

SUPPLEMENTAL DATA REPORT

Sheldon West

1139 West Street and 20 Hancock Street

Wrentham, Massachusetts



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Table of Contents

Existing Conditions1
Proposed Conditions1
Zoning
Dimensional Requirements
Senior Living Community – Special Permit Criteria4
Hydrology9
Standard 1. No New Untreated Discharges9
Standard 2. Post-development Peak Discharge Rates Not to Exceed Pre-development Peak Discharge Rates
Standard 3. Minimize or Eliminate Loss of Annual Recharge to Groundwater
Standard 4. Stormwater Management System to Remove 80% of the Average Annual Load of Total Suspended Solids (TSS)
Standard 5. Land Uses with Higher Potential Pollutant Loads
Standard 6. Stormwater Discharges to Critical Areas
Standard 7. Redevelopment Projects
Standard 8. Control Construction-related Impacts
Standard 9. Long-Term Operation and Maintenance Plan18
Standard 10. No Illicit Discharges
Appendix A – Operation and Maintenance Plan
Infiltration Basin
Grassed Swale
ACF Rain Guardian23



Appendix B – Erosion and Sediment Control Notes and General Construction Sequence
Erosion and Sediment Control Notes25
General Construction Sequence
Appendix C – ACF Rain Guardian Supplemental Information27
Appendix D – Locus Map28
Appendix E – Pre and Post Drainage Maps29
Appendix F – HydroCAD, Stage Storage and Hydrographs
Appendix G – Mounding Calculations31



Existing Conditions

The subject site for the Sheldon West Development exists as the western portion of property located at 1139 West Street and 20 Hancock Street, Wrentham, MA. The subject site is considered a single lot in common ownership, however for the purposes of the Sheldon West Development, this report will focus on a new 20-acre lot that will be created from a portion of 1139 West Street and a portion of 20 Hancock Street. This new lot will utilize frontage from West Street.

The property is located in the R-87 Agricultural and Residential Zoning District. The parcel remains undeveloped as an open field with a large wetland system to the rear, as well as a perennial stream to the east.

The parcel contains a mixture of open grassed field, woodlands, and wetland. Throughout the field portion of the site, the topography is fairly flat, generally sloping from north to south. Where the site maintains frontage along West Street, the site climbs approximately 10' in elevation to the road from the field. The open grassed field represents approximately 7.5 acres of upland.

The existing parcel entirely drains to a single analysis point, the wetland system surrounding the south, east, and west sides of the property.

Soil conditions on site are characterized as mainly Sudbury fine sandy loam, Scarboro and Birdsall soils which have hydrologic soil groups of B and A/D, respectively. Soil testing has been performed and has confirmed these soil groups.

While the site has no active utility connections, electric and water connections are available via the West Street Right of Way.

Proposed Conditions

The Sheldon West Development proposes to construct 9 single family homes within a Senior Living Community. An exterior walking loop is being provided to add to the existing wooded trails that navigate through the wooded, natural areas.

The homes within the community have been designed as a cluster around a common open space, with the road looping around the exterior of the cluster. This allows the creation of an interior common open space including a walkway which interconnects to each unit, as well as green space available for use by residents.



The roadway has been designed at 22' wide, per the SLC design standards, and totals approximately 1,366 LF in length. The exterior of the road is proposed to be curbed, while the interior of the road is proposed as a 10' pervious parking shoulder. This will reduce the amount of runoff while also providing adequate parking for the community.

Electric, cable, and communications service as well as water service will be provided through the available connections within West Street. Sewer will be serviced through an on-site community septic system. The septic system is proposed to be located beneath the centralized green space within the loop.

The proposed stormwater management utilizes treatment best management practices (BMP's), grassed swales, as well as an infiltration basin on the east side of the entry drive. The main entry drive is proposed to be superelevated towards the east, where the stormwater will enter a curb cut, be treated, and flow to the infiltration basin. The loop will be superelevated towards the outside of the road, enter one of two grassed swales through several curb cuts. Each curb cut contains a treatment BMP prior to entering the swale. The swales will carry the stormwater to one of two outlets, where they will exit the swales and enter the infiltration basin.

Using these series of treatment BMP's, swales, and the infiltration basin, the stormwater management system is designed to capture, treat, and infiltrate stormwater as required by the Massachusetts Stormwater Standards, as well as the local Wrentham Board of Health Stormwater Regulations. See the hydrology section of this report for further detail and information.



Zoning

The proposed parcel will remain within the R-87 Agricultural and Residential Zoning District. Within this district, a Senior Living Community (SLC) is approvable via a Site Plan and Special Permit Application to the Planning Board. This section with demonstrate compliance with appropriate dimensional requirements and special permit criteria as it relates to Section 13.5 (Senior Living Community) of the Wrentham Zoning Bylaws.

Dimensional Requirements

While the parcel exists within the R-87 Agricultural and Residential Zoning District, the SLC Special Permit allows adjusted dimensional requirements from the R-87 Agricultural and Residential Zoning District.

1139 West Street, Wrentham, MAR-87 Agricultural and ResidentialProposed Use: Senior Living Community

Dimensional Requirements (R-87 Agricultural and Residential) (SLC)	Required	Proposed
Lot Area (SLC)	871,200 SF (20 AC)	888,602 SF (20.4 AC)
Continuous Lot Frontage (SLC)	100' min.	253'±
Minimum Front Yard (SLC)	30' min.	308'±
Minimum Side Yard (SLC)	30' min.	68'±
Minimum Rear Yard (SLC)	30' min.	1,486'±
Maximum Building Coverage (SLC)	35% max.	2.6%
Minimum Open Space (SLC)	30% min.	39%
Maximum Stories (SLC)	2 max.	2
Maximum Building Height (SLC)	28' max.	23'-8"
Maximum Density (SLC)	4 Units/AC	0.45 Units/AC
Minimum Distance Between (SLC)	15'	15'
Parking Requirements	Required	Proposed
Number of Parking Spaces	23 Spaces	18 Garage Spaces 26 Surface Spaces 44 Total Spaces



OPEN SPACE CALCULATION

Total Site Area = 888,602 SF Total Wetland Area = 524,432 SF Total Non-Usable Space = 80,533 SF Required Open Space = $(0.30) \times 888,602$ SF = 266,581 SF* *Per the requirements of the SLC, not more than 25% of the required common open space may be wetland.' Allowable Wetland Area = $(0.25) \times 266,581$ SF = 66,646 SF Upland Open Space = 888,602 SF - (524,432 SF + 80,533 SF) = 283,637 SF Total Open Space = 283,637 + 66,646 = 350,283 SF Open Space % = 350,283 SF / 888,602 SF = 39%

Senior Living Community – Special Permit Criteria

Per Wrentham Bylaws Section 13.5.4 - Basic Requirements

A. A Senior Living Community Shall Comply with the following density regulations: 4 Units/Acre

As shown within the Zoning Table, the Sheldon West project has an overall density of 0.45 Units/Acre.

B. Maximum building coverage shall not exceed thirty-five percent (35%) of the lot area for new construction or expansion of existing structures.

The proposed building coverage on site totals 2.6%.

C. For single family, cottage dwellings, duplexes or triplex style dwellings, the minimum setback shall be thirty feet (30') from all property lines in the Residential Districts, unless the Planning Board determines that a reduced setback is necessary to achieve the purposes of this section and will not have a detrimental impact on the neighborhood.

All units on site are proposed as single family. All proposed buildings remain at least thirty feet from all property lines.

D. No dwelling unit in a SLC shall have more than two bedrooms.

No dwelling unit is proposed to have more than two (2) bedrooms.



E. The minimum distance between buildings in any SLC shall be fifteen feet (15').

The minimum distance between buildings proposed on site is more than 15', the lowest being 15.8'. Many of the buildings maintain approximately 20' or more of separation.

F. The minimum common open space in the development shall be thirty percent (30%) of the lot area and not more than twenty-five percent (25%) of the required minimum common open space shall consist of wetlands (as defined in MGL c.131, s40). The upland open space shall be contiguous and usable by residents of the development. A permanent Conversation Restriction running to or enforceable by the Town shall be recorded for the common open space area and shall include restrictions that the land be retained in perpetuity for conservation or passive recreation.

See Dimensional Requirements section for calculation of Common Open Space.

G. All SLC dwelling units shall be subject to an age restriction described in a deed, deed rider, restrictive covenant, or other document approved by the Planning Board that shall be recorded at the Registry of Deeds and/or Land Court. The age restriction shall limit occupancy of dwelling units to at least one individual age fifty-five (55) or over ad their spouse/partner and may provide for time-limited guest visitation rights of not more than one (1) month per year. The restriction, if the Planning Board so approved and specifies in the special permit, may authorize special exceptions that allow persons of all ages to live in a dwelling unit together with a senior resident for purposes such as care of a senior in ill health or enabling seniors to fulfill legal responsibilities of guardianship or custody. The special permit including age restriction shall run with the land in perpetuity and shall be enforceable by the Town and/or any owner(s) of the SLC dwelling units. In the event of the death of a qualifying owner or occupant(s) of a dwelling unit, or foreclosure or other involuntary transfer of a unit within the SLC, a one-year exemption to the restriction shall be allowed for the transfer of the unit to another eligible occupant.

This requirement is understood and agreeable to the applicant.

- H. Minimum off-street parking requirements shall comply with Article 6.4, except as modified by the following standards:
 - a. Single Family or Cottage style dwellings: two (2) spaces per unit
 - b. Guest parking: one (1) space per two (2) units or three (3) beds, as applicable.



Per these requirements, the project is subject to the requirement of 23 parking spaces. The project proposes a total of 18 garage spaces and 26 surface spaces for a total of 44 proposed parking spaces on site.

- I. All streets within a SLC shall be private, and all sewerage, drainage facilities and utilities shall be designed and constructed in compliance with the Town of Wrentham Subdivision Rules and Regulations, except as modified by the following standards:
 - a. The minimum width of paved roadways shall be twenty-two feet (22').
 - b. There shall be a five-foot (5') sidewalk installed along one side of the roadway.

The roadway within the proposed SLC is shown at 22' wide, with an additional 10' wide pervious paver shoulder and a 6' sidewalk along the exterior loop of the roadway with direct connection to the intersection of West Street.

J. A SLC may have one (1) free standing sign at each principal access to the development from a public way, indicating the name and/or street address of the SLC. Such sign shall not exceed twelve (12) square feet in area per side or four (4') feet in height. The provisions of Article 18 shall also apply to signage within the SLC.

Signage has not been proposed at this time. Proposed signage will be designed and addressed at a later date further along in the permitting process and will meet the requirements of the SLC.

K. A SLC shall have an amenity structure designed to allow for a variety of passive and active recreational activities that support the residents of the SLC. Such uses that may be considered are community program spaces, fitness/therapeutic space, educational, recreational and accessory space; areas for neighborhood meetings and event space; and any other amenities and opportunities that are intended to create and promote an integrated neighborhood type environment.

A community amenity space has been provided within the common green area on the interior of the proposed units. This space can be utilized for a number of different activities and includes an interconnected walking loop to each unit, as well an open green space to be utilized as desired by the residents. The project is also serviced by a 6' wide meandering sidewalk with benches placed intermittently at the outside of the project allowing a longer, uninterrupted loop at the edge of the wetland and forested areas for the use of the residents.



$Per \ Wrentham \ By laws \ Section \ 13.5.8-Development \ Standards$

As part of the Planning Board's special permit review process, the Board shall evaluate the proposed Senior Living Community (SLC) for conformance to the following minimum design standards.

A. Architectural planning and design shall incorporate energy efficient design techniques, such as natural heating and cooling systems, use of sun and wind energy generation systems, and so forth.

The architectural design of the single-family homes of the development will incorporate solar panel ready roof design for future installation of solar panels by unit owners. Also, all habitable rooms will have operable double hung window to take advantage of natural cooling/ventilation at the unit owners' discretion. The building envelope will be high efficiency so that mechanical system design loads can be reduced and be more energy efficient. The mechanical heating and cooling systems will be high efficiency electric heat pump split systems in conjunction with electric high efficiency water heaters.

Also, exterior bollard style pedestrian lighting for the interior green space is a solar charged light fixture.

B. Structures located near the project property lines shall be designed and located in a manner that reflects consistency and compatibility with neighboring areas, and shall include appropriate use of building density, heights, and design to minimize intrusion on neighbors.

