (fee		rf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
247.0 248.0 249.0 250.0)0)0	8,532 10,075 11,718	373.0 398.2 423.3 448.4	0 9,293 10,886 12,579	0 9,293 20,179 32,758	8,532 10,126 11,817 13,612
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	247.00'	-	0 in/hr Exfiltration o		
#2	Primary	247.00'	18.0 Inlet	ductivity to Groundwa " Round Culvert L= / Outlet Invert= 247.0 .012, Flow Area= 1.7	= 62.2' Ke= 0.500 00' / 246.00' S= 0.	
#3	Device 2	247.00'				l to weir flow at low heads
#4	Device 2	248.20'	-	x 2.0" Horiz. Orifice		-
#5	Secondary	249.00'	Limit 10.0 Head	rows C= 0.600 in 24.0 ted to weir flow at low ' long x 10.0' breadt d (feet) 0.20 0.40 0. f. (English) 2.49 2.56	heads h Broad-Crested 60 0.80 1.00 1.2	Rectangular Weir 0 1.40 1.60

Discarded OutFlow Max=3.98 cfs @ 12.37 hrs HW=248.96' (Free Discharge) **1=Exfiltration** (Controls 3.98 cfs)

Primary OutFlow Max=8.69 cfs @ 12.37 hrs HW=248.96' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 8.69 cfs of 9.35 cfs potential flow) -3=Orifice/Grate (Orifice Controls 1.24 cfs @ 6.29 fps) 4=Orifice/Grate (Orifice Controls 7.45 cfs @ 4.19 fps)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.00' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond SIS1: Stormtech

Inflow Area = 13,816 sf, 56.98% Impervious, Inflow Depth > 6.67" for 100-yr event Inflow = 2.36 cfs @ 12.09 hrs, Volume= 7.680 cf 0.51 cfs @ 12.50 hrs, Volume= 7,680 cf, Atten= 78%, Lag= 25.0 min Outflow = 0.51 cfs @ 12.50 hrs, Volume= Discarded = 7.680 cf Secondary = 0.00 cfs @ 2.00 hrs, Volume= 0 cf Routed to Link AP1 : To Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 250.30' @ 12.50 hrs Surf.Area= 1,242 sf Storage= 2,134 cf

Plug-Flow detention time= 29.8 min calculated for 7,673 cf (100% of inflow) Center-of-Mass det. time= 29.8 min (817.7 - 787.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	247.75'	1,151 cf	20.50'W x 60.58'L x 3.50'H Field A
			4,346 cf Overall - 1,470 cf Embedded = 2,876 cf x 40.0% Voids
#2A	248.25'	1,470 cf	ADS_StormTech SC-740 +Cap x 32 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			32 Chambers in 4 Rows
#3	248.00'	35 cf	4.00'D x 2.75'H Vertical Cone/Cylinder-Impervious
		2,655 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	247.75'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 245.51' Phase-In= 0.01'
#2	Secondary	250.60'	2.0" x 2.0" Horiz. Orifice/Grate X 7.00 columns
			X 7 rows C= 0.600 in 24.0" x 24.0" Grate (34% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=0.51 cfs @ 12.50 hrs HW=250.30' (Free Discharge) **1=Exfiltration** (Controls 0.51 cfs)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=247.75' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Pond SIS1: Stormtech - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

8 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 58.58' Row Length +12.0" End Stone x 2 = 60.58' Base Length 4 Rows x 51.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.50' Base Width

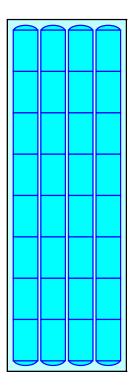
6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

32 Chambers x 45.9 cf = 1,470.1 cf Chamber Storage

4,346.4 cf Field - 1,470.1 cf Chambers = 2,876.3 cf Stone x 40.0% Voids = 1,150.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,620.6 cf = 0.060 afOverall Storage Efficiency = 60.3%Overall System Size = $60.58' \times 20.50' \times 3.50'$

32 Chambers 161.0 cy Field 106.5 cy Stone





Summary for Pond SIS2:

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=4)

Inflow Area =	24,351 sf, 65.70% Impervious,	Inflow Depth > 7.06" for 100-yr event
Inflow =	4.40 cfs @ 12.08 hrs, Volume=	14,323 cf
Outflow =	2.49 cfs @ 12.26 hrs, Volume=	14,322 cf, Atten= 43%, Lag= 10.6 min
Discarded =	1.11 cfs @ 12.26 hrs, Volume=	13,952 cf
Primary =	1.38 cfs @ 12.26 hrs, Volume=	371 cf
Routed to Pond	IB1 : Infiltration Basin #1	

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs / 3 Peak Elev= 253.64' @ 12.26 hrs Surf.Area= 1,748 sf Storage= 3,706 cf

Plug-Flow detention time= 31.1 min calculated for 14,309 cf (100% of inflow) Center-of-Mass det. time= 31.1 min (815.3 - 784.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	249.00'	1,603 cf	44.25'W x 39.22'L x 3.50'H Field A
			6,074 cf Overall - 2,067 cf Embedded = 4,006 cf x 40.0% Voids
#2A	249.50'	2,067 cf	ADS_StormTech SC-740 +Cap x 45 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			45 Chambers in 9 Rows
#3	250.80'	38 cf	4.00'D x 3.00'H Vertical Cone/Cylinder
		3,708 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	249.00'	8.270 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01'
#2	Primary	253.50'	2.0" x 2.0" Horiz. Orifice/Grate X 6.00 columns
	-		X 6 rows C= 0.600 in 24.0" x 24.0" Grate (25% open area)
			Limited to weir flow at low heads

Discarded OutFlow Max=1.11 cfs @ 12.26 hrs HW=253.64' (Free Discharge) **1=Exfiltration** (Controls 1.11 cfs)

Primary OutFlow Max=1.28 cfs @ 12.26 hrs HW=253.63' TW=248.87' (Dynamic Tailwater) 2=Orifice/Grate (Weir Controls 1.28 cfs @ 1.20 fps)

Pond SIS2: - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length)

Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

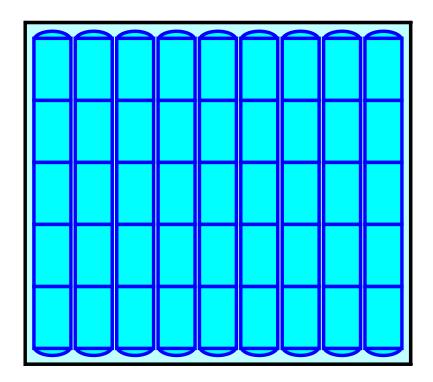
5 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 37.22' Row Length +12.0" End Stone x 2 = 39.22' Base Length 9 Rows x 51.0" Wide + 6.0" Spacing x 8 + 12.0" Side Stone x 2 = 44.25' Base Width 6.0" Stone Base + 30.0" Chamber Height + 6.0" Stone Cover = 3.50' Field Height

45 Chambers x 45.9 cf = 2,067.3 cf Chamber Storage

6,073.7 cf Field - 2,067.3 cf Chambers = 4,006.4 cf Stone x 40.0% Voids = 1,602.6 cf Stone Storage

Chamber Storage + Stone Storage = 3,669.9 cf = 0.084 afOverall Storage Efficiency = 60.4%Overall System Size = $39.22' \times 44.25' \times 3.50'$

45 Chambers 225.0 cy Field 148.4 cy Stone





Summary for Link AP1: To Wetlands

Inflow Area	a =	236,171 sf, 42.45% Impervious, Inflow Depth > 1.93" for 100-yr ev	vent
Inflow	=	10.34 cfs @ 12.32 hrs, Volume= 37,942 cf	
Primary	=	10.34 cfs @ 12.32 hrs, Volume= 37,942 cf, Atten= 0%, Lag= 0).0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP2: To Offsite

Inflow Area	a =	36,823 sf,	7.73% Impervious,	Inflow Depth > 4.3	5" for 100-yr event
Inflow	=	4.30 cfs @	12.09 hrs, Volume=	13,352 cf	
Primary	=	4.30 cfs @	12.09 hrs, Volume=	13,352 cf, At	tten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

Summary for Link AP3: Abutter Depression

Inflow Are	a =	26,605 sf,	5.66% Impervious,	Inflow Depth = 2.57"	for 100-yr event
Inflow	=	2.41 cfs @ 1	2.19 hrs, Volume=	5,701 cf	
Primary	=	2.41 cfs @ 1	2.19 hrs, Volume=	5,701 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.02 hrs

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Stage-Area-Storage for Pond AD:

F 1 (*	01		01		01
Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
248.60	0	249.66	0	250.72	0
248.62	0	249.68	0	250.74	0
248.64	0	249.70	0		
248.66	0	249.72	0		
248.68	0	249.74	0		
248.70	0	249.76	0		
248.72	0	249.78	0		
248.74	0	249.80	0		
248.76	0	249.82 249.84	0		
248.78 248.80	0 0	249.84 249.86	0 0		
248.82	0	249.88	0		
240.02	0	249.88	0		
248.86	0	249.90	0		
248.88	0	249.92	0		
248.90	0	249.94	0		
248.92	0	249.90	0		
248.92	0	250.00	0		
248.96	0	250.00	0		
248.98	0	250.02	0		
249.00	0	250.04	0		
249.02	0	250.00	0		
249.02	0	250.10	0		
249.06	0	250.10	0		
249.08	0 0	250.12	0 0		
249.10	Ő	250.16	0 0		
249.12	Ő	250.18	0 0		
249.14	Ő	250.20	Ő		
249.16	0	250.22	0		
249.18	0	250.24	0		
249.20	0	250.26	0		
249.22	0	250.28	0		
249.24	0	250.30	0		
249.26	0	250.32	0		
249.28	0	250.34	0		
249.30	0	250.36	0		
249.32	0	250.38	0		
249.34	0	250.40	0		
249.36	0	250.42	0		
249.38	0	250.44	0		
249.40	0	250.46	0		
249.42	0	250.48	0		
249.44	0	250.50	0		
249.46	0	250.52	0		
249.48	0	250.54	0		
249.50	0	250.56	0		
249.52	0	250.58	0		
249.54	0	250.60	0		
249.56	0	250.62	0		
249.58	0	250.64	0		
249.60 249.62	0 0	250.66 250.68	0 0		
249.62 249.64	0	250.68	0		
249.04	U	200.70	U		

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Stage-Area-Storage for Pond CB1: CB#1