Though the structures are not near the property lines due to the nature of the development, the new homes being constructed could possibly be seen by adjacent abutting properties, so attention has been given to the design of the new homes as if they were close to the adjacent property lines. All the new homes are over 50' from adjacent property lines. The new homes being built are consistent with the existing neighborhood in building footprint as well as in building height. The design of the roof lines is consistent with the surrounding neighborhood incorporating a main gable roof, gable and shed dormers, asphalt shingle roofs, horizontal lap and shake siding and double hung windows. Options are provided to allow for diversity within the development and provide visual interest. Providing (3) garage door styles and (3) exterior siding color options also provide further design diversity within the development. Most importantly, the development is treating the new homes as having (2) front elevations – one front elevation facing the ring road and the adjacent abutters as well as one front elevation facing inward to the "village green". The architectural design will not detract from the current feel of the surrounding neighborhood as this development enhances, reflects and is consistent with the surrounding existing neighborhood. C. Outdoor recreation or gathering areas, particularly those that may generate significant noise and/or light and glare, shall be located to minimize intrusion on neighboring properties.

The outdoor gathering area is located away from all property lines within the center of all the units, proposed as a "village green". All noise, light, or glare generated from this area will be shielded by the proposed structures. There is also a meandering perimeter sidewalk along the ring road of the development that allows for pedestrian walking without having to cross individual home driveways making for an uninterrupted walk within benches placed intermittently along the path. This increases the safety of pedestrians walking in and around the development. Lighting is provided along the ring road that are full cut off light fixtures so that there will not be any light spillage or glare onto adjacent abutting properties. These measures minimize any intrusion on neighboring properties.

D. Structures shall be clustered to reduce site disturbance and protect open spaces, natural and environmentally sensitive areas.

The proposed new homes are clustered around an internal "village green" common area to minimize site disturbance and protect open spaces on the site. The site plan and building placement respects natural land features and environmentally sensitive areas of the site.

E. Site design shall limit large grass areas and provide adequate access to shared amenities.

The site has been designed utilizing interior sidewalks within the formal greenspace and an exterior walking loop to interconnect and create universal accessibility to all areas on site. Large grass areas are avoided and the natural environment is celebrated by clustering the project and limiting the land disturbance to allow for existing natural areas to be preserved by this development.

F. Building design shall avoid use of long unbroken facades, and shall include use of balconies, offset wall, trellises and other design elements to provide visual interest.

The structures have been designed with several gables to break up long facades on both the exterior facing and interior faces of the structure. Patios and porches have also been incorporated into the design to provide visual interest to the exterior of the building.

G. Building design, colors and materials shall generally correspond to the natural setting of the project site and promote the appearance of the Town's New England character.



The buildings have been designed with the New England aesthetic in mind utilizing colors such as brown, blue, and gray with vinyl shake siding, and vinyl lap siding. The visual design also promotes the New England aesthetic by incorporating gables to the structure.

H. Walking trails shall be accessible to all abilities and installed throughout the project.

An exterior walking loop has been proposed to meander around the exterior of the site. This walking loop, as well as the interior sidewalks interconnecting the units, has been proposed to be paved and adhere to all current ADA requirements.

I. The development shall be served by public water.

This development will be served by public water, accessed from the West Street Right of Way.

Hydrology

Standard 1. No New Untreated Discharges

The Massachusetts Stormwater Handbook requires that the project demonstrates that no new stormwater conveyances (e.g. outfalls) discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

The proposed project will not discharge stormwater directly to, or cause erosion in, wetlands or water of the Commonwealth and will treat stormwater prior to discharge or infiltration.

The infiltration basin is adjacent to a wetland and has been proposed with an outlet weir to allow treated discharge to flow from the pond to the wetland. All outlets have been designed to incorporate rip rap to minimize or eliminate erosion to wetlands.

Storm Event	2-inch	2-year	10-year	50-year	100-year
P1 Peak Discharge (fps)	0.00	0.00	1.24	2.05	2.30



Standard 2. Post-development Peak Discharge Rates Not to Exceed Pre-development Peak Discharge Rates

Post-development peak discharge rates do not exceed the pre-development peak discharge rates and total runoff volumes for all storm events. The proposed condition reduces rates by collecting and controlling the stormwater runoff within the stormwater management system.

Storm Event	2-inch	2-year	10-year	50-year	100-year
Pre-development rates (cfs) AP1 to Wetland System	0.11	2.12	7.61	18.39	25.31
Volume (cf)	2,102	12,421	33,789	75,772	103,076
Post-development rates (cfs) AP1 to Wetland System	0.06	0.89	5.59	17.91	25.21
Volume (cf)	1,021	5,561	18,340	49,874	70,762
Rate reductions (cfs)	-0.05	-1.23	-2.02	-0.48	-0.10
Volume Reductions (cf)	-1,081	-6,860	-15,449	-25,898	-32,314

Standard 3. Minimize or Eliminate Loss of Annual Recharge to Groundwater

Groundwater recharge will be accomplished using the surface infiltration practices. As shown in the table summary for Standard 2, the project decreases the total volume and runoff for all storm events. All storms have a significant decrease over the existing condition for both volume and rate of runoff. This reduction in volume is generated by collecting and infiltrating all the impervious surfaces created on site.

RECHARGE VOLUME REQUIREMENT

- Rv = F x impervious area
- Rv = Required Recharge Volume, expressed in Ft3, cubic yards, or acre-feet
- F= Target Depth Factor associated with each Hydrologic Soil Group
- Impervious Area = pavement and rooftop area on site



RECHARGE VOLUME FOR THE ENTIRE SITE

Hydrologic Group Volume to Recharge (x Total Impervious Area)

A: 0.60 inches of runoff	No Impervious located within A Soils
B: 0.35 inches of runoff	0.35 in x (1 ft/12 in) x 86,097 sf = 2,511 cf
C: 0.25 inches of runoff	No C soils were found on site
D: 0.10 inches of runoff	No D soils were found on site

TOTAL SITE RECHARGE PROVIDED = 5,634 CF RECHARGE VOLUME (BELOW OUTLET WEIR) (P1) > 2,511 CF REQUIRED

100-YEAR DRAWDOWN WITHIN 72 HOURS

Pond P1: 5,634 cf / [(8.27 in/hr)*(1 ft/12 in)*(8,187 sf)] = 1.0 hours < 72 hours, OK

10-YEAR DRAWDOWN WITHIN 24 HOURS

Pond P1: 5,634 cf / [(8.27 in/hr)*(1 ft/12 in)*(8,187 sf)] = 1.0 hours < 24 hours, OK

Volumes and surface area for ponds acquired from HydroCAD stage storage tables. These tables are attached as an appendix at the end of the HydroCAD analysis.



Standard 4. Stormwater Management System to Remove 80% of the Average Annual Load of Total Suspended Solids (TSS)

The stormwater management system is designed to remove > 80% annual total suspended solids (TSS) from the proposed roadway, driveways, and sidewalks.

TSS REMOVAL CALCULATION

TREATMENT TRAIN #1 - RG1, RG3, RG4 TO INFILTRATION BASIN P1

Area of Impervious = 19,807 SF

- ACF Rain Guardian
 - -100% * 88% = 88% Removed
 - 100% 88% = 12% *Remaining*

TSS Pretreatment Removal = $88\% \ge 44\%$

- ACF Rain Guardian
 - 100% * 88% = 88% Removed
 - 100% 88% = 12% Remaining
- Infiltration Pond (with pretreatment):
 - 12% * 80% = 9% Removed
 - 12% 9% = 3% *Remaining*

TSS Removal of the proposed drainage = 88% + 9% = 97%

TREATMENT TRAIN #2 - RG2, RG5, RG6, FH TO INFILTRATION BASIN P1

Area of Impervious = 46,669 SF

- ACF Rain Guardian
 - -100% * 79% = 79% Removed
 - 100% 79% = 21% *Remaining*

TSS Pretreatment Removal = $79\% \ge 44\%$

- ACF Rain Guardian
 - -100% * 79% = 79% Removed
 - 100% 79% = 21% *Remaining*
- Infiltration Pond (with pretreatment):
 - -21% * 80% = 16% Removed
 - 21% 16% = 5% *Remaining*

TSS Removal of the proposed drainage = 79% + 16% = 95%



TREATMENT TRAIN #3 – UNTREATED SIDEWALKS

Area of Impervious = 8,822 SF

■ No Treatment – 0%

WEIGHTED TSS REMOVAL CALCULATION

On-Site Impervious Area – 75,298 (Total analyzed impervious[86,097 SF] – off-site impervious [10,799 SF])

■ Treatment Train # 1 – 19,807 SF Percentage of Site Impervious = 19,807 SF / 75,298 SF = 26.3%

Weighted TSS Removal = 97% x 26.3% = 25.5%

Treatment Train # 2 - 46,669 SF
 Percentage of Site Impervious = 46,669 SF / 75,298 SF = 61.9%

Weighted TSS Removal = 95% x 61.9% = 58.8%

Treatment Train # 3 – 8,822 SF
 Percentage of Site Impervious = 8,822 SF / 75,298 SF = 11.8%

Weighted TSS Removal = 0% x 11.8% = 0%

Total Sitewide TSS removal = 25.5% + 58.8% = 84.3% > 80% OK

WATER QUALITY VOLUME

For new development, stormwater management systems must be designed to remove 80% of the average annual load (post-development conditions) of Total Suspended Solids (TSS). It is presumed that this standard is met when:

- a) Suitable nonstructural practices for source control and pollution prevention are implemented.
- b) Stormwater management best management practices (BMPs) are sized to capture the prescribed runoff volume; and
- c) Stormwater management BMPs are maintained as designed.

In order to achieve the rated TSS Removal Rates, each BMP must be sized adequately. This development proposes to use ACF Rain Guardian Turrets, ACF Rain Guardian Foxholes, as well as an infiltration basin. The ACF Rain Guardian Turrets and ACF Rain Guardian Foxholes are flow based devices, and the flow calculations can be found below.

ACF Rain Guardian Turret 1 (RG1): ACF Rain Guardian Turret rated for 88% removal up to 0.25 cfs ACF Rain Guardian Turret rated for 79% removal up to 0.50 cfs

Flow rate associated with ACF Rain Guardian Turret 1:

 $Q = (qu)^*(A)^*(WQV)$, where:

Q = Peak flow rate associated with first 1-inch of runoff

qu = the unit peak discharge, in csm/in (774 csm/in for Tc associated with 6 minutes)

A = impervious surface drainage area (in square miles) - 8,702 sf = 0.000312 square miles WQV = water quality volume in watershed inches

 $Q = (774 \text{ csm/in})^*(0.000312 \text{ square miles})^*(1 \text{ inch})$ Q = 0.24 CFS

Required Capacity = 0.24 CFS

ACF Turret 88% Removal Capacity = 0.25 CFS (See Appendix D for calculation)

0.25 CFS > 0.24 CFS, **OK 88% Removal**

ACF Rain Guardian Turret 2 (RG2):

ACF Rain Guardian Turret rated for 88% removal up to 0.25 cfs ACF Rain Guardian Turret rated for 79% removal up to 0.50 cfs

Flow rate associated with ACF Rain Guardian Turret 2:

 $Q = (qu)^*(A)^*(WQV)$, where:

Q = Peak flow rate associated with first 1-inch of runoff

qu = the unit peak discharge, in csm/in (774 csm/in for Tc associated with 6 minutes)

A = impervious surface drainage area (in square miles) - 11,183 sf = 0.000401 square miles

WQV = water quality volume in watershed inches

Q = (774 csm/in)*(0.000401 square miles)*(1 inch) Q = 0.31 CFS

Required Capacity = 0.31 CFS ACF Turret 79% Removal Capacity = 0.50 CFS (See Appendix D for calculation)

 $0.50\ \mathrm{CFS}$ > $0.31\ \mathrm{CFS},$ OK 79% Removal



ACF Rain Guardian Turret 3 (RG3):

ACF Rain Guardian Turret rated for 88% removal up to 0.25 cfs ACF Rain Guardian Turret rated for 79% removal up to 0.50 cfs

Flow rate associated with ACF Rain Guardian Turret 3:

 $Q = (qu)^*(A)^*(WQV)$, where:

Q = Peak flow rate associated with first 1-inch of runoff

qu = the unit peak discharge, in csm/in (774 csm/in for Tc associated with 6 minutes)

A = impervious surface drainage area (in square miles) -5,738 sf = 0.000206 square miles

WQV = water quality volume in watershed inches

$$\label{eq:Q} \begin{split} \mathbf{Q} &= (774 \ \mathrm{csm/in})^* (0.000206 \ \mathrm{square\ miles})^* (1 \ \mathrm{inch}) \\ \mathbf{Q} &= 0.16 \ \mathrm{CFS} \end{split}$$

Required Capacity = 0.16 CFS ACF Turret 88% Removal Capacity = 0.25 CFS (See Appendix D for calculation)

0.25 CFS > 0.16 CFS, **OK 88% Removal**

ACF Rain Guardian Turret 4 (RG4):

ACF Rain Guardian Turret rated for 88% removal up to 0.25 cfs ACF Rain Guardian Turret rated for 79% removal up to 0.50 cfs

Flow rate associated with ACF Rain Guardian Turret 4:

 $Q = (qu)^*(A)^*(WQV)$, where:

Q = Peak flow rate associated with first 1-inch of runoff

qu = the unit peak discharge, in csm/in (774 csm/in for Tc associated with 6 minutes)

A = impervious surface drainage area (in square miles) -5,367 sf = 0.000193 square miles

WQV = water quality volume in watershed inches

 $Q = (774 \text{ csm/in})^*(0.000193 \text{ square miles})^*(1 \text{ inch})$ Q = 0.15 CFS

Required Capacity = 0.15 CFS ACF Turret 88% Removal Capacity = 0.25 CFS (See Appendix D for calculation)

 $0.25\ \mathrm{CFS}$ > $0.15\ \mathrm{CFS},$ OK 88% Removal



ACF Rain Guardian Turret 5 (RG5):

ACF Rain Guardian Turret rated for 88% removal up to 0.25 cfs ACF Rain Guardian Turret rated for 79% removal up to 0.50 cfs

Flow rate associated with ACF Rain Guardian Turret 5: Q = (qu)*(A)*(WQV), where: Q = Peak flow rate associated with first 1-inch of runoff qu = the unit peak discharge, in csm/in (774 csm/in for Tc associated with 6 minutes) A = impervious surface drainage area (in square miles) – 11,403 sf = 0.000409 square miles WQV = water quality volume in watershed inches

 $Q = (774 \text{ csm/in})^*(0.000409 \text{ square miles})^*(1 \text{ inch})$ Q = 0.32 CFS

Required Capacity = 0.32 CFS

ACF Turret 79% Removal Capacity = 0.50 CFS (See Appendix D for calculation)

0.50 CFS > 0.32 CFS, **OK 79% Removal**

ACF Rain Guardian Turret 6 (RG6):

ACF Rain Guardian Turret rated for 88% removal up to 0.25 cfs ACF Rain Guardian Turret rated for 79% removal up to 0.50 cfs

Flow rate associated with ACF Rain Guardian Turret 6: Q = (qu)*(A)*(WQV), where: Q = Peak flow rate associated with first 1-inch of runoff qu = the unit peak discharge, in csm/in (774 csm/in for Tc associated with 6 minutes) A = impervious surface drainage area (in square miles) – 9,809 sf = 0.000352 square miles WQV = water quality volume in watershed inches

Q = (774 csm/in)*(0.000352 square miles)*(1 inch) Q = 0.27 CFS

Required Capacity = 0.27 CFS ACF Turret 79% Removal Capacity = 0.50 CFS (See Appendix D for calculation)

 $0.50\ \mathrm{CFS}$ > $0.27\ \mathrm{CFS},$ OK 79% Removal



ACF Rain Guardian Foxhole (FH):

ACF Rain Guardian Foxhole rated for 79% removal up to 0.50 cfs

Flow rate associated with ACF Rain Guardian Foxhole:

 $Q = (qu)^*(A)^*(WQV)$, where:

 \mathbf{Q} = Peak flow rate associated with first 1-inch of runoff

qu = the unit peak discharge, in csm/in (774 csm/in for Tc associated with 6 minutes)

A = impervious surface drainage area (in square miles) - 14,274 sf = 0.000512 square miles

WQV = water quality volume in watershed inches

Q = (774 csm/in)*(0.000512 square miles)*(1 inch) Q = 0.40 CFS

Required Capacity = 0.40 CFS ACF Turret 79% Removal Capacity = 0.50 CFS (See Appendix D for calculation)

 $0.50\ {\rm CFS}$ > $0.40\ {\rm CFS},$ OK 79% Removal



Standard 5. Land Uses with Higher Potential Pollutant Loads

The development is not considered a land use that generally produces higher potential pollutant loads.

Standard 6. Stormwater Discharges to Critical Areas

The proposed stormwater system does not discharge to a critical area.

Standard 7. Redevelopment Projects

The project is not considered a redevelopment project.

Standard 8. Control Construction-related Impacts

The project will install erosion and sediment controls prior to any earthwork activity. Erosion control barriers will be placed down slope from the proposed construction to prevent erosion and sedimentation into the surrounding areas. The barriers will be maintained and inspected periodically during construction; sediment buildup will be removed, and any damaged barrier will be replaced as needed.

Standard 9. Long-Term Operation and Maintenance Plan

See **Appendix A** for the operation and maintenance requirements of the stormwater management system.

Standard 10. No Illicit Discharges

An illicit discharge compliance statement will be provided by the property owner under separate cover.

Appendix A – Operation and Maintenance Plan



Infiltration Basin

System Owner: Sheldon West, LLC, or future owner. Estimated Annual Maintenance: \$1,000 (Per DEP Stormwater Structural BMP's Vol 2)

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Inspect the basin and outlet structure to ensure no structural damage has occurred and that they are functioning properly and up to design standards.

Inspection and preventive maintenance is required at least twice per year, and after each major storm event. Note how long water remains standing in the basin after a storm. If water remains standing after 48 to 72 hours after a storm, the infiltration basin may be clogged.

At least twice per year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings, accumulated organic matter, trash and debris at this time.

Remove sediment from the basin as necessary when the basin is dry. Use light equipment when removing the top layer, as to not compact the underlying soil. Use deep tilling to break and remove any clogged surfaces and revegetate immediately.

Important items to check during inspections include:

- Signs of differential settlement
- Cracking
- Erosion
- Leakage in the embankments
- Tree growth on the embankments
- Condition of rip rap
- Sediment accumulation
- Health of vegetation, turf

* Paying careful attention to pretreatment, and operation and maintenance can extend the life of the soil media

Date	Inspector	Condition	Maintenance Performed*

*Evidence of maintenance (i.e. receipts) must be provided.



Grassed Swale

System Owner: Sheldon West, LLC, or future owner. Estimated Annual Maintenance: \$1,000 (Per DEP Stormwater Structural BMP's Vol 2)

Inspect semi-annually for the first year and at least once per year thereafter. Inspect the grass for growth and the side slopes for signs of erosion and formation of rills and gullies. If original grass cover is not successfully established, contact the Design Engineer for evaluation. If grass growth is impaired by winter road salt or other deicer use, re-establish grass in the spring.

Sediment Removal: Check on a yearly basis and clean as needed. Use hand methods (i.e., a person with a shovel) when cleaning to minimize disturbance to vegetation and underlying soils. Sediment build up in the grass swale reduces its capacity to convey water events.

* Paying careful attention to pretreatment, and operation and maintenance can extend the life of the soil media

Date	Inspector	Condition	Maintenance Performed*

*Evidence of maintenance (i.e. receipts) must be provided.



ACF Rain Guardian

System Owner: Sheldon West, LLC, or future owner. Estimated Annual Maintenance: \$250-\$500 (Per Manufacturer)

Depending on the characteristics of the contributing watershed and seasonal variation, common maintenance needs include periodic removal of accumulated leaves (and other organic debris) and garbage from the top grate and sediment and fine debris from the concrete dry filter box. Contributing watersheds with high sediment concentrations may require inspections monthly and clean them out at least four times a year. More frequent visits may be needed to satisfy maintenance needs.

If sediment accumulates beyond an acceptable level in the system, it will be necessary to remove. This can be done by manual removal with a shovel or mechanical device. The filter screen can be cleaned manually through brushing or with pressurized water.

Date	Inspector	Condition	Maintenance Performed*

*Evidence of maintenance (i.e. receipts) must be provided.



Appendix B – Erosion and Sediment Control Notes and General Construction Sequence



Erosion and Sediment Control Notes

- Erosion and sediment control measures must be installed prior to the start of construction and maintained and upgraded as necessary during construction by the contractor. It is the contractor's responsibility to inspect and install additional control measures as needed during construction.
- 2) All catch basins receiving drainage from the project site must be provided with a catch basin filter.
- 3) Stabilization of all re-graded and soil stockpile areas must be maintained during all phases of construction.
- 4) Sediment removed from erosion and sediment control devices must be properly removed and disposed. All damaged controls must be removed and replaced.
- 5) The contractor is responsible for implementing the erosion and sediment control plan. This includes the installation and maintenance of control measures, informing all parties engaged on the construction site of the requirements and objectives of the plan, and notifying the proper city agency of any transfer of this responsibility.
- 6) The contractor shall be responsible for controlling wind erosion and dust throughout the life of his contract. Dust control may include, but is not limited to, sprinkling of water on exposed soils and street sweeping adjacent roadways.
- 7) If final grading is to be delayed for more than 21 days after land disturbance activities cease, temporary vegetation or mulch shall be used to stabilize soils within 14 days of the last disturbance.
- 8) If a disturbed area will be exposed for greater than one year, permanent grasses or other approved cover must be installed.
- 9) The contractor must keep on-site at all times additional silt fence and hay bales for the installation at the direction of the engineer or the city to mitigate any emergency condition.
- 10) The construction fencing and erosion and sediment controls as shown may not be practical during all stages of construction. Earthwork activity on-site must be done in a manner such that runoff is directed to a sediment control device or infiltrated to the ground.
- 11) Demolition and construction debris must be properly contained and disposed of.
- 12) Disposal of all demolished materials is the responsibility of the contractor and must be hauled off-site in accordance with all federal, state, and local requirements.



General Construction Sequence

- 1) Install erosion and sediment controls prior to starting any earthworks activity.
- 2) Begin clearing, grubbing and demolition.
- 3) Begin utility installations.
- 4) Construct building foundation.
- 5) Install site furnishings.
- 6) Install landscaping.
- 7) Erosion and sediment controls shall be maintained until permanent cover is established.

Appendix C – ACF Rain Guardian Supplemental Information





RAIN GUARDIAN TURRET AND FOXHOLE ENGINEERING PROPERTIES

RAIN GUARDIAN TURRET:

Turret Flow Rate Capacity:

Outflow is possible through three locations. Please note the vertical filter within the chamber was assumed to be 100% clogged because its primary function is to allow the chamber to dry out between rain events.

1) Filter overflow – water can pass between the top of the filter and the bottom of the metal grate; calculated using the continuity equation (i.e. $Q=V^*A$)

2) Grate overflow – water can pass through the top metal grate beyond the vertical filter wall; calculated using an orifice equation (i.e. $Q=0.0108*A*\sqrt{d}$)

3) High volume overflow – water can overtop the front debris wall onto the splash pad; calculated using a standard broad crested weir equation (i.e. $Q=C*L*H^{(3/2)}$)

Filter overflow – 0.45 CFS

Grate overflow – 2.59 CFS

Emergency overflow - 0.41 CFS

TOTAL: 3.45 CFS

Turret Internal Storage Vol: (i.e. storage capacity below the top of the filter wall): 4.02 ft³

RAIN GUARDIAN FOXHOLE:

Below are the flow and storage data for the Rain Guardian Foxhole with an inlet, middle, and outlet (i.e. 6' top lid). (the addition of mid section (for longer units) would improve the sediment storage capacity).

Foxhole Flow Rate Capacity:

Outflow is possible through three locations. Please note the vertical filter within the chamber was assumed to be 100% clogged because its primary function is to allow the chamber to dry out between rain events.

1) Filter overflow – water can pass between the top of the filter and the bottom of the metal grate; calculated using the continuity equation (i.e. $Q=V^*A$)

2) Grate overflow – water can pass through the top metal grate beyond the vertical filter wall; calculated using an orifice equation (i.e. $Q=0.0108*A*\sqrt{d}$)

3) High volume overflow – water can overtop the front debris wall onto the splash pad; calculated using a standard broad crested weir equation (i.e. $Q=C*L*H^{(3/2)}$)

Filter overflow – 0.30 CFS Grate overflow – 2.69 CFS Emergency overflow - 0.52 CFS TOTAL: **3.51 CFS**

<u>Foxhole Internal Storage Volume</u> (i.e. storage capacity below the top of the filter wall):

Inlet + Outlet: 2.0 ft^3

Middle: 2.65 ft^3

TOTAL: 4.65 ft³

From:	Lee Jones
To:	Patrick Bogle
Subject:	FW: ACF Environmental - Rain Guardians
Date:	Friday, December 4, 2020 9:30:52 AM
Attachments:	2017.11.21 Flow Data.pdf

Patrick,

Good morning

Great talking with you yesterday.