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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	249.00	0	250.06	0	251.12	0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	249.02	0	250.08	0	251.14	0
249.080 250.14 0 251.20 0 249.10 0 250.16 0 251.22 0 249.14 0 250.20 0 251.24 0 249.14 0 250.22 0 251.28 0 249.18 0 250.24 0 251.30 0 249.20 0 250.26 0 251.32 0 249.20 0 250.26 0 251.34 0 249.24 0 250.32 0 251.38 0 249.26 0 250.34 0 251.38 0 249.26 0 250.34 0 251.44 0 249.26 0 250.36 0 251.44 0 249.26 0 250.44 0 251.44 0 249.30 0 250.44 0 251.46 0 249.34 0 250.44 0 251.46 0 249.38 0 250.44 0 251.50 0 249.44 0 250.50 0249.440 249.45 0 250.56 0249.540 249.46 0 250.56 0249.540 249.54 0 250.72 0249.540 249.54 0 250.76 0249.540 249.54 0 250.76 0249.660 249.66 0 250.72 0249.640 249.74 0 250.88 0249.74 <td>249.04</td> <td>0</td> <td>250.10</td> <td>0</td> <td>251.16</td> <td>0</td>	249.04	0	250.10	0	251.16	0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	249.06	0	250.12	0	251.18	0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	249.08	0	250.14	0	251.20	0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	249.10	0	250.16	0	251.22	0
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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	249.20	0	250.26	0	251.32	0
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Stage-Area-Storage for Pond CB2:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
251.20	0	252.26	0	253.32	0
251.22	0	252.28	0	253.34	0
251.24	0	252.30	0	253.36	0
251.26	0	252.32	0	253.38	0
251.28	0	252.34	0	253.40	0
251.30	Ō	252.36	0	253.42	0
251.32	Ő	252.38	Ő	253.44	0
251.34	Ō	252.40	0	253.46	0
251.36	Ō	252.42	0	253.48	0
251.38	0	252.44	0	253.50	0
251.40	0	252.46	0	253.52	0
251.42	0	252.48	0	253.54	0
251.44	0	252.50	0	253.56	0
251.46	Ō	252.52	0	253.58	0
251.48	0	252.54	0	253.60	0
251.50	0	252.56	0	253.62	0
251.52	0	252.58	0	253.64	0
251.54	0	252.60	0	253.66	0
251.56	0	252.62	0	253.68	0
251.58	0	252.64	0	253.70	0
251.60	0	252.66	0	253.72	0
251.62	0	252.68	0	253.74	0
251.64	0	252.70	0	253.76	0
251.66	0	252.72	0	253.78	0
251.68	0	252.74	0	253.80	0
251.70	0	252.76	0	253.82	0
251.72	0	252.78	0	253.84	0
251.74	0	252.80	0	253.86	0
251.76	0	252.82	0	253.88	0
251.78	0	252.84	0	253.90	0
251.80	0	252.86	0	253.92	0
251.82	0	252.88	0	253.94	0
251.84	0	252.90	0	253.96	0
251.86	0	252.92	0	253.98	0
251.88	0	252.94	0	254.00	0
251.90	0	252.96	0	254.02	0
251.92	0	252.98	0		
251.94	0	253.00	0		
251.96	0	253.02	0		
251.98	0	253.04	0		
252.00	0	253.06	0		
252.02	0	253.08	0		
252.04	0	253.10	0		
252.06	0	253.12	0		
252.08	0	253.14	0		
252.10	0	253.16	0		
252.12	0	253.18	0		
252.14	0	253.20	0		
252.16	0	253.22	0		
252.18	0	253.24	0		
252.20	0	253.26	0		
252.22	0	253.28	0		
252.24	0	253.30	0		
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Stage-Area-Storage for Pond CB4:

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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
248.010 249.07 0 250.13 0 248.03 0 249.09 0 250.15 0 248.05 0 249.11 0 250.17 0 248.07 0 249.13 0 250.19 0 248.09 0 249.15 0 250.23 0 248.11 0 249.17 0 250.25 0 248.13 0 249.17 0 250.25 0 248.15 0 249.23 0 250.27 0 248.17 0 249.23 0 250.31 0 248.21 0 249.27 0 250.35 0 248.23 0 249.27 0 250.35 0 248.24 0 249.27 0 250.35 0 248.25 0 249.33 0 250.37 0 248.24 0 249.37 0 250.43 0 248.29 0 249.33 0 250.43 0 248.31 0 249.39 0 250.45 0 248.33 0 249.43 0 250.45 0 248.34 0 249.47 0 250.55 0 248.43 0 249.47 0 250.55 0 248.44 0 249.47 0 250.55 0 248.43 0 249.45 0 250.57 0 248.44 0 249.45 0 250.55 0 248.45 0 249.55 0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
248.110 249.17 0 250.23 0 248.13 0 249.19 0 250.25 0 248.15 0 249.21 0 250.27 0 248.17 0 249.23 0 250.29 0 248.19 0 249.25 0 250.31 0 248.21 0 249.27 0 250.33 0 248.23 0 249.31 0 250.37 0 248.27 0 249.33 0 250.39 0 248.29 0 249.35 0 250.41 0 248.31 0 249.37 0 250.43 0 248.33 0 249.43 0 250.45 0 248.34 0 249.41 0 250.47 0 248.35 0 249.43 0 250.45 0 248.34 0 249.45 0 250.55 0 248.43 0 249.47 0 250.55 0 248.43 0 249.47 0 250.55 0 248.44 0 249.55 0 250.55 0 248.45 0 249.57 0 250.663 0 248.53 0 249.57 0 250.65 0 248.53 0 249.63 0 250.65 0 248.54 0 249.65 0 250.65 0 248.63 0 249.67 0 250.65 0 248.63 0 249.67 0
248.130 249.19 0 250.25 0 248.15 0 249.21 0 250.27 0 248.17 0 249.23 0 250.29 0 248.19 0 249.25 0 250.31 0 248.21 0 249.27 0 250.33 0 248.23 0 249.29 0 250.35 0 248.25 0 249.31 0 250.37 0 248.27 0 249.33 0 250.43 0 248.31 0 249.35 0 250.43 0 248.33 0 249.49 0 250.45 0 248.34 0 249.41 0 250.45 0 248.35 0 249.41 0 250.47 0 248.36 0 249.43 0 250.55 0 248.43 0 249.45 0 250.55 0 248.44 0 249.47 0 250.55 0 248.45 0 249.51 0 250.55 0 248.44 0 249.55 0 250.55 0 248.45 0 249.55 0 250.63 0 248.47 0 249.55 0 250.65 0 248.49 0 249.63 0 250.65 0 248.51 0 249.65 0 250.65 0 248.53 0 249.67 0 250.65 0 248.63 0 249.67 0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
248.490249.550250.610248.510249.570250.630248.530249.590250.650248.550249.610250.670248.570249.630250.690248.590249.650250.690248.610249.670249.630
248.530249.590250.650248.550249.610250.670248.570249.630250.690248.590249.650250.690248.610249.670249.630248.630249.6901
248.550249.610250.670248.570249.630250.690248.590249.650249.650248.610249.670249.690
248.570249.630250.690248.590249.650248.610249.670248.630249.690
248.590249.650248.610249.670248.630249.690
248.61 0 249.67 0 248.63 0 249.69 0
248.63 0 249.69 0
248.65 0 249.71 0
248.67 0 249.73 0
248.69 0 249.75 0
248.71 0 249.77 0
248.73 0 249.79 0
248.75 0 249.81 0
248.77 0 249.83 0 248.79 0 249.85 0
248.79 0 249.85 0 248.81 0 249.87 0
248.83 0 249.89 0
248.85 0 249.91 0
248.87 0 249.93 0

Stage-Area-Storage for Pond CB5:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
250.30	0	251.36	0	252.42	0
250.32	0	251.38	0 0	252.44	Ő
250.34	0 0	251.40	0 0	202.11	Ũ
250.36	0 0	251.42	0 0		
250.38	0 0	251.44	0		
250.40	Õ	251.46	ů 0		
250.42	0 0	251.48	0 0		
250.44	0 0	251.50	0 0		
250.46	0 0	251.52	0 0		
250.48	Ő	251.54	0		
250.50	Ō	251.56	0		
250.52	0	251.58	0		
250.54	0	251.60	0		
250.56	0	251.62	0		
250.58	0	251.64	0		
250.60	0	251.66	0		
250.62	0	251.68	0		
250.64	0	251.70	0		
250.66	0	251.72	0		
250.68	0	251.74	0		
250.70	0	251.76	0		
250.72	0	251.78	0		
250.74	0	251.80	0		
250.76	0	251.82	0		
250.78	0	251.84	0		
250.80	0	251.86	0		
250.82	0	251.88	0		
250.84	0	251.90	0		
250.86	0	251.92	0		
250.88	0	251.94	0		
250.90	0	251.96	0		
250.92	0	251.98	0		
250.94	0	252.00	0		
250.96	0	252.02	0		
250.98	0	252.04	0		
251.00	0	252.06	0		
251.02	0	252.08	0		
251.04	0	252.10	0		
251.06	0	252.12	0		
251.08	0	252.14	0		
251.10	0	252.16	0		
251.12	0	252.18	0		
251.14	0	252.20	0		
251.16	0	252.22	0		
251.18	0	252.24	0		
251.20	0	252.26	0		
251.22	0	252.28	0		
251.24 251.26	0 0	252.30 252.32	0 0		
251.26	0	252.32	0		
251.20	0	252.34	0		
251.30	0	252.30	0		
251.32	0	252.40	0		
201.04	0		5		

Stage-Area-Storage for Pond CB6:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
248.30	0	249.36	0	250.42	0
248.32	0	249.38	0	250.44	0
248.34	0	249.40	0	250.46	0
248.36	0	249.42	0	250.48	0
248.38	0	249.44	0	250.50	0
248.40	0	249.46	0	250.52	0
248.42	0	249.48	0	250.54	0
248.44	0	249.50	0	250.56	0
248.46	0	249.52	0	250.58	0
248.48	0	249.54	0	250.60	0
248.50	0	249.56	0	250.62	0
248.52	0	249.58	0	250.64	0
248.54	0	249.60	0	250.66	0
248.56	0	249.62	0	250.68	0
248.58	0	249.64	0	250.70	0
248.60	0	249.66	0	250.72	0
248.62	0	249.68	0	250.74	0
248.64	0	249.70	0	250.76	0
248.66	0	249.72	0	250.78	0
248.68	0	249.74	0	250.80	0
248.70	0	249.76	0	250.82	0
248.72	0	249.78	0		-
248.74	0	249.80	0 0		
248.76	0	249.82	0 0		
248.78	0	249.84	0 0		
248.80	0	249.86	0 0		
248.82	0	249.88	Ō		
248.84	0	249.90	0		
248.86	0	249.92	0		
248.88	0	249.94	0		
248.90	0	249.96	0		
248.92	0	249.98	0		
248.94	0	250.00	0		
248.96	0	250.02	0		
248.98	0	250.04	0		
249.00	0	250.06	0		
249.02	0	250.08	0		
249.04	0	250.10	0		
249.06	0	250.12	0		
249.08	0	250.14	0		
249.10	0	250.16	0		
249.12	0	250.18	0		
249.14	0	250.20	0		
249.16	0	250.22	0		
249.18	0	250.24	0		
249.20	0	250.26	0		
249.22	0	250.28	0		
249.24	0	250.30	0		
249.26	0	250.32	0		
249.28	0	250.34	0		
249.30	0	250.36	0		
249.32	0	250.38	0		
249.34	0	250.40	0		
		l		l	

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Stage-Area-Storage for Pond CB7:

Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)
248.00	0	250.65	0
248.05	0	250.70	0
248.10	0	250.75	0
248.15	0	250.80	0
248.20	0	250.85	0
248.25	0	250.90	0
248.30	0	250.95	0
248.35	0	251.00	0
248.40	0	251.05	0
248.45	0	251.10	0 0
248.50 248.55	0 0	251.15 251.20	0
248.55	0	251.20	0
248.65	0	251.25	0
248.00	0	251.30	0
248.70	0	251.35	0
248.80	0	251.45	0
248.85	0	251.50	0
248.90	0	251.55	0 0
248.95	Ő	251.60	Õ
249.00	0	251.65	ů 0
249.05	Ő	251.70	Ő
249.10	Ő	251.75	Ő
249.15	0	251.80	Ō
249.20	0	251.85	0
249.25	0	251.90	0
249.30	0	251.95	0
249.35	0	252.00	0
249.40	0	252.05	0
249.45	0	252.10	0
249.50	0	252.15	0
249.55	0	252.20	0
249.60	0	252.25	0
249.65	0	252.30	0
249.70	0	252.35	0
249.75	0	252.40	0
249.80	0	252.45	0
249.85	0	252.50	0
249.90	0	252.55	0
249.95	0 0	252.60	0 0
250.00 250.05	0	252.65 252.70	0
250.05	0	252.70	0
250.10	0	252.80	0
250.10	0	252.85	0
250.25	0	252.90	ů 0
250.30	Ő	252.95	Õ
250.35	Ő	253.00	0 0
250.40	Ő		5
250.45	Ő		
250.50	Ō		
250.55	0		
250.60	0		
		l	

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Stage-Area-Storage for Pond DMH1:

Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)
248.28	0	250.93	0
248.33	0	250.98	0
248.38	0	251.03	0
248.43	0	251.08	0
248.48	0	251.13	0
248.53	0	251.18	0
248.58	0	251.23	0
248.63	0	251.28	0
248.68	0	251.33	0
248.73	0	251.38	0
248.78	0	251.43	0
248.83	0	251.48	0
248.88	0	251.53	0
248.93	0	251.58	0
248.98	0	251.63	0
249.03 249.08	0	251.68	0 0
	0	251.73	
249.13 249.18	0 0	251.78 251.83	0 0
249.18	0	251.88	0
249.23	0	251.88	0
249.20	0	251.93	0
249.33	0	252.03	0
249.30	0	252.03	0
249.43	0	252.08	0
249.40	0	252.13	0
249.58	0	252.10	0
249.63	0	252.28	0
249.68	0	252.20	0
249.73	Ő	252.38	0 0
249.78	Ő	252.43	Ő
249.83	Ő	252.48	0 0
249.88	Ő	202.10	0
249.93	Ő		
249.98	Ő		
250.03	Ō		
250.08	0		
250.13	0		
250.18	0		
250.23	0		
250.28	0		
250.33	0		
250.38	0		
250.43	0		
250.48	0		
250.53	0		
250.58	0		
250.63	0		
250.68	0		
250.73	0		
250.78	0		
250.83	0		
250.88	0		
		I	

Stage-Area-Storage for Pond DMH3:

	_		_
Elevation	Storage	Elevation	Storage
(feet) 247.45	(cubic-feet)	(feet)	(cubic-feet)
247.45 247.50	0 0	250.10 250.15	0 0
247.55	0	250.13	0
247.60	0	250.25	0
247.65	0 0	250.30	0
247.70	Ő	250.35	0 0
247.75	0	250.40	0
247.80	0	250.45	0
247.85	0	250.50	0
247.90	0	250.55	0
247.95	0	250.60	0
248.00	0	250.65	0
248.05 248.10	0 0	250.70 250.75	0 0
248.10	0	250.75	0
248.20	0	250.85	0
248.25	Ő	250.90	Ő
248.30	0	250.95	0
248.35	0	251.00	0
248.40	0	251.05	0
248.45	0	251.10	0
248.50	0	251.15	0
248.55	0	251.20	0
248.60 248.65	0 0	251.25 251.30	0 0
248.70	0	251.30	0
248.75	0	251.40	0
248.80	Ő	251.45	Ő
248.85	0	251.50	0
248.90	0	251.55	0
248.95	0	251.60	0
249.00	0	251.65	0
249.05	0	251.70	0
249.10	0	251.75	0
249.15 249.20	0 0	251.80 251.85	0 0
249.20	0	251.00	0
249.30	0	251.95	0
249.35	Õ	201.00	Ũ
249.40	Ō		
249.45	0		
249.50	0		
249.55	0		
249.60	0		
249.65	0		
249.70 249.75	0 0		
249.75	0		
249.85	0 0		
249.90	Ő		
249.95	0		
250.00	0		
250.05	0		
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Stage-Area-Storage for Pond DMH4:

Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)
246.60	0	249.25	0
246.65 246.70	0	249.30 249.35	0 0
246.75	0 0	249.35	0
246.80	0	249.45	0
246.85	0	249.50	0
246.90	Õ	249.55	ů 0
246.95	Ő	249.60	0
247.00	0	249.65	0
247.05	0	249.70	0
247.10	0	249.75	0
247.15	0	249.80	0
247.20	0	249.85	0
247.25	0	249.90	0
247.30	0	249.95	0
247.35 247.40	0 0	250.00 250.05	0 0
247.40 247.45	0	250.05	0
247.50	0	250.10	0
247.55	0 0	250.20	0
247.60	ů 0	250.25	0
247.65	Ő	250.30	0
247.70	0	250.35	0
247.75	0	250.40	0
247.80	0	250.45	0
247.85	0	250.50	0
247.90	0	250.55	0
247.95	0	250.60	0
248.00	0	250.65	0
248.05 248.10	0 0	250.70 250.75	0 0
248.10	0	250.75	0
248.20	0 0	250.85	0
248.25	Ő	250.90	ů 0
248.30	Ō	250.95	0
248.35	0	251.00	0
248.40	0		
248.45	0		
248.50	0		
248.55	0		
248.60	0		
248.65 248.70	0		
248.75	0 0		
248.80	0		
248.85	Õ		
248.90	Õ		
248.95	0		
249.00	0		
249.05	0		
249.10	0		
249.15	0		
249.20	0		

Stage-Area-Storage for Pond DMH5:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
251.65	0	252.71	0	253.77	0
251.67	0	252.73	0	253.79	0
251.69	0	252.75	0	253.81	0
251.71	0 0	252.77	Ő	253.83	0
251.73	0 0	252.79	ů 0	253.85	0
			0		
251.75	0	252.81		253.87	0
251.77	0	252.83	0	253.89	0
251.79	0	252.85	0	253.91	0
251.81	0	252.87	0	253.93	0
251.83	0	252.89	0	253.95	0
251.85	0	252.91	0	253.97	0
251.87	0	252.93	0	253.99	0
251.89	0	252.95	0		
251.91	0	252.97	0		
251.93	0	252.99	0		
251.95	0	253.01	0		
251.97	0 0	253.03	Ő		
251.99	Ő	253.05	Ő		
252.01	0 0	253.07	Ŭ Ŭ		
252.01	0	253.07	0		
		253.09			
252.05	0		0		
252.07	0	253.13	0		
252.09	0	253.15	0		
252.11	0	253.17	0		
252.13	0	253.19	0		
252.15	0	253.21	0		
252.17	0	253.23	0		
252.19	0	253.25	0		
252.21	0	253.27	0		
252.23	0	253.29	0		
252.25	0	253.31	0		
252.27	0	253.33	0		
252.29	0	253.35	Ō		
252.31	0	253.37	Ō		
252.33	0 0	253.39	Ő		
252.35	0 0	253.41	Ŭ Ŭ		
252.37	0	253.43	0 0		
252.37	0	253.45	0		
	-		-		
252.41	0	253.47	0		
252.43	0	253.49	0		
252.45	0	253.51	0		
252.47	0	253.53	0		
252.49	0	253.55	0		
252.51	0	253.57	0		
252.53	0	253.59	0		
252.55	0	253.61	0		
252.57	0	253.63	0		
252.59	0	253.65	0		
252.61	0	253.67	0		
252.63	0 0	253.69	0 0		
252.65	Ő	253.71	Ő		
252.67	0 0	253.73	Ŭ Ŭ		
252.69	0	253.75	0		
202.00	0	200.70	0		

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Stage-Area-Storage for Pond DMH6:

$ \begin{array}{c c c c c c c c c c c c c c c c c c c $				
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Elevation	Storage	Elevation	Storage
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	(feet)		(feet)	(cubic-feet)
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		0		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	250.46	0		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	250.56	0		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	250.61			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		0		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	250.71	0		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	250.76			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	250.81			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	250.86			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
251.96 0 252.01 0 252.06 0 252.11 0 252.16 0 252.21 0 252.26 0				
252.01 0 252.06 0 252.11 0 252.16 0 252.21 0 252.26 0				
252.06 0 252.11 0 252.16 0 252.21 0 252.26 0				
252.11 0 252.16 0 252.21 0 252.26 0				
252.16 0 252.21 0 252.26 0				
252.21 0 252.26 0				
252.26 0				
	202.01	5		

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Stage-Area-Storage for Pond DMH7:

$\begin{array}{c c} Elevation & Storage \\ \hline (feet) & (cubic-feet) \\ \hline 247.60 & 0 \\ 247.75 & 0 & 250.25 & 0 \\ 247.75 & 0 & 250.30 & 0 \\ 247.80 & 0 & 250.45 & 0 \\ 247.80 & 0 & 250.45 & 0 \\ 247.95 & 0 & 250.65 & 0 \\ 247.95 & 0 & 250.65 & 0 \\ 248.00 & 0 & 250.65 & 0 \\ 248.00 & 0 & 250.75 & 0 \\ 248.10 & 0 & 250.75 & 0 \\ 248.20 & 0 & 250.85 & 0 \\ 248.20 & 0 & 250.85 & 0 \\ 248.20 & 0 & 250.95 & 0 \\ 248.30 & 0 & 250.95 & 0 \\ 248.45 & 0 & 250.95 & 0 \\ 248.45 & 0 & 251.00 & 0 \\ 248.45 & 0 & 251.10 & 0 \\ 248.45 & 0 & 251.15 & 0 \\ 248.65 & 0 & 251.20 & 0 \\ 248.85 & 0 & 251.20 & 0 \\ 248.85 & 0 & 251.25 & 0 \\ 248.85 & 0 & 251.20 & 0 \\ 248.85 & 0 & 251.25 & 0 \\ 248.85 & 0 & 251.25 & 0 \\ 248.90 & 0 & 248.85 & 0 \\ 248.90 & 0 & 248.85 & 0 \\ 248.90 & 0 & 248.85 & 0 \\ 248.90 & 0 & 248.95 & 0 \\ 249.00 & 0 & 249.15 & 0 \\ 249.00 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.55 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.45 & 0 \\ 249.90 & 0 & 249.55 & 0 \\ 249.90 & 0 & 249.55 & 0 \\ 249.90 & 0 & 249.55 & 0 \\ 249.90 & 0 & 249.90 & 0 \\ 249.95 & 0 & 249.90 & 0 \\ 249.95 & 0 & 249.90 & 0 \\ 249.95 & 0 & 250.15 & 0 \\ 250.15 & 0 & 250.15 & 0 \\ 250.10 & 0 & 250.20 & 0 \\ 250.10 & 0 & 250.20 & 0 \\ 250.10 & 0 & 250.20 & 0 \\ 250.20 & 0 & 0 \\ 250.20$				
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	Elevation	Storage	Elevation	Storage
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	(feet)		(feet)	(cubic-feet)
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	247.60			0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		0		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$			251.25	0
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	248.65			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	248.70			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	248.75	0		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	248.80			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	248.85	0		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	248.90	0		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	248.95			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		-		
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
249.650249.700249.750249.800249.850249.900249.950250.000250.100250.150				
249.700249.750249.800249.850249.950249.950250.000250.100250.150				
249.75 0 249.80 0 249.85 0 249.90 0 249.95 0 250.00 0 250.10 0 250.15 0				
249.80 0 249.85 0 249.90 0 249.95 0 250.00 0 250.10 0 250.15 0				
249.850249.900249.950250.000250.100250.150				
249.90 0 249.95 0 250.00 0 250.10 0 250.15 0				
249.95 0 250.00 0 250.05 0 250.10 0 250.15 0				
250.00 0 250.05 0 250.10 0 250.15 0				
250.05 0 250.10 0 250.15 0				
250.10 0 250.15 0				
250.15 0				
250.20 0	250.15	0		
I	250.20	0		
			l	