Per our discussion, I have reviewed the study.

The study was performed by flow rates which are shown and not by storm events. I am also attaching the Flow Data pdf.

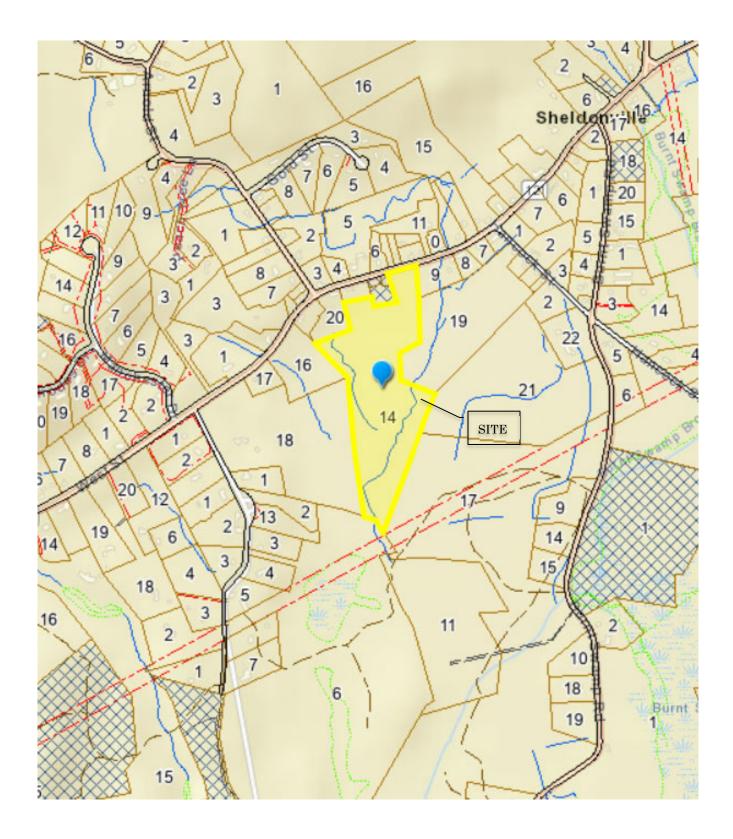
- Rain Guardian Gross Solids and Sediment Removal Report
 - Bunker sediment capture 75.6% at 0.5 CFS and 91.7% at 0.25 CFS
 - Bunker gross solids capture 61.4% at 0.5 CFS and 78.8% at 0.25 CFS
 - Turret sediment capture 79.1% at 0.5 CFS and 88.4% at 0.25 CFS
 - Turret gross solids capture 72.4% at 0.5 CFS and 86.7% at 0.25 CFS
 - NOTE: Grass and rock lined inlets were also tested and achieved similar removal efficiencies. While the grass lined inlet and rock lined inlets removed similar amounts of sediment under the flow rates tested, the ease of maintenance, long-term effectiveness, storage capacity, and stability of the Rain Guardians set them apart from the grass and rock. The 'Maintenance Considerations' section (5.4) on pages 65 68 of the report highlights some advantages of the Rain Guardian products.

Please let me know if I can be of help Best regards, Lee

Leland (Lee) Jones, QSM BMP Specialist – New England ACF Environmental 508-745-7052 cell <u>ljones@acfenv.com</u> <u>www.acfenvironmental.com</u> "Start each day with a positive thought and a grateful heart" Roy T. Bennett

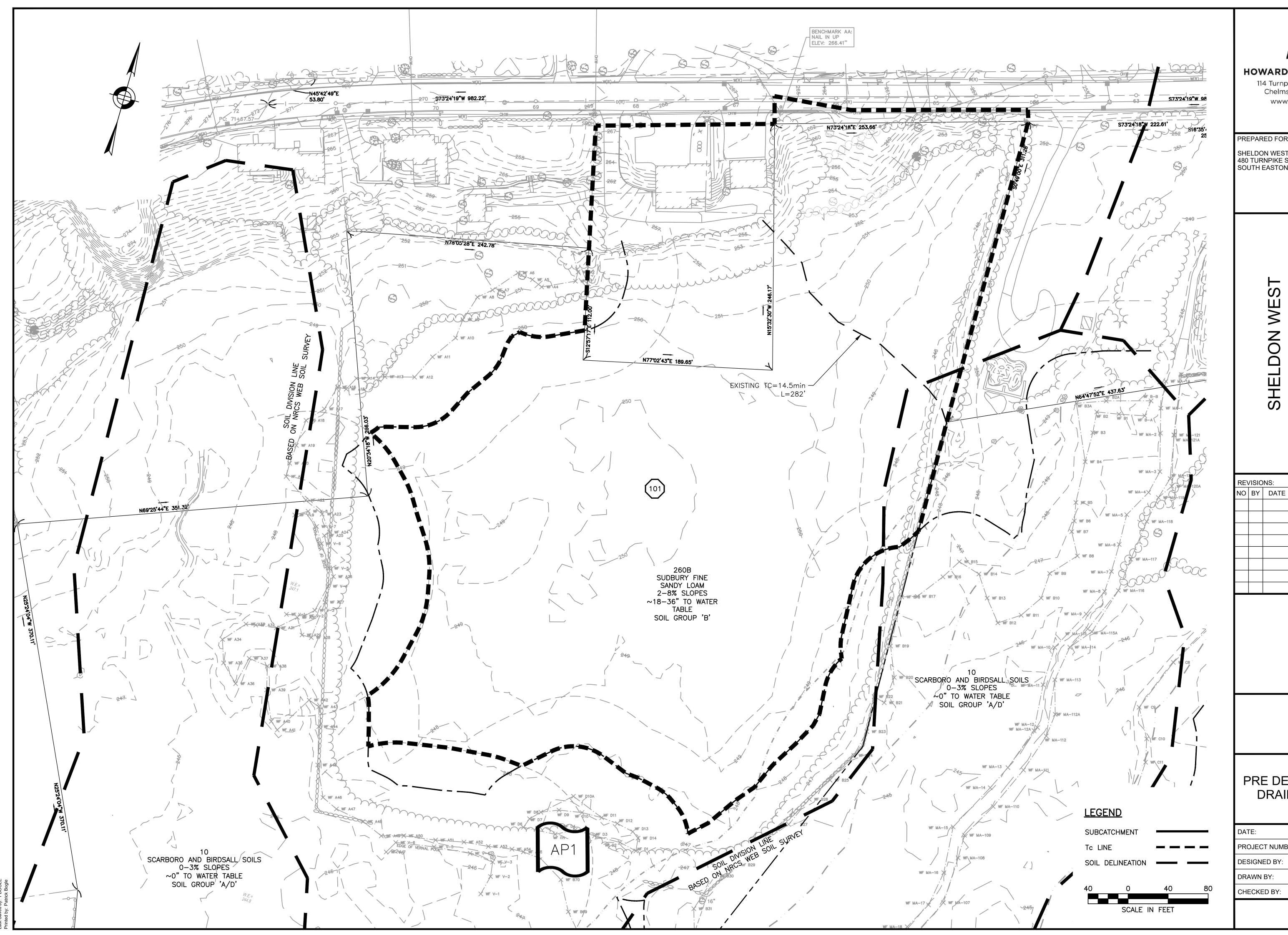


Appendix D – Locus Map



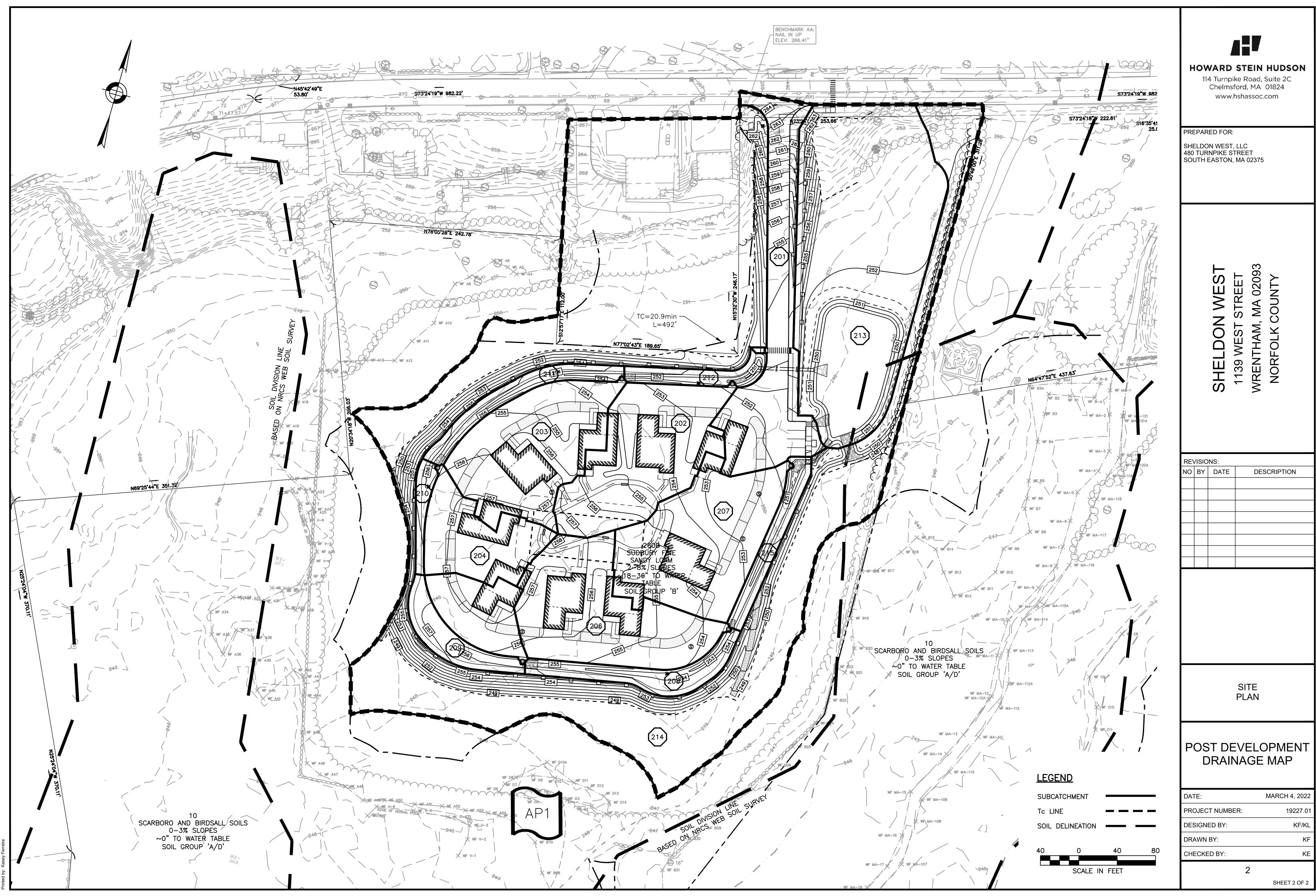
HOWARD STEIN HUDSON

Appendix E – Pre and Post Drainage Maps



2 L:\1927\West St - CURRENT\19227 - Drainage.dw ved by: PBOGLE

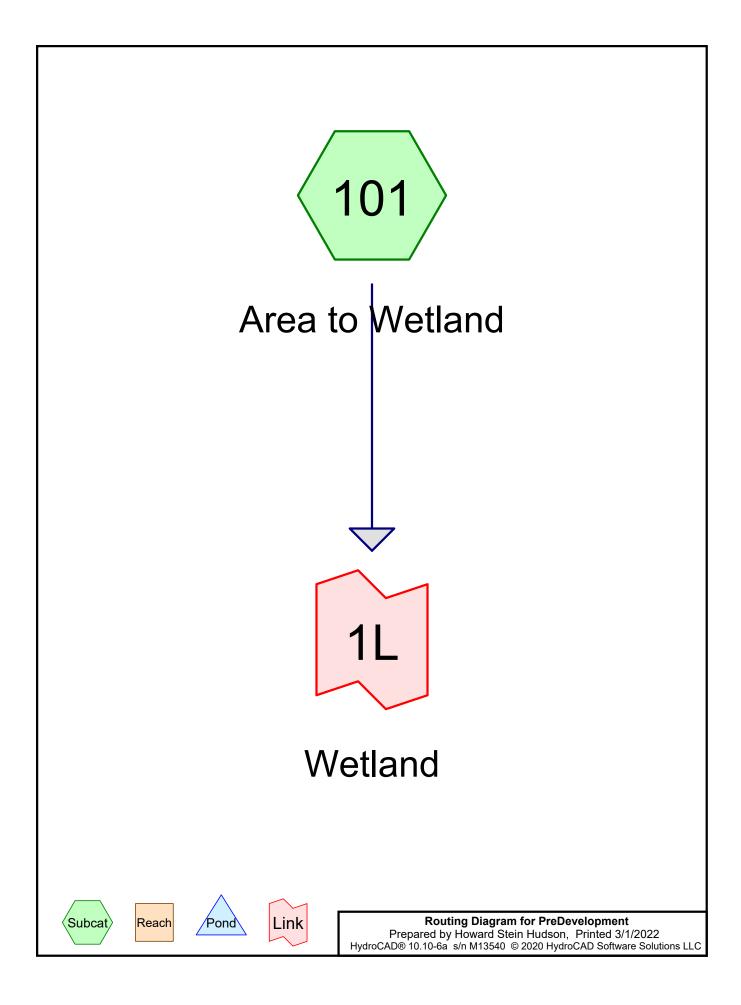
HOWARD STEIN HUDSON 114 Turnpike Road, Suite 2C Chelmsford, MA 01824 www.hshassoc.com						
PREPARED FOR: SHELDON WEST, LLC 480 TURNPIKE STREET SOUTH EASTON, MA 02375						
SHELDON WEST 1139 WEST STREET WRENTHAM, MA 02093 NORFOLK COUNTY						
REVISIONS: NO BY DATE DESCRIPTION						
SITE PLAN						
PRE DEVELOPMENT DRAINAGE MAP						
DATE: MARCH 4, 2022						
PROJECT NUMBER: 19227.01 DESIGNED BY: KF/KL						
DRAWN BY: KF						
CHECKED BY: KE						



2 L:\19227\West St - CURRENT\19227 - Drainage.d ved by: MBAKER



Appendix F – HydroCAD, Stage Storage and Hydrographs



Project Notes

Rainfall events imported from "19227 - Post Dev_West St.hcp"

PreDevelopment Prepared by Howard Stein Hudson HydroCAD® 10.10-6a s/n M13540 © 2020 HydroCAD Software Solutions LLC

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Inch	Type III 24-hr		Default	24.00	1	2.00	2
2	2-yr	Type III 24-hr		Default	24.00	1	3.27	2
3	10-yr	Type III 24-hr		Default	24.00	1	4.92	2
4	50-yr	Type III 24-hr		Default	24.00	1	7.42	2
5	100-yr	Type III 24-hr		Default	24.00	1	8.86	2

Rainfall Events Listing

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
4,850	39	>75% Grass cover, Good, HSG A (101)
259,865	61	>75% Grass cover, Good, HSG B (101)
8,418	98	Paved parking, HSG B (101)
3,519	98	Roofs, HSG B (101)
2,367	30	Woods, Good, HSG A (101)
13,927	55	Woods, Good, HSG B (101)
292,946	62	TOTAL AREA

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
7,217	HSG A	101
285,729	HSG B	101
0	HSG C	
0	HSG D	
0	Other	
292,946		TOTAL AREA

PreDevelopment

Prepared by Howard Stein Hudson	
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HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Su Nu
4,850	259,865	0	0	0	264,715	>75% Grass cover, Good	
0	8,418	0	0	0	8,418	Paved parking	
0	3,519	0	0	0	3,519	Roofs	
2,367	13,927	0	0	0	16,294	Woods, Good	
7,217	285,729	0	0	0	292,946	TOTAL AREA	

Ground Covers (all nodes)

PreDevelopment	Type III 24-hr	2-Inch Rainfall=2.00"
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment101: Area to WetlandRunoff Area=292,946 sf 4.07% ImperviousRunoff Depth>0.09"Flow Length=282'Tc=14.5 minCN=62Runoff=0.11 cfs 2,102 cf

Link 1L: Wetland

Inflow=0.11 cfs 2,102 cf Primary=0.11 cfs 2,102 cf

Total Runoff Area = 292,946 sf Runoff Volume = 2,102 cf Average Runoff Depth = 0.09" 95.93% Pervious = 281,009 sf 4.07% Impervious = 11,937 sf

Summary for Subcatchment 101: Area to Wetland

Runoff = 0.11 cfs @ 12.60 hrs, Volume= 2,102 cf, Depth> 0.09" Routed to Link 1L : Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Inch Rainfall=2.00"

A	rea (sf)	CN D	escription					
	4,850	39 >	39 >75% Grass cover, Good, HSG A					
	13,927	55 V	Voods, Go	od, HSG B				
	2,367	30 V	Voods, Go	od, HSG A				
	8,418	98 P	aved park	ing, HSG B	}			
	3,519	98 F	Roofs, HSG	βB				
2	259,865	61 >	75% Gras	s cover, Go	bod, HSG B			
2	92,946	62 V	Veighted A	verage				
2	81,009	9	5.93% Per	vious Area				
	11,937	4	.07% Impe	ervious Area	а			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.4	35	0.1000	0.07		Sheet Flow,			
					Woods: Dense underbrush n= 0.800 P2= 3.27"			
1.7	15	0.0330	0.14		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.27"			
3.1	194	0.0220	1.04		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
1.3	38	0.0100	0.50		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
14.5	282	Total						

Summary for Link 1L: Wetland

Inflow Are	a =	292,946 sf,	4.07% Impervious,	Inflow Depth > 0.09"	for 2-Inch event
Inflow	=	0.11 cfs @ 12	2.60 hrs, Volume=	2,102 cf	
Primary	=	0.11 cfs @ 12	2.60 hrs, Volume=	2,102 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

PreDevelopment	Type III 24-hr 2-yr Rainfall=3.27"
Prepared by Howard Stein Hudson	Printed 3/1/2022
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment101: Area to WetlandRunoff Area=292,946 sf4.07% ImperviousRunoff Depth>0.51"Flow Length=282'Tc=14.5 minCN=62Runoff=2.12 cfs12,421 cf

Link 1L: Wetland

Inflow=2.12 cfs 12,421 cf Primary=2.12 cfs 12,421 cf

Total Runoff Area = 292,946 sf Runoff Volume = 12,421 cfAverage Runoff Depth = 0.51"95.93% Pervious = 281,009 sf4.07% Impervious = 11,937 sf

Summary for Subcatchment 101: Area to Wetland

Runoff = 2.12 cfs @ 12.27 hrs, Volume= 12,421 cf, Depth> 0.51" Routed to Link 1L : Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=3.27"

A	rea (sf)	CN E	Description					
	4,850	39 >	39 >75% Grass cover, Good, HSG A					
	13,927	55 V	Voods, Go	od, HSG B				
	2,367	30 V	Voods, Go	od, HSG A				
	8,418	98 F	aved park	ing, HSG B	6			
	3,519	98 F	Roofs, HSC	βB				
2	59,865	61 >	75% Gras	s cover, Go	ood, HSG B			
2	92,946	62 V	Veighted A	verage				
2	81,009	9	5.93% Per	vious Area				
	11,937	4	.07% Impe	ervious Area	a			
Tc	Length	Slope	Velocity		Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.4	35	0.1000	0.07		Sheet Flow,			
					Woods: Dense underbrush n= 0.800 P2= 3.27"			
1.7	15	0.0330	0.14		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.27"			
3.1	194	0.0220	1.04		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
1.3	38	0.0100	0.50		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
14.5	282	Total						

Summary for Link 1L: Wetland

Inflow Are	a =	292,946 sf,	4.07% Impervious,	Inflow Depth > 0.51"	for 2-yr event
Inflow	=	2.12 cfs @ 1	12.27 hrs, Volume=	12,421 cf	
Primary	=	2.12 cfs @ 1	12.27 hrs, Volume=	12,421 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

PreDevelopment	Type III 24-hr	10-yr Rainfall=4.92"
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment101: Area to WetlandRunoff Area=292,946 sf4.07% ImperviousRunoff Depth>1.38"Flow Length=282'Tc=14.5 minCN=62Runoff=7.61 cfs33,789 cf

Link 1L: Wetland

Inflow=7.61 cfs 33,789 cf Primary=7.61 cfs 33,789 cf

Total Runoff Area = 292,946 sf Runoff Volume = 33,789 cfAverage Runoff Depth = 1.38"95.93% Pervious = 281,009 sf4.07% Impervious = 11,937 sf

Summary for Subcatchment 101: Area to Wetland

Runoff = 7.61 cfs @ 12.22 hrs, Volume= 33,789 cf, Depth> 1.38" Routed to Link 1L : Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-yr Rainfall=4.92"

A	rea (sf)	CN E	Description						
	4,850	39 >	75% Gras	s cover, Go	bod, HSG A				
	13,927	55 V	Voods, Go	od, HSG B					
	2,367	30 V	Voods, Go	od, HSG A					
	8,418	98 F	98 Paved parking, HSG B						
	3,519	98 F	98 Roofs, HSG B						
2	59,865	61 >	75% Gras	s cover, Go	ood, HSG B				
2	92,946	62 V	Veighted A	verage					
2	81,009	9	5.93% Per	vious Area					
	11,937	4	.07% Impe	ervious Area	a				
Tc	Length	Slope	Velocity		Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
8.4	35	0.1000	0.07		Sheet Flow,				
					Woods: Dense underbrush n= 0.800 P2= 3.27"				
1.7	15	0.0330	0.14		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.27"				
3.1	194	0.0220	1.04		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
1.3	38	0.0100	0.50		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
14.5	282	Total							

Summary for Link 1L: Wetland

Inflow Are	a =	292,946 sf, 4.	07% Impervious,	Inflow Depth > 1.38"	for 10-yr event
Inflow	=	7.61 cfs @ 12.2	2 hrs, Volume=	33,789 cf	
Primary	=	7.61 cfs @ 12.2	2 hrs, Volume=	33,789 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

PreDevelopment	Type III 24-hr 50-yr Rainfall=7.42"
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment101: Area to WetlandRunoff Area=292,946 sf4.07% ImperviousRunoff Depth>3.10"Flow Length=282'Tc=14.5 minCN=62Runoff=18.39 cfs75,772 cf

Link 1L: Wetland

Inflow=18.39 cfs 75,772 cf Primary=18.39 cfs 75,772 cf

Total Runoff Area = 292,946 sf Runoff Volume = 75,772 cf Average Runoff Depth = 3.10" 95.93% Pervious = 281,009 sf 4.07% Impervious = 11,937 sf

Summary for Subcatchment 101: Area to Wetland

Runoff = 18.39 cfs @ 12.21 hrs, Volume= 75,772 cf, Depth> 3.10" Routed to Link 1L : Wetland

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=7.42"

A	rea (sf)	CN E	escription					
	4,850	39 >	39 >75% Grass cover, Good, HSG A					
	13,927	55 V						
	2,367	30 V	Voods, Go	od, HSG A				
	8,418	98 F	aved park	ing, HSG B	8			
	3,519	98 F	Roofs, HSC	Β́Β				
2	259,865	61 >	75% Gras	s cover, Go	bod, HSG B			
2	92,946	62 V	Veighted A	verage				
2	81,009	9	5.93% Per	rvious Area				
	11,937	4	.07% Impe	ervious Area	а			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.4	35	0.1000	0.07		Sheet Flow,			
					Woods: Dense underbrush n= 0.800 P2= 3.27"			
1.7	15	0.0330	0.14		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.27"			
3.1	194	0.0220	1.04		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
1.3	38	0.0100	0.50		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
14.5	282	Total						

Summary for Link 1L: Wetland

Inflow Are	a =	292,946 sf,	4.07% Impervious,	Inflow Depth > 3.10	" for 50-yr event
Inflow	=	18.39 cfs @ 1	12.21 hrs, Volume=	75,772 cf	
Primary	=	18.39 cfs @ 1	12.21 hrs, Volume=	75,772 cf, At	ten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

PreDevelopment	Type III 24-hr	100-yr Rainfall=8.86"
Prepared by Howard Stein Hudson		Printed 3/1/2022
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment101: Area to WetlandRunoff Area=292,946 sf4.07% ImperviousRunoff Depth>4.22"Flow Length=282'Tc=14.5 minCN=62Runoff=25.31 cfs103,076 cf

Link 1L: Wetland

Inflow=25.31 cfs 103,076 cf Primary=25.31 cfs 103,076 cf

Total Runoff Area = 292,946 sf Runoff Volume = 103,076 cf Average Runoff Depth = 4.22" 95.93% Pervious = 281,009 sf 4.07% Impervious = 11,937 sf