Stage-Area-Storage for Pond EX: Existing Abutter Depression

	.	01		o (<u>.</u>
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
249.80	164 184	0 2	250.33	1,835	456 475
249.81 249.82	204	2 4	250.34 250.35	1,879 1,923	475 494
249.82	204	4	250.35	1,923	494 513
249.83	224	8	250.30	2,012	533
249.85	264	11	250.37	2,012	553
249.86	283	13	250.38	2,000	574
249.87	303	16	250.39	2,144	595
249.88	323	10	250.40	2,193	617
249.89	343	23	250.42	2,242	639
249.90	363	26	250.43	2,291	662
249.91	383	30	250.44	2,341	685
249.92	403	34	250.45	2,390	709
249.93	423	38	250.46	2,439	733
249.94	443	42	250.47	2,488	758
249.95	463	47	250.48	2,537	783
249.96	482	52	250.49	2,586	808
249.97	502	57	250.50	2,636	834
249.98	522	62	250.51	2,685	861
249.99	542	67	250.52	2,734	888
250.00	562	73	250.53	2,783	916
250.01	597	78	250.54	2,832	944
250.02	632	85	250.55	2,881	972
250.03	667	91	250.56	2,930	1,001
250.04	702	98	250.57	2,980	1,031
250.05 250.06	737 772	105 113	250.58 250.59	3,029 3,078	1,061 1,091
250.00	807	121	250.60	3,078 3,127	1,091 1,122
250.07	842	121	230.00	3,127	1,122
250.00	877	137			
250.00	912	146			
250.11	946	156			
250.12	981	165			
250.13	1,016	175			
250.14	1,051	186			
250.15	1,086	196			
250.16	1,121	207			
250.17	1,156	219			
250.18	1,191	230			
250.19	1,226	242			
250.20	1,261	255			
250.21	1,305	268			
250.22	1,349	281			
250.23	1,393	295			
250.24 250.25	1,438 1,482	309 323			
250.25	1,526	339			
250.20	1,570	354			
250.27	1,614	370			
250.20	1,658	386			
250.30	1,703	403			
250.31	1,747	420			
250.32	1,791	438			
			l		

Stage-Area-Storage for Pond HW2:

Elevation	Storage	Elevation	Storage	Elevation	Storage
	ubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
253.00	0	253.53	0	254.06	0
253.01	0	253.54	0	254.07	0
253.02	0	253.55	0	254.08	0
253.03	0	253.56	0	254.09	0
253.04	0	253.57	0	254.10	0
253.05	0	253.58	0	254.11	0
253.06	0	253.59	0	254.12	0
253.07	0	253.60	0	254.13	0
253.08	0	253.61	0	254.14	0
253.09	0	253.62	0	254.15	0
253.10	0	253.63	0	254.16	0
253.11	0	253.64	0	254.17	0
253.12	0	253.65	0	254.18	0
253.13	0	253.66	0	254.19	0
253.14	0	253.67	0	254.20	0
253.15	0	253.68	Ō	254.21	Ō
253.16	0	253.69	Ō	254.22	0
253.17	0	253.70	0	254.23	0
253.18	Ő	253.71	Ő	254.24	0
253.19	Ő	253.72	Ő	254.25	Ő
253.20	Ő	253.73	0 0		· ·
253.21	Ő	253.74	Ő		
253.22	Ő	253.75	Ő		
253.23	Ő	253.76	Ő		
253.24	Ő	253.77	Õ		
253.25	Ő	253.78	Ő		
253.26	Ő	253.79	Ő		
253.27	Ő	253.80	Ő		
253.28	Ő	253.81	Ő		
253.29	Ő	253.82	0 0		
253.30	Ő	253.83	Ő		
253.31	Ő	253.84	Ő		
253.32	Ő	253.85	Ő		
253.33	0	253.86	Ō		
253.34	Ő	253.87	Ő		
253.35	Ő	253.88	Ő		
253.36	0	253.89	Ō		
253.37	0	253.90	0		
253.38	0	253.91	0		
253.39	0	253.92	Ō		
253.40	Ō	253.93	Ō		
253.41	Ō	253.94	Ō		
253.42	0	253.95	0		
253.43	0	253.96	0		
253.44	Ō	253.97	Ō		
253.45	0	253.98	0		
253.46	0 0	253.99	Ő		
253.47	0	254.00	Ő		
253.48	0	254.01	Ő		
253.49	Ő	254.02	Ő		
253.50	Ő	254.03	Ő		
253.51	0	254.04	Ō		
253.52	0	254.05	0		
		I		I	

Stage-Area-Storage for Pond IB1: Infiltration Basin #1

			·		
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	<u>(sq-ft)</u>	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
247.00	8,532	0	249.65	12,837	28,157
247.05	8,606	428	249.70	12,925	28,801
247.10	8,681	861	249.75	13,014	29,449
247.15	8,755	1,297	249.80	13,103	30,102
247.20	8,830	1,736	249.85	13,192	30,759
247.25	8,906	2,180	249.90	13,281	31,421
247.30	8,981	2,627	249.95	13,371	32,088
247.35	9,057	3,078	250.00	13,461	32,758
247.40	9,134	3,532			
247.45	9,210 9,287	3,991			
247.50 247.55	9,267 9,365	4,454			
247.60	9,303	4,920			
247.65	9,442 9,520	5,390 5,864			
247.05	9,520	6,342			
247.75	9,599 9,677	6,824			
247.80	9,756	7,310			
247.85	9,835	7,800			
247.90	9,915	8,293			
247.95	9,995	8,791			
248.00	10,075	9,293			
248.05	10,154	9,799			
248.10	10,234	10,308			
248.15	10,314	10,822			
248.20	10,394	11,340			
248.25	10,474	11,861			
248.30	10,555	12,387			
248.35	10,636	12,917			
248.40	10,717	13,451			
248.45	10,799	13,989			
248.50	10,881	14,531			
248.55	10,963	15,077			
248.60	11,046	15,627			
248.65	11,129	16,181			
248.70	11,212	16,740			
248.75	11,296	17,302			
248.80	11,379	17,869			
248.85	11,464	18,440			
248.90	11,548	19,016			
248.95	11,633	19,595			
249.00	11,718	20,179			
249.05	11,802	20,767			
249.10	11,887	21,359			
249.15	11,972	21,956			
249.20	12,057	22,556			
249.25	12,142	23,161			
249.30	12,228	23,771			
249.35	12,314	24,384			
249.40	12,401	25,002			
249.45	12,487	25,624			
249.50	12,574	26,251			
249.55	12,662	26,882			
249.60	12,749	27,517			

Stage-Area-Storage for Pond SIS1: Stormtech

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
247.75	1,242	0	250.40	1,242	2,204
247.80	1,242	25	250.45	1,242	2,237
247.85	1,242	50	250.50	1,242	2,269
247.90	1,242	75	250.55	1,242	2,299
247.95	1,242	99	250.60	1,242	2,327
248.00	1,242	124	250.65	1,242	2,354
248.05	1,242	150	250.70	1,242	2,381
248.10	1,242	175	250.75	1,242	2,407
248.15	1,242	201	250.80	1,242	2,432
248.20	1,242	226	250.85	1,242	2,456
248.25	1,242	252	250.90	1,242	2,481
248.30	1,242	302	250.95	1,242	2,506
248.35	1,242	353	251.00	1,242	2,500
248.40	1,242	404	251.00	1,242	2,556
248.45	1,242	404 455	251.05	1,242	2,550
248.50	1,242	505	251.10	1,242	
	1,242	556	251.15	1,242	2,605
248.55 248.60	1,242		251.20		2,630
	1,242	606 656	201.20	1,242	2,655
248.65		656 706			
248.70	1,242	706			
248.75	1,242	755			
248.80	1,242	805			
248.85	1,242	854			
248.90	1,242	903			
248.95	1,242	952			
249.00	1,242	1,000			
249.05	1,242	1,048			
249.10	1,242	1,096			
249.15	1,242	1,144			
249.20	1,242	1,192			
249.25	1,242	1,239			
249.30	1,242	1,286			
249.35	1,242	1,332			
249.40	1,242	1,379			
249.45	1,242	1,424			
249.50	1,242	1,470			
249.55	1,242	1,515			
249.60	1,242	1,560			
249.65	1,242	1,604			
249.70	1,242	1,648			
249.75	1,242	1,692			
249.80	1,242	1,735			
249.85	1,242	1,778			
249.90	1,242	1,820			
249.95	1,242	1,861			
250.00	1,242	1,902			
250.05	1,242	1,942			
250.10	1,242	1,982			
250.15	1,242	2,021			
250.20	1,242	2,059			
250.25	1,242	2,097			
250.30	1,242	2,134			
250.35	1,242	2,169			
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Stage-Area-Storage for Pond SIS2:

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
249.00	1,735	0	251.65	1,748	3,056
249.05	1,735	35	251.70	1,748	3,102
249.10	1,735	69	251.75	1,748	3,146
249.15	1,735	104	251.80	1,748	3,188
249.20	1,735	139	251.85	1,748	3,227
249.25	1,735	174	251.90	1,748	3,265
249.30	1,735	208	251.95	1,748	3,302
249.35	1,735	243	252.00	1,748	3,338
249.40	1,735	278	252.05	1,748	3,373
249.45	1,735	312	252.10	1,748	3,409
249.50	1,735	347	252.15	1,748	3,444
249.55 249.60	1,735 1,735	418 488	252.20	1,748 1,748	3,479
249.65	1,735	400 558	252.25 252.30	1,748	3,515 3,550
249.00	1,735	628	252.30	1,748	3,585
249.75	1,735	698	252.40	1,748	3,621
249.80	1,735	768	252.45	1,748	3,656
249.85	1,735	837	252.50	1,748	3,691
249.90	1,735	907	252.55	1,748	3,692
249.95	1,735	976	252.60	1,748	3,692
250.00	1,735	1,044	252.65	1,748	3,693
250.05	1,735	1,113	252.70	1,748	3,694
250.10	1,735	1,181	252.75	1,748	3,694
250.15	1,735	1,249	252.80	1,748	3,695
250.20	1,735	1,316	252.85	1,748	3,696
250.25	1,735	1,383	252.90	1,748	3,696
250.30	1,735	1,450	252.95	1,748	3,697
250.35	1,735	1,516	253.00	1,748	3,697
250.40	1,735	1,582	253.05	1,748	3,698
250.45 250.50	1,735 1,735	1,648 1,713	253.10 253.15	1,748 1,748	3,699 3,699
250.55	1,735	1,778	253.10	1,748	3,700
250.60	1,735	1,843	253.25	1,748	3,700
250.65	1,735	1,907	253.30	1,748	3,701
250.70	1,735	1,970	253.35	1,748	3,702
250.75	1,735	2,033	253.40	1,748	3,703
250.80	1,748	2,095	253.45	1,748	3,703
250.85	1,748	2,158	253.50	1,748	3,704
250.90	1,748	2,220	253.55	1,748	3,704
250.95	1,748	2,281	253.60	1,748	3,705
251.00	1,748	2,342	253.65	1,748	3,706
251.05	1,748	2,402	253.70	1,748	3,706
251.10	1,748	2,462	253.75	1,748	3,707
251.15	1,748	2,520	253.80	1,748	3,708
251.20	1,748	2,578			
251.25 251.30	1,748 1,748	2,635 2,691			
251.35	1,748	2,031			
251.40	1,748	2,801			
251.45	1,748	2,855			
251.50	1,748	2,907			
251.55	1,748	2,958			
251.60	1,748	3,008			

Stage-Area-Storage for Pond AD:

Flowetien	Characte	Flovetion	Ctore re		Ctowners
Elevation (feet)	Storage (cubic-feet)	Elevation	Storage (cubic-feet)	Elevation	Storage (cubic-feet)
248.60		(feet) 249.66	0	(feet) 250.72	
248.60	0 0	249.68	0	250.72	0 0
248.64	0	249.00	0	230.74	0
248.66	0	249.70	0		
248.68	0	249.72	0		
248.00	0	249.74	0		
248.70	0	249.78	0		
248.72	0	249.70	0		
248.76	0	249.82	0		
248.78	0 0	249.84	0 0		
248.80	Ő	249.86	0 0		
248.82	Ő	249.88	Ő		
248.84	0	249.90	0		
248.86	0	249.92	0		
248.88	0	249.94	0		
248.90	0	249.96	0		
248.92	0	249.98	0		
248.94	0	250.00	0		
248.96	0	250.02	0		
248.98	0	250.04	0		
249.00	0	250.06	0		
249.02	0	250.08	0		
249.04	0	250.10	0		
249.06	0	250.12	0		
249.08	0	250.14	0		
249.10	0	250.16	0		
249.12	0	250.18	0		
249.14	0	250.20	0		
249.16	0	250.22	0		
249.18	0	250.24	0		
249.20	0	250.26	0		
249.22	0	250.28	0		
249.24	0 0	250.30	0 0		
249.26 249.28	0	250.32 250.34	0		
249.20	0	250.34	0		
249.30	0	250.30	0		
249.34	0	250.40	0		
249.36	0	250.42	0		
249.38	0 0	250.44	0 0		
249.40	Ő	250.46	Ő		
249.42	0 0	250.48	0 0		
249.44	0	250.50	0		
249.46	0	250.52	0		
249.48	0	250.54	0		
249.50	0	250.56	0		
249.52	0	250.58	0		
249.54	0	250.60	0		
249.56	0	250.62	0		
249.58	0	250.64	0		
249.60	0	250.66	0		
249.62	0	250.68	0		
249.64	0	250.70	0		
		I		I	

Stage-Area-Storage for Pond CB1: CB#1

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
249.00	0	250.06	0	251.12	0
249.02	0	250.08	0	251.14	0
249.04	0	250.10	0	251.16	0
249.06	0	250.12	0	251.18	0
249.08	0	250.14	0	251.20	0
249.10	0	250.16	0	251.22	0
249.12	0	250.18	0	251.24	0
249.14	0	250.20	0	251.26	0
249.16	0	250.22	0	251.28	0
249.18	0	250.24	0	251.30	0
249.20	0	250.26	0	251.32	0
249.22	0 0	250.28	0 0	251.34 251.36	0
249.24 249.26	0	250.30 250.32	0	251.30	0 0
249.20	0	250.32	0	251.38	0
249.30	0	250.34	0	251.40	0
249.32	0	250.38	0	251.44	0
249.34	Ŭ Ŭ	250.40	Ŭ Ŭ	251.46	Ő
249.36	Ő	250.42	Ő	251.48	0 0
249.38	Ō	250.44	Ō	251.50	Ō
249.40	0	250.46	0		
249.42	0	250.48	0		
249.44	0	250.50	0		
249.46	0	250.52	0		
249.48	0	250.54	0		
249.50	0	250.56	0		
249.52	0	250.58	0		
249.54	0	250.60	0		
249.56	0 0	250.62	0 0		
249.58 249.60	0	250.64 250.66	0		
249.60	0	250.68	0		
249.64	0	250.00	0		
249.66	Ŭ Ŭ	250.72	Ŭ Ŭ		
249.68	Ő	250.74	Ő		
249.70	Ō	250.76	Ō		
249.72	0	250.78	0		
249.74	0	250.80	0		
249.76	0	250.82	0		
249.78	0	250.84	0		
249.80	0	250.86	0		
249.82	0	250.88	0		
249.84	0	250.90	0		
249.86	0	250.92	0		
249.88	0	250.94	0		
249.90	0	250.96	0		
249.92 249.94	0 0	250.98 251.00	0 0		
249.94 249.96	0	251.00	0		
249.90	0	251.02	0		
250.00	0	251.04	0		
250.02	Õ	251.08	Ő		
250.04	0	251.10	0		
		I		l	

Stage-Area-Storage for Pond CB2:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
251.20	0	252.26	0	253.32	0
251.22	0	252.28	0	253.34	0
251.24	0	252.30	0	253.36	0
251.26	0	252.32	0	253.38	0
251.28	0	252.34	0	253.40	0
251.30	0	252.36	0	253.42	0
251.32	0	252.38	0	253.44	0
251.34	0	252.40	0	253.46	0
251.36	0	252.42	0	253.48	0
251.38 251.40	0 0	252.44 252.46	0 0	253.50 253.52	0 0
251.40	0	252.40	0	253.52	0
251.42	0	252.40	0	253.54	0
251.46	0	252.50	0	253.58	0
251.48	0 0	252.54	0 0	253.60	Ő
251.50	0	252.56	Ő	253.62	Ő
251.52	0	252.58	0	253.64	0
251.54	0	252.60	0	253.66	0
251.56	0	252.62	0	253.68	0
251.58	0	252.64	0	253.70	0
251.60	0	252.66	0	253.72	0
251.62	0	252.68	0	253.74	0
251.64	0	252.70	0	253.76	0
251.66 251.68	0 0	252.72 252.74	0 0	253.78 253.80	0 0
251.00	0	252.74	0	253.80	0
251.72	0	252.78	0	253.84	0
251.74	0 0	252.80	Ő	253.86	Ő
251.76	0	252.82	0	253.88	0
251.78	0	252.84	0	253.90	0
251.80	0	252.86	0	253.92	0
251.82	0	252.88	0	253.94	0
251.84	0	252.90	0	253.96	0
251.86	0	252.92	0	253.98	0
251.88	0 0	252.94	0	254.00	0 0
251.90 251.92	0	252.96 252.98	0 0	254.02	0
251.92	0	253.00	0		
251.96	0	253.02	0		
251.98	0 0	253.04	Ő		
252.00	0	253.06	0		
252.02	0	253.08	0		
252.04	0	253.10	0		
252.06	0	253.12	0		
252.08	0	253.14	0		
252.10	0	253.16	0		
252.12	0	253.18	0		
252.14 252.16	0 0	253.20 253.22	0 0		
252.10	0	253.22	0		
252.20	0	253.24	0		
252.22	0 0	253.28	0 0		
252.24	0	253.30	0		
	I			l	

Stage-Area-Storage for Pond CB4:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
247.83	0	248.89	0	249.95	0
247.85	0	248.91	0	249.97	0
247.87	0	248.93	0	249.99	0
247.89	0	248.95	0	250.01	0
247.91	0	248.97	0 0	250.03	0
247.93	0	248.99	0 0	250.05	Ő
247.95	0	249.01	0	250.03	0
247.93	0	249.01	0	250.07	0
247.97					
	0	249.05	0	250.11	0
248.01	0	249.07	0	250.13	0
248.03	0	249.09	0	250.15	0
248.05	0	249.11	0	250.17	0
248.07	0	249.13	0	250.19	0
248.09	0	249.15	0	250.21	0
248.11	0	249.17	0	250.23	0
248.13	0	249.19	0	250.25	0
248.15	0	249.21	0	250.27	0
248.17	0	249.23	0	250.29	0
248.19	0	249.25	0	250.31	0
248.21	0	249.27	0	250.33	0
248.23	0	249.29	0	250.35	0
248.25	0	249.31	0	250.37	0
248.27	0	249.33	Ō	250.39	0
248.29	0	249.35	Ő	250.41	Ő
248.31	0 0	249.37	Ő	250.43	0
248.33	0 0	249.39	0 0	250.45	Ő
248.35	0 0	249.41	0 0	250.47	Ő
248.37	0	249.43	0 0	250.49	Ő
248.39	0	249.45	0	250.51	0
248.41	0	249.47	0	250.53	0
248.43	0	249.49	0	250.55	0
248.45	0	249.49	0	250.55	0
248.47	0	249.53	0	250.59	0
248.49	0	249.55	0	250.61	0
248.51	0	249.57	0	250.63	0
248.53	0	249.59	0	250.65	0
248.55	0	249.61	0	250.67	0
248.57	0	249.63	0	250.69	0
248.59	0	249.65	0		
248.61	0	249.67	0		
248.63	0	249.69	0		
248.65	0	249.71	0		
248.67	0	249.73	0		
248.69	0	249.75	0		
248.71	0	249.77	0		
248.73	0	249.79	0		
248.75	0	249.81	0		
248.77	0	249.83	0		
248.79	0	249.85	0		
248.81	0	249.87	0		
248.83	0	249.89	0		
248.85	0	249.91	0		
248.87	0	249.93	0		
	l			l	