Summary for Subcatchment 101: Area to Wetland

Runoff = 25.31 cfs @ 12.21 hrs, Volume= 103,076 cf, Depth> 4.22" Routed to Link 1L : Wetland

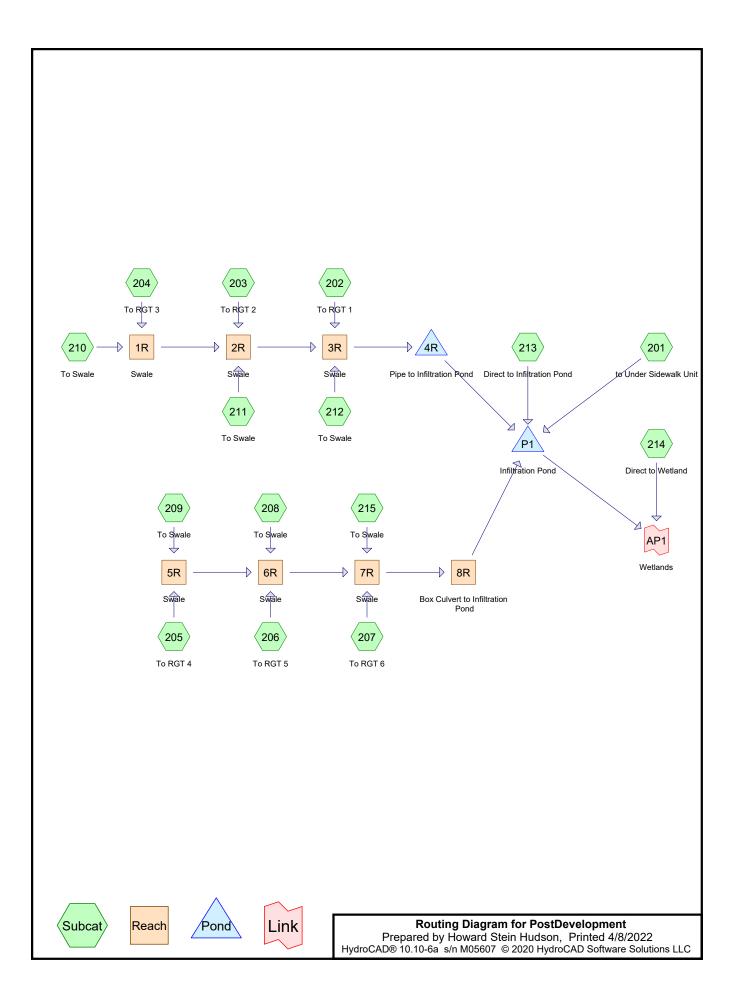
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-yr Rainfall=8.86"

A	rea (sf)	CN E	Description					
	4,850	39 >	75% Gras	s cover, Go	ood, HSG A			
	13,927	55 V	Voods, Go	od, HSG B				
	2,367	30 V	30 Woods, Good, HSG A					
	8,418	98 F	aved park	ing, HSG B				
	3,519	98 F	Roofs, HSC	Β́Β				
2	259,865	61 >	75% Gras	s cover, Go	bod, HSG B			
2	92,946	62 V	Veighted A	verage				
2	81,009	9	5.93% Per	vious Area				
	11,937	4	.07% Impe	ervious Area	a			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.4	35	0.1000	0.07		Sheet Flow,			
					Woods: Dense underbrush n= 0.800 P2= 3.27"			
1.7	15	0.0330	0.14		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.27"			
3.1	194	0.0220	1.04		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
1.3	38	0.0100	0.50		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
14.5	282	Total						

Summary for Link 1L: Wetland

Inflow Are	a =	292,946 sf,	4.07% Impervious,	Inflow Depth > 4.22"	for 100-yr event
Inflow	=	25.31 cfs @ 1	12.21 hrs, Volume=	103,076 cf	
Primary	=	25.31 cfs @ 1	12.21 hrs, Volume=	103,076 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



Project Notes

Rainfall events imported from "PreDevelopment.hcp"

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-Inch	Type III 24-hr		Default	24.00	1	2.00	2
2	2-yr	Type III 24-hr		Default	24.00	1	3.27	2
3	10-yr	Type III 24-hr		Default	24.00	1	4.92	2
4	50-yr	Type III 24-hr		Default	24.00	1	7.42	2
5	100-yr	Type III 24-hr		Default	24.00	1	8.86	2

Rainfall Events Listing

Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
6,167	39	>75% Grass cover, Good, HSG A (213, 214)
189,826	61	>75% Grass cover, Good, HSG B (201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215)
62,930	98	Paved parking, HSG B (201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215)
23,167	98	Roofs, HSG B (201, 202, 203, 204, 205, 206, 207, 214)
1,050	30	Woods, Good, HSG A (214)
9,806	55	Woods, Good, HSG B (214)
292,946	71	TOTAL AREA

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
7,217	HSG A	213, 214
285,729	HSG B	201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215
0	HSG C	
0	HSG D	
0	Other	
292,946		TOTAL AREA

PostDe	ve	lopm	ent	
_				-

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Printed 4/8/2022 Page 6

ł	HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Sub Nun
	6,167	189,826	0	0	0	195,993	>75% Grass	
							cover, Good	
	0	62,930	0	0	0	62,930	Paved parking	
	0	23,167	0	0	0	23,167	Roofs	
	1,050	9,806	0	0	0	10,856	Woods, Good	
	7,217	285,729	0	0	0	292,946	TOTAL AREA	

Ground Covers (all nodes)

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Pipe Listing (all nodes)									
Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Width	Diam/Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)

6.0 0.0050

55.0 0.0051

0.040

0.012

60.0

0.0

1

2

8R

4R

250.00

250.00

249.97

249.72

0.0

0.0

18.0

15.0

Pipe Listing (all nodes)

PostDevelopment Prepared by Howard Stein Hudson <u>HydroCAD® 10.10-6a s/n M05607 © 2020 Hy</u>	Type III 24-hr 2-Inch Rainfall=2.00" Printed 4/8/2022 droCAD Software Solutions LLC Page 8
Runoff by SCS TF	24.00 hrs, dt=0.01 hrs, 2401 points x 3 R-20 method, UH=SCS, Weighted-CN d method . Pond routing by Dyn-Stor-Ind method
Subcatchment 201: to Under Sidewalk Ur	nit Runoff Area=17,748 sf 80.43% Impervious Runoff Depth>1.16" Tc=6.0 min CN=91 Runoff=0.56 cfs 1,719 cf
Subcatchment 202: To RGT 1	Runoff Area=17,947 sf 48.49% Impervious Runoff Depth>0.52" Tc=6.0 min CN=79 Runoff=0.23 cfs 780 cf
Subcatchment 203: To RGT 2	Runoff Area=17,507 sf 63.88% Impervious Runoff Depth>0.79" Tc=6.0 min CN=85 Runoff=0.37 cfs 1,158 cf
Subcatchment 204: To RGT 3	Runoff Area=9,147 sf 62.73% Impervious Runoff Depth>0.74" Tc=6.0 min CN=84 Runoff=0.18 cfs 566 cf
Subcatchment 205: To RGT 4	Runoff Area=6,183 sf 86.80% Impervious Runoff Depth>1.31" Tc=6.0 min CN=93 Runoff=0.22 cfs 677 cf
Subcatchment 206: To RGT 5	Runoff Area=18,720 sf 60.91% Impervious Runoff Depth>0.74" Tc=6.0 min CN=84 Runoff=0.36 cfs 1,159 cf
Subcatchment 207: To RGT 6	Runoff Area=19,445 sf 50.44% Impervious Runoff Depth>0.56" Tc=6.0 min CN=80 Runoff=0.27 cfs 910 cf
Subcatchment 208: To Swale	Runoff Area=4,919 sf 31.45% Impervious Runoff Depth>0.32" Tc=6.0 min CN=73 Runoff=0.03 cfs 131 cf
Subcatchment 209: To Swale	Runoff Area=3,858 sf 28.90% Impervious Runoff Depth>0.29" Tc=6.0 min CN=72 Runoff=0.02 cfs 94 cf
Subcatchment 210: To Swale	Runoff Area=1,505 sf 48.11% Impervious Runoff Depth>0.52" Tc=6.0 min CN=79 Runoff=0.02 cfs 65 cf
Subcatchment 211: To Swale	Runoff Area=4,032 sf 35.99% Impervious Runoff Depth>0.35" Tc=6.0 min CN=74 Runoff=0.03 cfs 117 cf
Subcatchment 212: To Swale	Runoff Area=2,719 sf 32.88% Impervious Runoff Depth>0.32" Tc=6.0 min CN=73 Runoff=0.02 cfs 72 cf
Subcatchment 213: Direct to Infiltration	Runoff Area=43,710 sf 4.37% Impervious Runoff Depth>0.07" Tc=6.0 min CN=61 Runoff=0.01 cfs 265 cf
Subcatchment 214: Direct to Wetland	Runoff Area=122,030 sf 8.85% Impervious Runoff Depth>0.10" Flow Length=492' Tc=20.9 min CN=63 Runoff=0.06 cfs 1,021 cf
Subcatchment 215: To Swale	Runoff Area=3,476 sf 33.92% Impervious Runoff Depth>0.35" Tc=6.0 min CN=74 Runoff=0.02 cfs 101 cf
Reach 1R: Swale n=0.030	Avg. Flow Depth=0.07' Max Vel=1.62 fps Inflow=0.20 cfs 632 cf L=24.0' S=0.0417 '/' Capacity=69.88 cfs Outflow=0.20 cfs 632 cf

PostDevelopment Prepared by Howard Stein Hudson HydroCAD® 10.10-6a s/n M05607 © 2020	Type III 24-hr 2-Inch Rainfall=2.00"Printed 4/8/20220 HydroCAD Software Solutions LLCPage 9
Reach 2R: Swale n=0.030	Avg. Flow Depth=0.18' Max Vel=1.56 fps Inflow=0.59 cfs 1,907 cf L=230.0' S=0.0130 '/' Capacity=39.10 cfs Outflow=0.56 cfs 1,902 cf
Reach 3R: Swale n=0.030	Avg. Flow Depth=0.22' Max Vel=1.74 fps Inflow=0.80 cfs 2,754 cf L=149.0' S=0.0134 '/' Capacity=39.66 cfs Outflow=0.78 cfs 2,749 cf
Reach 5R: Swale n=0.03	Avg. Flow Depth=0.09' Max Vel=1.42 fps Inflow=0.24 cfs 770 cf 0 L=139.0' S=0.0234 '/' Capacity=52.35 cfs Outflow=0.23 cfs 769 cf
Reach 6R: Swale n=0.030	Avg. Flow Depth=0.20' Max Vel=1.48 fps Inflow=0.62 cfs 2,059 cf L=167.0' S=0.0105 '/' Capacity=35.05 cfs Outflow=0.60 cfs 2,055 cf
Reach 7R: Swale n=0.030	Avg. Flow Depth=0.28' Max Vel=1.31 fps Inflow=0.89 cfs 3,066 cf L=259.0' S=0.0058 '/' Capacity=26.05 cfs Outflow=0.82 cfs 3,055 cf
	Avg. Flow Depth=0.19' Max Vel=0.84 fps Inflow=0.82 cfs 3,055 cf 40 L=6.0' S=0.0050 '/' Capacity=13.65 cfs Outflow=0.82 cfs 3,054 cf
Pond 4R: Pipe to Infiltration Pond 15.0" Round	Peak Elev=250.33' Storage=16 cf Inflow=0.78 cfs 2,749 cf Culvert x 2.00 n=0.012 L=55.0' S=0.0051 '/' Outflow=0.78 cfs 2,748 cf
Pond P1: Infiltration Pond Discard	Peak Elev=249.28' Storage=244 cf Inflow=2.06 cfs 7,787 cf ed=1.60 cfs 7,785 cf Primary=0.00 cfs 0 cf Outflow=1.60 cfs 7,785 cf
Link AP1: Wetlands	Inflow=0.06 cfs 1,021 cf Primary=0.06 cfs 1,021 cf

Total Runoff Area = 292,946 sf Runoff Volume = 8,837 cf Average Runoff Depth = 0.36" 70.61% Pervious = 206,849 sf 29.39% Impervious = 86,097 sf

Summary for Subcatchment 201: to Under Sidewalk Unit

Page 10

Runoff 0.56 cfs @ 12.09 hrs, Volume= 1,719 cf, Depth> 1.16" = Routed to Pond P1 : Infiltration Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Inch Rainfall=2.00"