Stage-Area-Storage for Pond CB5:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
250.30	0	251.36	0	252.42	0
250.32	0	251.38	Õ	252.44	ů 0
250.34	Ő	251.40	Ő	252.46	0
250.36	0	251.42	0	252.48	0
250.38	0	251.44	0	252.50	0
250.40	0	251.46	0	252.52	0
250.42	0	251.48	0	252.54	0
250.44	0	251.50	0	252.56	0
250.46	0	251.52	0	252.58	0
250.48	0	251.54	0	252.60	0
250.50	0	251.56	0	252.62	0
250.52	0	251.58	0		
250.54 250.56	0 0	251.60 251.62	0 0		
250.58	0	251.62	0		
250.60	0	251.66	0		
250.62	0 0	251.68	0 0		
250.64	Õ	251.70	Ő		
250.66	Ő	251.72	Ő		
250.68	0	251.74	0		
250.70	0	251.76	0		
250.72	0	251.78	0		
250.74	0	251.80	0		
250.76	0	251.82	0		
250.78	0	251.84	0		
250.80	0	251.86	0		
250.82	0	251.88	0		
250.84	0	251.90	0		
250.86	0	251.92	0		
250.88 250.90	0 0	251.94 251.96	0 0		
250.90	0	251.90	0		
250.92	0	252.00	0		
250.96	Õ	252.02	Ő		
250.98	Ō	252.04	Ō		
251.00	0	252.06	0		
251.02	0	252.08	0		
251.04	0	252.10	0		
251.06	0	252.12	0		
251.08	0	252.14	0		
251.10	0	252.16	0		
251.12	0	252.18	0		
251.14	0	252.20	0 0		
251.16 251.18	0 0	252.22 252.24	0		
251.18	0	252.24	0		
251.20	0	252.28	0		
251.22	0 0	252.30	0 0		
251.24	Ŭ Ŭ	252.32	0		
251.28	Ō	252.34	Ō		
251.30	0	252.36	0		
251.32	0	252.38	0		
251.34	0	252.40	0		
		I		I	

Stage-Area-Storage for Pond CB6:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
248.30	0	249.36	0	250.42	0
248.32	0	249.38	0	250.42	0
248.34	0 0	249.40	0	250.46	Ő
248.36	Ő	249.42	ů 0	250.48	Ő
248.38	0 0	249.44	0	250.50	ů 0
248.40	0 0	249.46	0	250.52	Ő
248.42	Ő	249.48	ů 0	250.54	Ő
248.44	Ő	249.50	ů 0	250.56	Ő
248.46	Ő	249.52	0 0	250.58	Ő
248.48	0	249.54	0	250.60	Ō
248.50	0	249.56	0	250.62	0
248.52	0	249.58	0	250.64	0
248.54	0	249.60	0	250.66	0
248.56	0	249.62	0	250.68	0
248.58	0	249.64	0	250.70	0
248.60	0	249.66	0	250.72	0
248.62	0	249.68	0	250.74	0
248.64	0	249.70	0	250.76	0
248.66	0	249.72	0	250.78	0
248.68	0	249.74	0	250.80	0
248.70	0	249.76	0	250.82	0
248.72	0	249.78	0		
248.74	0	249.80	0		
248.76	0	249.82	0		
248.78	0	249.84	0		
248.80	0	249.86	0		
248.82	0	249.88	0		
248.84	0	249.90	0		
248.86	0	249.92	0		
248.88	0	249.94	0		
248.90	0	249.96	0		
248.92	0	249.98	0		
248.94	0	250.00 250.02	0 0		
248.96 248.98	0 0	250.02	0		
240.90	0	250.04	0		
249.00	0	250.00	0		
249.02	0	250.00	0		
249.04	0	250.10	0		
249.08	0	250.12	0		
249.10	Ő	250.16	ů 0		
249.12	Ő	250.18	0 0		
249.14	0 0	250.20	0		
249.16	Ō	250.22	0		
249.18	0	250.24	0		
249.20	0	250.26	0		
249.22	0	250.28	0		
249.24	0	250.30	0		
249.26	0	250.32	0		
249.28	0	250.34	0		
249.30	0	250.36	0		
249.32	0	250.38	0		
249.34	0	250.40	0		
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Stage-Area-Storage for Pond CB7:

ElevationStorage (cubic-feet)ElevationStorage (cubic-feet) 248.00 0 250.65 0 248.05 0 250.75 0 248.15 0 250.75 0 248.15 0 250.85 0 248.25 0 250.95 0 248.30 0 251.00 0 248.35 0 251.00 0 248.45 0 251.10 0 248.45 0 251.15 0 248.45 0 251.15 0 248.65 0 251.20 0 248.65 0 251.30 0 248.65 0 251.30 0 248.65 0 251.30 0 248.65 0 251.35 0 248.65 0 251.60 0 248.80 0 251.60 0 248.90 0 251.65 0 248.95 0 251.60 0 249.05 0 251.70 0 249.05 0 251.70 0 249.05 0 251.80 0 249.05 0 252.10 0 249.45 0 252.20 0 249.45 0 252.25 0 249.45 0 252.45 0 249.95 0 252.45 0 249.95 0 252.45 0 249.95 0 252.45 0 249.95 0 252.45 0 249.95 0		-	I — · · ·	•
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248.050 250.70 0 248.10 0 250.85 0 248.15 0 250.85 0 248.20 0 250.85 0 248.25 0 250.95 0 248.35 0 251.05 0 248.40 0 251.05 0 248.45 0 251.15 0 248.45 0 251.25 0 248.45 0 251.25 0 248.65 0 251.25 0 248.65 0 251.35 0 248.65 0 251.40 0 248.75 0 251.55 0 248.85 0 251.55 0 248.85 0 251.55 0 248.85 0 251.55 0 248.90 0 251.75 0 248.95 0 251.75 0 249.05 0 251.75 0 249.05 0 251.75 0 249.05 0 251.75 0 249.05 0 251.75 0 249.05 0 251.75 0 249.05 0 251.75 0 249.35 0 252.45 0 249.45 0 252.25 0 249.45 0 252.25 0 249.45 0 252.45 0 249.85 0 252.45 0 249.95 0 252.45 0 249.95 0 252.45 0 249.95				
248.100 250.75 0 248.15 0 250.85 0 248.20 0 250.95 0 248.25 0 250.95 0 248.30 0 251.05 0 248.35 0 251.10 0 248.40 0 251.15 0 248.45 0 251.10 0 248.55 0 251.10 0 248.60 0 251.25 0 248.65 0 251.25 0 248.70 0 251.45 0 248.75 0 251.45 0 248.80 0 251.45 0 248.80 0 251.45 0 248.90 0 251.65 0 248.95 0 251.60 0 248.95 0 251.60 0 249.00 0 251.65 0 249.05 0 251.70 0 249.05 0 251.85 0 249.05 0 251.85 0 249.20 0 251.85 0 249.35 0 252.05 0 249.45 0 252.05 0 249.45 0 252.25 0 249.60 0 252.25 0 249.75 0 252.45 0 249.85 0 252.45 0 249.95 0 252.45 0 249.95 0 252.65 0 249.95 0 252.65 0 250.15		-		
248.150 250.80 0 248.20 0 250.95 0 248.25 0 250.95 0 248.30 0 250.95 0 248.35 0 251.00 0 248.45 0 251.15 0 248.45 0 251.15 0 248.55 0 251.15 0 248.65 0 251.20 0 248.65 0 251.35 0 248.70 0 251.35 0 248.75 0 251.40 0 248.85 0 251.55 0 248.85 0 251.55 0 248.90 0 251.55 0 248.90 0 251.55 0 248.95 0 251.75 0 249.05 0 251.75 0 249.05 0 251.75 0 249.05 0 251.75 0 249.10 0 251.75 0 249.25 0 251.80 0 249.35 0 251.95 0 249.45 0 252.15 0 249.45 0 252.20 0 249.65 0 252.25 0 249.65 0 252.40 0 249.75 0 252.40 0 249.95 0 252.75 0 249.95 0 252.75 0 249.95 0 252.75 0 249.95 0 252.75 0 250.10				
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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				
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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	249.90	0	252.55	0
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250.100252.750250.150252.800250.200252.850250.250252.900250.300252.950250.350253.000250.400250.450250.500250.550				
250.150252.800250.200252.850250.250252.900250.300252.950250.350253.000250.400250.450250.500250.550				
250.200252.850250.250252.900250.300252.950250.350253.000250.400250.450250.500250.550				
250.250252.900250.300252.950250.350253.000250.400250.450250.500250.550				
250.300252.950250.350253.000250.400250.450250.500250.550				
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250.50 0 250.55 0				
250.55 0				
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Stage-Area-Storage for Pond DMH1:

Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)
248.28	0	250.93	0
248.33	0	250.98	0
248.38	0	251.03	0
248.43	0	251.08	0
248.48	0	251.13	0
248.53	Ő	251.18	Ő
248.58	Ő	251.23	Ũ
248.63	Ő	251.28	Ő
248.68	Ő	251.33	Ő
248.73	0 0	251.38	0
248.78	0 0	251.43	0
248.83	0	251.48	0
248.88	0	251.53	0
248.93	0	251.53	0
248.93	0	251.63	0
249.03	0	251.68	0
249.08	0	251.73	0
249.13	0	251.78	0
249.18	0	251.83	0
249.23	0	251.88	0
249.28	0	251.93	0
249.33	0	251.98	0
249.38	0	252.03	0
249.43	0	252.08	0
249.48	0	252.13	0
249.53	0	252.18	0
249.58	0	252.23	0
249.63	0	252.28	0
249.68	0	252.33	0
249.73	0	252.38	0
249.78	0	252.43	0
249.83	0	252.48	0
249.88	0		
249.93	0		
249.98	0		
250.03	0		
250.08	0		
250.13	0		
250.18	0		
250.23	0		
250.28	0		
250.33	0		
250.38	0		
250.43	Ō		
250.48	0		
250.53	0		
250.58	Ő		
250.63	Ő		
250.68	Ő		
250.00	0 0		
250.78	0 0		
250.83	0		
250.83	0		
200.00	0		

Type III 24-hr 100-yr Rainfall=8.86" Printed 4/11/2022 Page 27

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Stage-Area-Storage for Pond DMH3:

Flovation	Storage		Storage
Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
247.45	0	250.10	0
247.50	0	250.15	0
247.55	0	250.20	0
247.60	0	250.25	0
247.65	0	250.30	0
247.70 247.75	0 0	250.35 250.40	0 0
247.75	0	250.40	0
247.85	0 0	250.50	0
247.90	Ő	250.55	0 0
247.95	0	250.60	0
248.00	0	250.65	0
248.05	0	250.70	0
248.10 248.15	0 0	250.75 250.80	0 0
248.15	0	250.80	0
248.25	Õ	250.90	0 0
248.30	0	250.95	0
248.35	0	251.00	0
248.40	0	251.05	0
248.45 248.50	0 0	251.10 251.15	0 0
248.50	0	251.15	0
248.60	0 0	251.25	0
248.65	0	251.30	0
248.70	0	251.35	0
248.75	0	251.40	0
248.80	0	251.45	0 0
248.85 248.90	0 0	251.50 251.55	0
248.95	0	251.60	0
249.00	Ő	251.65	Ő
249.05	0	251.70	0
249.10	0	251.75	0
249.15	0	251.80	0
249.20 249.25	0 0	251.85 251.90	0 0
249.20	0	251.90	0
249.35	Ő	201.00	0
249.40	0		
249.45	0		
249.50	0		
249.55 249.60	0 0		
249.60	0		
249.70	ů 0		
249.75	0		
249.80	0		
249.85	0		
249.90 249.95	0 0		
249.95	0		
250.05	Ő		
		l	

Stage-Area-Storage for Pond DMH4:

Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)
246.60	0	249.25	0
246.65	0	249.30	0
246.70	0	249.35	0
246.75	0	249.40	0
246.80	0	249.45	0
246.85	0	249.50	0
246.90	0	249.55	0
246.95 247.00	0 0	249.60 249.65	0
247.00	0	249.05	0 0
247.00	0	249.70	0
247.10	0	249.80	0
247.10	0	249.85	0
247.25	Ő	249.90	0
247.30	Ő	249.95	0 0
247.35	Ő	250.00	0 0
247.40	Ő	250.05	0
247.45	0	250.10	0
247.50	0	250.15	0
247.55	0	250.20	0
247.60	0	250.25	0
247.65	0	250.30	0
247.70	0	250.35	0
247.75	0	250.40	0
247.80	0	250.45	0
247.85	0	250.50	0
247.90	0	250.55	0
247.95	0	250.60	0
248.00	0	250.65	0
248.05	0	250.70	0
248.10	0	250.75	0
248.15 248.20	0	250.80 250.85	0
248.20 248.25	0 0	250.85	0 0
248.30	0	250.90	0
248.35	0	251.00	0
248.40	0	201.00	0
248.45	ů 0		
248.50	Ő		
248.55	0		
248.60	0		
248.65	0		
248.70	0		
248.75	0		
248.80	0		
248.85	0		
248.90	0		
248.95	0		
249.00	0		
249.05	0		
249.10	0		
249.15	0 0		
249.20	U		
		•	

Stage-Area-Storage for Pond DMH5:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
251.65	0	252.71	0	253.77	0
251.67	0	252.73	Ő	253.79	ů 0
251.69	Ő	252.75	Ő	253.81	ů 0
251.71	0 0	252.77	Ő	253.83	ů 0
251.73	0 0	252.79	0 0	253.85	0
251.75	0 0	252.81	Ő	253.87	0
251.77	Ő	252.83	Ő	253.89	ů 0
251.79	Ő	252.85	Ő	253.91	ů 0
251.81	Ő	252.87	Ő	253.93	ů 0
251.83	Ő	252.89	Ő	253.95	0
251.85	0	252.91	0	253.97	0
251.87	0	252.93	0	253.99	0
251.89	0	252.95	0		
251.91	0	252.97	0		
251.93	0	252.99	0		
251.95	0	253.01	0		
251.97	0	253.03	0		
251.99	0	253.05	0		
252.01	0	253.07	0		
252.03	0	253.09	0		
252.05	0	253.11	0		
252.07	0	253.13	0		
252.09	0	253.15	0		
252.11	0	253.17	0		
252.13	0	253.19	0		
252.15	0	253.21	0		
252.17	0	253.23	0		
252.19	0	253.25	0		
252.21	0	253.27	0		
252.23	0	253.29	0		
252.25	0	253.31	0		
252.27	0	253.33	0		
252.29	0	253.35	0		
252.31	0	253.37	0		
252.33	0	253.39	0		
252.35	0	253.41	0		
252.37	0	253.43	0		
252.39	0	253.45	0		
252.41	0	253.47	0		
252.43	0	253.49	0		
252.45	0	253.51	0		
252.47	0	253.53	0		
252.49	0	253.55	0		
252.51	0	253.57	0		
252.53	0	253.59	0		
252.55	0	253.61	0		
252.57	0	253.63	0		
252.59	0	253.65	0		
252.61	0	253.67	0		
252.63	0	253.69	0		
252.65	0	253.71	0		
252.67	0 0	253.73	0 0		
252.69	U	253.75	U		
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Stage-Area-Storage for Pond DMH6:

Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)
249.71	0	252.36	0
249.76	0	252.41	0
249.81	0	252.46	0
249.86	0	252.51	0
249.91	0	252.56	0
249.96	0	252.61	0
250.01	0	252.66	0
250.06	0	252.71	0
250.11	0	252.76	0
250.16	0	252.81	0
250.21	0	252.86	0 0
250.26	0	252.91	0
250.31 250.36	0 0		
250.30	0		
250.41	0		
250.51	0		
250.56	0		
250.61	Ő		
250.66	Ő		
250.71	0		
250.76	0		
250.81	0		
250.86	0		
250.91	0		
250.96	0		
251.01	0		
251.06	0		
251.11	0		
251.16	0		
251.21	0		
251.26	0		
251.31	0		
251.36	0		
251.41 251.46	0 0		
251.51	0		
251.56	0		
251.61	0		
251.66	0 0		
251.71	Ő		
251.76	0		
251.81	0		
251.86	0		
251.91	0		
251.96	0		
252.01	0		
252.06	0		
252.11	0		
252.16	0		
252.21	0		
252.26	0		
252.31	0		

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Stage-Area-Storage for Pond DMH7:

Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)
247.60	0	250.25	0
247.65	0	250.30	0
247.70	0	250.35	0
247.75	Ō	250.40	0
247.80	Ő	250.45	0
247.85	Ő	250.50	0
247.90	0 0	250.55	0
247.95	0 0	250.60	0
248.00	0	250.65	0
248.05	0	250.05	0
248.10	0	250.75	0
248.10	0	250.75	0
248.20	0	250.85	
	0		0
248.25		250.90	0
248.30	0	250.95	0
248.35	0	251.00	0
248.40	0	251.05	0
248.45	0	251.10	0
248.50	0	251.15	0
248.55	0	251.20	0
248.60	0	251.25	0
248.65	0		
248.70	0		
248.75	0		
248.80	0		
248.85	0		
248.90	0		
248.95	0		
249.00	0		
249.05	0		
249.10	0		
249.15	0		
249.20	0		
249.25	0		
249.30	0		
249.35	0		
249.40	0		
249.45	0		
249.50	0		
249.55	0		
249.60	Ő		
249.65	Ő		
249.70	Ő		
249.75	Ő		
249.80	Ő		
249.85	Ő		
249.90	0 0		
249.95	Ő		
250.00	Ő		
250.05	Ő		
250.00	0 0		
250.10	0		
250.10	0		
200.20	U		
	•		

Stage-Area-Storage for Pond EX: Existing Abutter Depression

			· _·		-
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
249.80	164	0	250.33	1,835	456
249.81	184	2	250.34	1,879	475
249.82	204	4	250.35	1,923	494
249.83	224	6	250.36	1,967	513
249.84	244	8	250.37	2,012	533
249.85	264	11	250.38	2,056	553
249.86	283	13	250.39	2,100	574
249.87	303	16	250.40	2,144	595
249.88	323	19	250.41	2,193	617
249.89	343	23	250.42	2,242	639
249.90	363	26	250.43	2,291	662
249.91	383	30	250.44	2,341	685
249.92	403	34	250.45	2,390	709
249.93	423	38	250.46	2,439	733
249.94	443	42	250.47	2,488	758
249.95	463	47	250.48	2,537	783
249.96	482	52	250.49	2,586	808
249.97	502	57	250.50	2,636	834
249.98	522	62	250.51	2,685	861
249.99	542	67	250.52	2,734	888
250.00	562	73	250.53	2,783	916
250.01	597	78	250.54	2,832	944
250.02	632	85	250.55	2,881	972
250.03	667	91	250.56	2,930	1,001
250.04	702	98	250.57	2,980	1,031
250.05	737	105	250.58	3,029	1,061
250.06	772	113	250.59	3,078	1,091
250.07	807	121	250.60	3,127	1,122
250.08	842	129			
250.09	877	137			
250.10	912	146			
250.11	946	156			
250.12	981	165			
250.13	1,016	175			
250.14	1,051	186			
250.15	1,086	196			
250.16	1,121	207			
250.17	1,156	219			
250.18	1,191	230			
250.19	1,226	242			
250.20	1,261	255			
250.21	1,305	268			
250.22	1,349	281			
250.23	1,393	295			
250.24	1,438	309			
250.25	1,482	323			
250.26	1,526	339			
250.27	1,570	354			
250.28	1,614	370			
250.29	1,658	386			
250.30	1,703	403			
250.31	1,747	420			
250.32	1,791	438			
			•		

Stage-Area-Storage for Pond HW2:

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
253.00	0	253.53	0	254.06	0
253.01	0	253.54	0	254.07	0
253.02	0	253.55	0	254.08	0
253.03	0	253.56	0	254.09	0
253.04	0	253.57	0	254.10	0
253.05	0	253.58	0	254.11	0
253.06	0	253.59	0	254.12	0
253.07	0	253.60	0	254.13	0
253.08	0	253.61	0	254.14	0
253.09	0	253.62	0	254.15	0
253.10	0	253.63	0	254.16	0
253.11	0	253.64	0	254.17	0
253.12	0	253.65	0	254.18	0
253.13	0	253.66	0	254.19	0
253.14	0	253.67	0	254.20	0
253.15	0	253.68	0	254.21	0
253.16	0	253.69	0	254.22	0
253.17	0	253.70	0	254.23	0
253.18	0	253.71	0	254.24	0
253.19	0	253.72	0	254.25	0
253.20	0	253.73	0		
253.21	0	253.74	0		
253.22	0	253.75	0		
253.23	0	253.76	0		
253.24	0	253.77	0 0		
253.25	0 0	253.78 253.79	0		
253.26 253.27	0	253.79	0		
253.27	0	253.80	0		
253.20	0	253.81	0		
253.30	0	253.82	0		
253.31	0	253.84	0		
253.32	0	253.85	0		
253.33	Õ	253.86	Ő		
253.34	Ő	253.87	Ő		
253.35	0 0	253.88	0 0		
253.36	0	253.89	0		
253.37	0	253.90	0		
253.38	0	253.91	0		
253.39	0	253.92	0		
253.40	0	253.93	0		
253.41	0	253.94	0		
253.42	0	253.95	0		
253.43	0	253.96	0		
253.44	0	253.97	0		
253.45	0	253.98	0		
253.46	0	253.99	0		
253.47	0	254.00	0		
253.48	0	254.01	0		
253.49	0	254.02	0		
253.50	0	254.03	0		
253.51	0	254.04	0		
253.52	0	254.05	0		
		•		•	

Elevation Surface Storage Elevation Surface Storage (feet) (cubic-feet) (feet) (sq-ft) (cubic-feet) (sq-ft) 247.00 8,532 249.65 12,837 28,157 0 12,925 247.05 8,606 428 249.70 28,801 247.10 8,681 861 13,014 29,449 249.75 247.15 8,755 1,297 13,103 30,102 249.80 8,830 1,736 13,192 30,759 247.20 249.85 247.25 8,906 2,180 249.90 13,281 31,421 247.30 8,981 2,627 249.95 13,371 32,088 247.35 9,057 3,078 250.00 13,461 32,758 247.40 9,134 3,532 247.45 9,210 3,991 247.50 9,287 4,454 247.55 9,365 4,920 247.60 9,442 5,390 247.65 9,520 5,864 247.70 9,599 6,342 6,824 247.75 9,677 247.80 9,756 7,310 247.85 9,835 7,800 247.90 9,915 8,293 247.95 9,995 8,791 248.00 10,075 9,293 248.05 10,154 9,799 10,234 10,308 248.10 248.15 10,314 10,822 10,394 11,340 248.20 10,474 11,861 248.25 12,387 248.30 10,555 12,917 248.35 10.636 248.40 10,717 13,451 248.45 10,799 13.989 14,531 248.50 10,881 248.55 10,963 15,077 11,046 15,627 248.60 16,181 248.65 11,129 16,740 248.70 11,212 17,302 248.75 11,296 248.80 11,379 17,869 18,440 248.85 11,464 248.90 11,548 19,016 248.95 11,633 19,595 249.00 11,718 20,179 249.05 11.802 20,767 21,359 249.10 11,887 249.15 11,972 21,956 12,057 22,556 249.20 249.25 12,142 23,161 12,228 23,771 249.30 24,384 249.35 12,314 25,002 12,401 249.40 25,624 249.45 12,487 249.50 12,574 26,251 249.55 12,662 26,882 12,749 249.60 27,517

Stage-Area-Storage for Pond IB1: Infiltration Basin #1

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			-		
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
247.75	1,242	0	250.40	1,242	2,204
247.80	1,242	25	250.45	1,242	2,237
247.85	1,242	50	250.50	1,242	2,269
247.90	1,242	75	250.55	1,242	2,299
247.95	1,242	99	250.60	1,242	2,327
248.00	1,242	124	250.65	1,242	2,354
248.05	1,242	150	250.70	1,242	2,381
248.10	1,242	175	250.75	1,242	2,407
248.15	1,242	201	250.80	1,242	2,432
248.20	1,242	226	250.85	1,242	2,456
248.25	1,242	252	250.90	1,242	2,481
248.30	1,242	302	250.95	1,242	2,506
248.35	1,242	353	251.00	1,242	2,531
248.40	1,242	404	251.05	1,242	2,556
248.45	1,242	455	251.10	1,242	2,581
248.50	1,242	505	251.15	1,242	2,605
248.55	1,242	556	251.20	1,242	2,630
248.60	1,242	606	251.25	1,242	2,655
248.65	1,242	656			
248.70	1,242	706			
248.75	1,242	755			
248.80	1,242	805			
248.85	1,242	854			
248.90	1,242	903			
248.95	1,242	952			
249.00	1,242	1,000			
249.05	1,242	1,048			
249.10	1,242	1,096			
249.15	1,242	1,144			
249.20	1,242	1,192			
249.25	1,242	1,239			
249.30	1,242	1,286			
249.35	1,242	1,332			
249.40	1,242	1,379			
249.45	1,242	1,424			
249.50	1,242	1,470			
249.55	1,242	1,515			
249.60	1,242	1,560			
249.65	1,242	1,604			
249.70	1,242	1,648			
249.75	1,242	1,692			
249.80	1,242	1,735			
249.85 249.90	1,242 1,242	1,778 1,820			
249.90	1,242	1,861			
250.00	1,242	1,902			
250.00	1,242	1,902			
250.00	1,242	1,942			
250.10	1,242	2,021			
250.20	1,242	2,021			
250.25	1,242	2,000			
250.30	1,242	2,007			
250.35	1,242	2,169			
	,	_,			

Stage-Area-Storage for Pond SIS1: Stormtech

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Stage-Area-Storage for Pond SIS2:

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
249.00	1,735	0	251.65	1,748	3,056
249.05	1,735	35	251.70	1,748	3,102
249.10	1,735	69	251.75	1,748	3,146
249.15	1,735	104	251.80	1,748	3,188
249.20	1,735	139	251.85	1,748	3,227
249.25	1,735	174	251.90	1,748	3,265
249.30	1,735	208	251.95	1,748	3,302
249.35	1,735	243	252.00	1,748	3,338
249.40	1,735	278	252.05	1,748	3,373
249.45	1,735	312	252.10	1,748	3,409
249.50	1,735	347	252.15	1,748	3,444
249.55 249.60	1,735 1,735	418 488	252.20 252.25	1,748 1,748	3,479 3,515
249.60	1,735	400 558	252.25	1,748	3,515
249.00	1,735	628	252.35	1,748	3,585
249.75	1,735	698	252.40	1,748	3,621
249.80	1,735	768	252.45	1,748	3,656
249.85	1,735	837	252.50	1,748	3,691
249.90	1,735	907	252.55	1,748	3,692
249.95	1,735	976	252.60	1,748	3,692
250.00	1,735	1,044	252.65	1,748	3,693
250.05	1,735	1,113	252.70	1,748	3,694
250.10	1,735	1,181	252.75	1,748	3,694
250.15	1,735	1,249	252.80	1,748	3,695
250.20	1,735	1,316	252.85	1,748	3,696
250.25	1,735	1,383	252.90	1,748	3,696
250.30 250.35	1,735 1,735	1,450 1,516	252.95 253.00	1,748 1,748	3,697 3,697
250.33	1,735	1,582	253.00	1,748	3,698
250.45	1,735	1,648	253.10	1,748	3,699
250.50	1,735	1,713	253.15	1,748	3,699
250.55	1,735	1,778	253.20	1,748	3,700
250.60	1,735	1,843	253.25	1,748	3,701
250.65	1,735	1,907	253.30	1,748	3,701
250.70	1,735	1,970	253.35	1,748	3,702
250.75	1,735	2,033	253.40	1,748	3,703
250.80	1,748	2,095	253.45	1,748	3,703
250.85	1,748	2,158	253.50	1,748	3,704
250.90	1,748	2,220	253.55	1,748	3,704
250.95	1,748	2,281	253.60	1,748	3,705
251.00 251.05	1,748 1,748	2,342 2,402	253.65 253.70	1,748 1,748	3,706 3,706
251.00	1,748	2,402	253.75	1,748	3,700
251.15	1,748	2,520	253.80	1,748	3,708
251.20	1,748	2,578	200.00	1,1 10	0,100
251.25	1,748	2,635			
251.30	1,748	2,691			
251.35	1,748	2,747			
251.40	1,748	2,801			
251.45	1,748	2,855			
251.50	1,748	2,907			
251.55	1,748	2,958			
251.60	1,748	3,008			



SUPPLEMENTAL DATA REPORT Sheldon Meadow Development – 20 Hancock Street & 1139 West Street April 2022

Appendix I – Mounding Calculations

Sheldon Meadow - Infiltration Basin 1

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

	use consistent units (e.g. feet & days or inches & hours)	Conversion Tab inch/hour fe	et/day
R	Recharge (infiltration) rate (feet/day)	0.67	1.33
Sy	Specific yield, Sy (dimensionless, between 0 and 1)		
к	Horizontal hydraulic conductivity, Kh (feet/day)*	2.00	4.00 In the report accompanying this spreadsheet
х	1/2 length of basin (x direction, in feet)		(USGS SIR 2010-5102), vertical soil permeability
У	1/2 width of basin (y direction, in feet)	hours d	ays (ft/d) is assumed to be one-tenth horizontal
t	duration of infiltration period (days)	36	1.50 hydraulic conductivity (ft/d).
hi(0)	initial thickness of saturated zone (feet)		

15.634 h(max) 0.634 Δh(max) Ground- Distance from

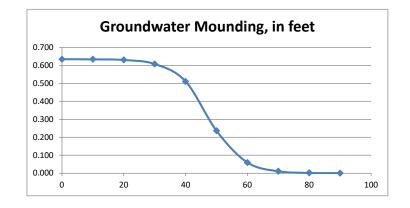
Input Values 0.1650 0.260 1.65 47.500 45.000 1.000 15.000

water center of basin Mounding, in in x direction, in feet feet

icci		icci	
	0.634	0	
	0.634	10	
	0.631	20	
	0.608	30	
	0.511	40	
	0.236	50	
	0.059	60	
	0.011	70	
	0.001	80	
	0.000	90	

maximum thickness of saturated zone (beneath center of basin at end of infiltration period) maximum groundwater mounding (beneath center of basin at end of infiltration period)

Re-Calculate Now



Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

Sheldon Meadow - Subsurface Infiltration System 1

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

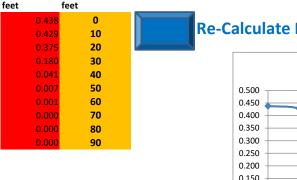
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	use consistent units (e.g. feet & days or inches & hours)	Conversion Tabl inch/hour fee	e et/day
R	Recharge (infiltration) rate (feet/day)	0.67	1.33
Sy	Specific yield, Sy (dimensionless, between 0 and 1)		
5 K	Horizontal hydraulic conductivity, Kh (feet/day)*	2.00	4.00 In the report accompanying this spreadsheet
x C	1/2 length of basin (x direction, in feet)		(USGS SIR 2010-5102), vertical soil permeability
y y	1/2 width of basin (y direction, in feet)	hours da	
) t	duration of infiltration period (days)	36	1.50 hydraulic conductivity (ft/d).
) hi(0)	initial thickness of saturated zone (feet)		

h(max) 15.438 0.438 Δh(max) Ground-Distance from

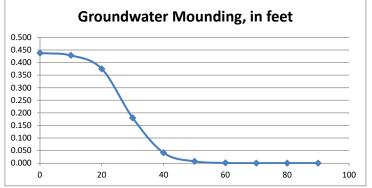
Input Values 0.1650 0.260 1.65 28.450 9.250 1.000 15.000

water center of basin Mounding, in in x direction, in



maximum thickness of saturated zone (beneath center of basin at end of infiltration period) maximum groundwater mounding (beneath center of basin at end of infiltration period)

Re-Calculate Now



Disclaimer

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Sheldon Meadow - Subsurface Infiltration System 2

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

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	use consistent units (e.g. feet & days or inches & hours)	Conversion Ta inch/hour f	Table feet/day	
R	Recharge (infiltration) rate (feet/day)	0.67	1.33	
Sy	Specific yield, Sy (dimensionless, between 0 and 1)			
<mark>б</mark> К	Horizontal hydraulic conductivity, Kh (feet/day)*	2.00) 4.00 In the report accompanying this spreadsheet	
) x	1/2 length of basin (x direction, in feet)		(USGS SIR 2010-5102), vertical soil permeability	v
) y	1/2 width of basin (y direction, in feet)	hours d	days (ft/d) is assumed to be one-tenth horizontal	'
) t	duration of infiltration period (days)	36	1.50 hydraulic conductivity (ft/d).	
) hi(0)	initial thickness of saturated zone (feet)			

15.569 h(max) 0.569 Δh(max) Ground- Distance from

Input Values 0.1650 0.260 1.65 17.800 23.500 1.000 15.000

water center of basin Mounding, in in x direction, in feet feet

 Non-state
 Feet

 0.569
 0

 0.553
 5

 0.500
 10

 0.396
 15

 0.237
 20

 0.123
 25

 0.059
 30

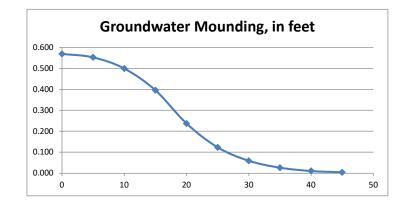
 0.026
 35

 0.011
 40

 0.004
 45

maximum thickness of saturated zone (beneath center of basin at end of infiltration period) maximum groundwater mounding (beneath center of basin at end of infiltration period)

Re-Calculate Now



Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.