Are	ea (sf)	CN	Description			
1	3,365	98	Paved park	ing, HSG B	5	
	3,474	61	>75% Gras	s cover, Go	ood, HSG B	
	909	98	Roofs, HSG	БB		
1	7,748	91	Weighted Average			
	3,474		19.57% Per	vious Area		
1	4,274		80.43% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description	
6.0					Direct Entry,	

Summary for Subcatchment 202: To RGT 1

Runoff = 0.23 cfs @ 12.10 hrs, Volume= 780 cf, Depth> 0.52" Routed to Reach 3R : Swale

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Inch Rainfall=2.00"

A	rea (sf)	CN I	Description			
	4,676	98	Paved park	ing, HSG B	3	
	9,245	61 :	>75% Gras	s cover, Go	ood, HSG B	
	4,026	98	Roofs, HSG	БВ		
	17,947	79	Weighted Average			
	9,245	!	51.51% Pervious Area			
	8,702	4	48.49% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)		
6.0					Direct Entry,	

Summary for Subcatchment 203: To RGT 2

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 1,158 cf, Depth> 0.79" Routed to Reach 2R : Swale

A	rea (sf)	CN I	Description				
	7,022	98	Paved park	ing, HSG B	3		
	6,324	61 :	>75% Gras	s cover, Go	bod, HSG B		
	4,161	98	Roofs, HSG	БВ			
	17,507	85 Weighted Average					
	6,324		36.12% Pervious Area				
	11,183	(63.88% Imp	pervious Ar	ea		
-		01		0			
Tc	Length	Slope		Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 204: To RGT 3

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 566 cf, Depth> 0.74" Routed to Reach 1R : Swale

/	Area (sf)	CN	Description			
	3,638		Paved park			
	3,409	61	>75% Gras	s cover, Go	ood, HSG B	
	2,100	98	Roofs, HSC	Э В		
	9,147	7 84 Weighted Average				
	3,409	37.27% Pervious Area				
	5,738		62.73% Imp	pervious Ar	ea	
Тс	5	Slope	,	Capacity	Description	
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)		
6.0					Direct Entry,	
(min)	(feet)	Slope (ft/ft)	,	Capacity (cfs)	•	

Summary for Subcatchment 205: To RGT 4

Runoff = 0.22 cfs @ 12.09 hrs, Volume= 677 cf, Depth> 1.31" Routed to Reach 5R : Swale

A	rea (sf)	CN	Description				
	4,135		Paved park				
	816	61	>75% Gras	s cover, Go	ood, HSG B		
	1,232	98	Roofs, HSC	βB			
	6,183	93 Weighted Average					
	816		13.20% Pervious Area				
	5,367		86.80% Imp	pervious Ar	rea		
Tc	Length	Slope		Capacity			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 206: To RGT 5

Runoff = 0.36 cfs @ 12.09 hrs, Volume= 1,159 cf, Depth> 0.74" Routed to Reach 6R : Swale

A	rea (sf)	CN I	Description				
	7,564	98 I	Paved park	ing, HSG B	В		
	7,317	61 >	>75% Gras	s cover, Go	ood, HSG B		
	3,839	98 I	Roofs, HSG	БВ			
	18,720	84 V	84 Weighted Average				
	7,317	3	39.09% Pervious Area				
	11,403	6	60.91% Imp	pervious Ar	rea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	•		
6.0					Direct Entry,		

Summary for Subcatchment 207: To RGT 6

Runoff = 0.27 cfs @ 12.10 hrs, Volume= 910 cf, Depth> 0.56" Routed to Reach 7R : Swale

A	rea (sf)	CN I	Description				
	6,428	98	Paved park	ing, HSG B	3		
	9,636	61 3	>75% Gras	s cover, Go	ood, HSG B		
	3,381	98	Roofs, HSG	6 B			
	19,445	80	80 Weighted Average				
	9,636	4	49.56% Pervious Area				
	9,809	:	50.44% Imp	pervious Ar	rea		
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Subcatchment 208: To Swale

Runoff = 0.03 cfs @ 12.11 hrs, Volume= 131 cf, Depth> 0.32" Routed to Reach 6R : Swale

A	rea (sf)	CN I	Description					
	1,547	98	Paved park	ing, HSG B	В			
	3,372	61 :	75% Grass cover, Good, HSG B					
	4,919	73	Neighted A	verage				
	3,372	(68.55% Pervious Area					
	1,547	:	31.45% Imp	pervious Are	rea			
_				. .				
Tc	Length	Slope	,	Capacity	•			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			
					-			

Summary for Subcatchment 209: To Swale

Runoff = 0.02 cfs @ 12.12 hrs, Volume= 94 cf, Depth> 0.29" Routed to Reach 5R : Swale

Α	rea (sf)	CN	Description				
	1,115	98	Paved park	ing, HSG B	3		
	2,743	61	75% Grass cover, Good, HSG B				
	3,858	72	Weighted A	verage			
	2,743		71.10% Pervious Area				
	1,115		28.90% Impervious Area				
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		
					•		

Summary for Subcatchment 210: To Swale

Runoff = 0.02 cfs @ 12.10 hrs, Volume= 65 cf, Depth> 0.52" Routed to Reach 1R : Swale

Are	ea (sf)	CN I	Description				
	724	98 I	Paved park	ing, HSG B			
	781	61 >	75% Grass cover, Good, HSG B				
	1,505	79	Veighted A	verage			
	781	Į	51.89% Pervious Area				
	724	4	18.11% Imp	pervious Ar	ea		
Tc I (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Subcatchment 211: To Swale

Runoff = 0.03 cfs @ 12.11 hrs, Volume= 117 cf, Depth> 0.35" Routed to Reach 2R : Swale

rea (sf)	CN [Description				
1,451	98 F	Paved park	ing, HSG B	}		
2,581	61 >	75% Grass cover, Good, HSG B				
4,032	74 \	Veighted A	verage			
2,581	6	64.01% Pervious Area				
1,451	3	85.99% Imp	pervious Are	ea		
Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
				Direct Entry,		
	1,451 2,581 4,032 2,581 1,451 Length	1,451 98 F 2,581 61 > 4,032 74 V 2,581 6 1,451 3 Length Slope	1,451 98 Paved park 2,581 61 >75% Grass 4,032 74 Weighted A 2,581 64.01% Per 1,451 35.99% Imp Length Slope Velocity	1,45198Paved parking, HSG E2,58161>75% Grass cover, Go4,03274Weighted Average2,58164.01% Pervious Area1,45135.99% Impervious ArLengthSlopeVelocityCapacity		

Summary for Subcatchment 212: To Swale

Runoff = 0.02 cfs @ 12.11 hrs, Volume= 72 cf, Depth> 0.32" Routed to Reach 3R : Swale

A	rea (sf)	CN	Description				
	894	98	Paved park	ing, HSG B	3		
	1,825	61	>75% Grass cover, Good, HSG B				
	2,719	73	Weighted A	verage			
	1,825		67.12% Pervious Area				
	894		32.88% Imp	pervious Ar	rea		
Та	l e ve exte	Clana	Volesity	Consolity	Description		
Tc	Length	Slope	,	Capacity	1		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 213: Direct to Infiltration Pond

Runoff = 0.01 cfs @ 12.50 hrs, Volume= 265 cf, Depth> 0.07" Routed to Pond P1 : Infiltration Pond

Are	ea (sf)	CN	Description				
	1,912	98	Paved park	ing, HSG B			
3	39,571	61	>75% Ġras	s cover, Go	od, HSG B		
	2,227	39	>75% Gras	s cover, Go	od, HSG A		
4	13,710	61	61 Weighted Average				
4	1,798		95.63% Per	vious Area			
	1,912		4.37% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Subcatchment 214: Direct to Wetland

Runoff	=	0.06 cfs @	12.66 hrs,	Volume=	1,021 cf,	Depth>	0.10"
Routed	d to Link	AP1: Wetland	ds				

A	rea (sf)	CN [Description		
	7,280	98 F	Paved park	ing, HSG B	
	96,435	61 >	•75% Gras	s cover, Go	ood, HSG B
	9,806	55 V	Voods, Go	od, HSG B	
	3,519		Roofs, HSG		
	3,940			,	ood, HSG A
	1,050	30 V	Voods, Go	od, HSG A	
	22,030		Veighted A		
1	11,231	-	-	vious Area	
	10,799	8	8.85% Impe	ervious Are	а
То	Longth	Slope	Volocity	Capacity	Description
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
				(013)	Shoot Elow
9.5	50	0.1600	0.09		Sheet Flow, Woods: Dense underbrush n= 0.800 P2= 3.10"
0.4	41	0.1000	1.58		
0.4	41	0.1000	1.50		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
11.0	401	0.0075	0.61		Shallow Concentrated Flow,
11.0	+01	0.0075	0.01		Short Grass Pasture Kv= 7.0 fps
20.9	492	Total			
20.9	432	TULAI			

Summary for Subcatchment 215: To Swale

Runoff = 0.02 cfs @ 12.11 hrs, Volume= 101 cf, Depth> 0.35" Routed to Reach 7R : Swale

ea (sf)	CN I	Description				
1,179	98 I	Paved parki	ing, HSG B	3		
2,297	61 3	75% Grass cover, Good, HSG B				
3,476	74	Neighted A	verage			
2,297	(6.08% Per	vious Area	1		
1,179	:	33.92% Impervious Area				
Length (feet)		Velocity (ft/sec)	Capacity (cfs)	Description		
				Direct Entry,		
	1,179 2,297 3,476 2,297 1,179 _ength	1,179 98 F 2,297 61 ≥ 3,476 74 \ 2,297 6 1,179 3 _ength Slope	1,179 98 Paved park 2,297 61 >75% Grass 3,476 74 Weighted A 2,297 66.08% Per 1,179 33.92% Imp _ength Slope Velocity	1,17998Paved parking, HSG E2,29761>75% Grass cover, Go3,47674Weighted Average2,29766.08% Pervious Area1,17933.92% Impervious Ar_engthSlopeVelocityCapacity		

Summary for Reach 1R: Swale

10,652 sf, 60.66% Impervious, Inflow Depth > 0.71" for 2-Inch event Inflow Area = Inflow = 0.20 cfs @ 12.09 hrs, Volume= 632 cf = 0.20 cfs @ 12.10 hrs, Volume= Outflow 632 cf, Atten= 0%, Lag= 0.2 min Routed to Reach 2R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.62 fps, Min. Travel Time= 0.2 min Avg. Velocity = 0.65 fps, Avg. Travel Time= 0.6 min Peak Storage= 3 cf @ 12.10 hrs Average Depth at Peak Storage= 0.07', Surface Width= 1.86' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 69.88 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 24.0' Slope= 0.0417 '/' Inlet Invert= 256.00', Outlet Invert= 255.00'

Summary for Reach 2R: Swale

[62] Hint: Exceeded Reach 1R OUTLET depth by 0.11' @ 12.14 hrs

 Inflow Area =
 32,191 sf, 59.32% Impervious, Inflow Depth > 0.71" for 2-Inch event

 Inflow =
 0.59 cfs @
 12.10 hrs, Volume=
 1,907 cf

 Outflow =
 0.56 cfs @
 12.12 hrs, Volume=
 1,902 cf, Atten= 5%, Lag= 1.7 min

 Routed to Reach 3R : Swale
 Swale
 1,902 cf, Atten= 5%, Lag= 1.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.56 fps, Min. Travel Time= 2.5 min Avg. Velocity = 0.53 fps, Avg. Travel Time= 7.2 min

Peak Storage= 83 cf @ 12.12 hrs Average Depth at Peak Storage= 0.18', Surface Width= 2.42' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.10 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 230.0' Slope= 0.0130 '/' Inlet Invert= 255.00', Outlet Invert= 252.00'

Summary for Reach 3R: Swale

[62] Hint: Exceeded Reach 2R OUTLET depth by 0.04' @ 12.17 hrs

Inflow Area =52,857 sf, 54.28% Impervious, Inflow Depth > 0.63" for 2-Inch eventInflow =0.80 cfs @12.12 hrs, Volume=2,754 cfOutflow =0.78 cfs @12.13 hrs, Volume=2,749 cf, Atten= 2%, Lag= 1.0 minRouted to Pond 4R : Pipe to Infiltration PondPondPond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.74 fps, Min. Travel Time= 1.4 min Avg. Velocity = 0.61 fps, Avg. Travel Time= 4.0 min

Peak Storage= 67 cf @ 12.13 hrs Average Depth at Peak Storage= 0.22', Surface Width= 2.59' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.66 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 149.0' Slope= 0.0134 '/' Inlet Invert= 252.00', Outlet Invert= 250.00'

Summary for Reach 5R: Swale

10,041 sf, 64.56% Impervious, Inflow Depth > 0.92" for 2-Inch event Inflow Area = 0.24 cfs @ 12.09 hrs. Volume= Inflow = 770 cf = 0.23 cfs @ 12.11 hrs, Volume= 769 cf, Atten= 2%, Lag= 1.1 min Outflow Routed to Reach 6R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.42 fps, Min. Travel Time= 1.6 min Avg. Velocity = 0.50 fps, Avg. Travel Time= 4.6 min Peak Storage= 23 cf @ 12.11 hrs Average Depth at Peak Storage= 0.09', Surface Width= 1.97' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 52.35 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 139.0' Slope= 0.0234 '/' Inlet Invert= 257.00', Outlet Invert= 253.75'

Summary for Reach 6R: Swale

[62] Hint: Exceeded Reach 5R OUTLET depth by 0.11' @ 12.13 hrs

 Inflow Area =
 33,680 sf, 57.70% Impervious, Inflow Depth > 0.73" for 2-Inch event

 Inflow =
 0.62 cfs @
 12.10 hrs, Volume=
 2,059 cf

 Outflow =
 0.60 cfs @
 12.12 hrs, Volume=
 2,055 cf, Atten= 3%, Lag= 1.3 min

 Routed to Reach 7R : Swale
 Swale
 2,055 cf, Atten= 3%, Lag= 1.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.48 fps, Min. Travel Time= 1.9 min Avg. Velocity = 0.47 fps, Avg. Travel Time= 5.9 min

Peak Storage= 68 cf @ 12.12 hrs Average Depth at Peak Storage= 0.20', Surface Width= 2.51' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 35.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 167.0' Slope= 0.0105 '/' Inlet Invert= 253.75', Outlet Invert= 252.00'

Summary for Reach 7R: Swale

[62] Hint: Exceeded Reach 6R OUTLET depth by 0.09' @ 12.20 hrs

Inflow Area =56,601 sf, 53.74% Impervious, Inflow Depth > 0.65" for 2-Inch eventInflow =0.89 cfs @12.11 hrs, Volume=3,066 cfOutflow =0.82 cfs @12.15 hrs, Volume=3,055 cf, Atten= 8%, Lag= 2.3 minRouted to Reach 8R : Box Culvert to Infiltration Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.31 fps, Min. Travel Time= 3.3 min Avg. Velocity = 0.43 fps, Avg. Travel Time= 10.0 min

Peak Storage= 161 cf @ 12.15 hrs Average Depth at Peak Storage= 0.28', Surface Width= 2.91' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 26.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 259.0' Slope= 0.0058 '/' Inlet Invert= 252.00', Outlet Invert= 250.50'

Summary for Reach 8R: Box Culvert to Infiltration Pond

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area =56,601 sf, 53.74% Impervious, Inflow Depth > 0.65" for 2-Inch eventInflow =0.82 cfs @ 12.15 hrs, Volume=3,055 cfOutflow =0.82 cfs @ 12.15 hrs, Volume=3,054 cf, Atten= 0%, Lag= 0.1 minRouted to Pond P1 : Infiltration PondAtten

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 0.84 fps, Min. Travel Time= 0.1 min Avg. Velocity = 0.24 fps, Avg. Travel Time= 0.4 min

Peak Storage= 6 cf @ 12.15 hrs Average Depth at Peak Storage= 0.19', Surface Width= 5.00' Bank-Full Depth= 1.50' Flow Area= 7.5 sf, Capacity= 13.65 cfs

60.0" W x 18.0" H Box Pipe n= 0.040 Earth, cobble bottom, clean sides Length= 6.0' Slope= 0.0050 '/' Inlet Invert= 250.00', Outlet Invert= 249.97'

Summary for Pond 4R: Pipe to Infiltration Pond

[62] Hint: Exceeded Reach 3R OUTLET depth by 0.11' @ 12.15 hrs

Inflow Are	a =	52,857 sf	, 54.28% Impervious,	Inflow Depth > 0.62" for 2-Inch event			
Inflow	=	0.78 cfs @	12.13 hrs, Volume=	2,749 cf			
Outflow	=	0.78 cfs @	12.14 hrs, Volume=	2,748 cf, Atten= 0%, Lag= 0.3 min			
Primary	=	0.78 cfs @	12.14 hrs, Volume=	2,748 cf			
Routed to Pond P1 : Infiltration Pond							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 250.33' @ 12.14 hrs Surf.Area= 74 sf Storage= 16 cf

Plug-Flow detention time= 0.7 min calculated for 2,747 cf (100% of inflow) Center-of-Mass det. time= 0.4 min (865.7 - 865.3)

Volume	Inve	ert Avail.Sto	orage Storage	e Description	
#1	250.0)0' 6	20 cf Custor	m Stage Data (Pi	rismatic)Listed below (Recalc)
Elevation (feet)		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
250.00		22	0	0	
251.00		181	102	102	
252.00		855	518	620	
Device R	louting	Invert	Outlet Devic	es	
#1 P	rimary	250.00'	Inlet / Outlet	Invert= 250.00' /	L= 55.0' Ke= 0.500 249.72' S= 0.0051 '/' Cc= 0.900 ooth interior, Flow Area= 1.23 sf
Primary O	utFlow	Max=0 78 cfs	@ 12 14 hrs H	W=250 33' TW=	249 27' (Dynamic Tailwater)

Primary OutFlow Max=0.78 cfs @ 12.14 hrs HW=250.33' TW=249.27' (Dynamic Tailwater) -1=Culvert (Barrel Controls 0.78 cfs @ 2.29 fps)

Summary for Pond P1: Infiltration Pond

Inflow Area =	170,916 sf	, 44.06% Impervious,	Inflow Depth > 0.55" for 2-Inch event	
Inflow =	2.06 cfs @	12.13 hrs, Volume=	7,787 cf	
Outflow =	1.60 cfs @	12.22 hrs, Volume=	7,785 cf, Atten= 23%, Lag= 5.5 mi	n
Discarded =	1.60 cfs @	12.22 hrs, Volume=	7,785 cf	
Primary =	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Routed to Link	AP1 : Wetland	ds		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 249.28' @ 12.22 hrs Surf.Area= 8,231 sf Storage= 244 cf

Plug-Flow detention time= 1.1 min calculated for 7,782 cf (100% of inflow) Center-of-Mass det. time= 0.9 min (860.0 - 859.0)

Volume	Inver	t Avail.Sto	rage Stor	age Description		
#1	249.25	5' 24,64	49 cf Cus	tom Stage Data (P	rismatic)Listed below	w (Recalc)
Elevatio (fee 249.2 250.0 251.0 251.7	25 00 00	Gurf.Area (sq-ft) 8,187 9,297 10,866 12,024	Inc.Store (cubic-feet 6,55 10,082 8,01) (cubic-feet)) 0 7 6,557 2 16,638		
Device	Routing	Invert	Outlet De	vices		
#1	Primary	249.90'			Broad-Crested Recta	
#2	Discarded	249.25'	Coef. (En 8.270 in/ł	glish) 2.68 2.70 2. In Exfiltration over	0.80 1.00 1.20 1.40 .70 2.64 2.63 2.64 Surface area Elevation = 247.00'	

Discarded OutFlow Max=1.60 cfs @ 12.22 hrs HW=249.28' (Free Discharge) **2=Exfiltration** (Controls 1.60 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=249.25' TW=0.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link AP1: Wetlands

Inflow Area	a =	292,946 sf,	29.39% Impervious,	Inflow Depth > 0.	.04" for 2-Inch event
Inflow	=	0.06 cfs @ 1	12.66 hrs, Volume=	1,021 cf	
Primary	=	0.06 cfs @	12.66 hrs, Volume=	1,021 cf,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

PostDevelopment Prepared by Howard Stein Hudson <u>HydroCAD® 10.10-6a</u> s/n M05607 © 2020 Hyd	Type III 24-hr 2-yr Rainfall=3.27" Printed 4/8/2022 IroCAD Software Solutions LLC Page 35
Runoff by SCS TR	4.00 hrs, dt=0.01 hrs, 2401 points x 3 -20 method, UH=SCS, Weighted-CN method - Pond routing by Dyn-Stor-Ind method
Subcatchment 201: to Under Sidewalk Un	it Runoff Area=17,748 sf 80.43% Impervious Runoff Depth>2.32" Tc=6.0 min CN=91 Runoff=1.09 cfs 3,434 cf
Subcatchment 202: To RGT 1	Runoff Area=17,947 sf 48.49% Impervious Runoff Depth>1.39" Tc=6.0 min CN=79 Runoff=0.66 cfs 2,075 cf
Subcatchment 203: To RGT 2	Runoff Area=17,507 sf 63.88% Impervious Runoff Depth>1.82" Tc=6.0 min CN=85 Runoff=0.86 cfs 2,649 cf
Subcatchment 204: To RGT 3	Runoff Area=9,147 sf 62.73% Impervious Runoff Depth>1.74" Tc=6.0 min CN=84 Runoff=0.43 cfs 1,326 cf
Subcatchment 205: To RGT 4	Runoff Area=6,183 sf 86.80% Impervious Runoff Depth>2.51" Tc=6.0 min CN=93 Runoff=0.40 cfs 1,294 cf
Subcatchment206: To RGT 5	Runoff Area=18,720 sf 60.91% Impervious Runoff Depth>1.74" Tc=6.0 min CN=84 Runoff=0.88 cfs 2,713 cf
Subcatchment207: To RGT 6	Runoff Area=19,445 sf 50.44% Impervious Runoff Depth>1.45" Tc=6.0 min CN=80 Runoff=0.76 cfs 2,356 cf
Subcatchment208: To Swale	Runoff Area=4,919 sf 31.45% Impervious Runoff Depth>1.03" Tc=6.0 min CN=73 Runoff=0.13 cfs 421 cf
Subcatchment209: To Swale	Runoff Area=3,858 sf 28.90% Impervious Runoff Depth>0.97" Tc=6.0 min CN=72 Runoff=0.09 cfs 312 cf
Subcatchment210: To Swale	Runoff Area=1,505 sf 48.11% Impervious Runoff Depth>1.39" Tc=6.0 min CN=79 Runoff=0.06 cfs 174 cf
Subcatchment211: To Swale	Runoff Area=4,032 sf 35.99% Impervious Runoff Depth>1.08" Tc=6.0 min CN=74 Runoff=0.11 cfs 364 cf
Subcatchment212: To Swale	Runoff Area=2,719 sf 32.88% Impervious Runoff Depth>1.03" Tc=6.0 min CN=73 Runoff=0.07 cfs 233 cf
Subcatchment 213: Direct to Infiltration	Runoff Area=43,710 sf 4.37% Impervious Runoff Depth>0.47" Tc=6.0 min CN=61 Runoff=0.36 cfs 1,719 cf
Subcatchment 214: Direct to Wetland	Runoff Area=122,030 sf 8.85% Impervious Runoff Depth>0.55" Flow Length=492' Tc=20.9 min CN=63 Runoff=0.89 cfs 5,561 cf
Subcatchment215: To Swale	Runoff Area=3,476 sf 33.92% Impervious Runoff Depth>1.08" Tc=6.0 min CN=74 Runoff=0.10 cfs 313 cf
	Avg. Flow Depth=0.12' Max Vel=2.21 fps Inflow=0.48 cfs 1,500 cf 24.0' S=0.0417 '/' Capacity=69.88 cfs Outflow=0.48 cfs 1,499 cf

PostDevelopment Prepared by Howard Steir HydroCAD® 10.10-6a s/n M05	Type III 24-hr2-yr Rainfall=3.27"n HudsonPrinted 4/8/20226607 © 2020 HydroCAD Software Solutions LLCPage 36
Reach 2R: Swale	Avg. Flow Depth=0.30' Max Vel=2.05 fps Inflow=1.45 cfs 4,512 cf n=0.030 L=230.0' S=0.0130 '/' Capacity=39.10 cfs Outflow=1.40 cfs 4,503 cf
Reach 3R: Swale	Avg. Flow Depth=0.37' Max Vel=2.32 fps Inflow=2.12 cfs 6,811 cf n=0.030 L=149.0' S=0.0134 '/' Capacity=39.66 cfs Outflow=2.10 cfs 6,804 cf
Reach 5R: Swale	Avg. Flow Depth=0.14' Max Vel=1.82 fps Inflow=0.50 cfs 1,606 cf n=0.030 L=139.0' S=0.0234 '/' Capacity=52.35 cfs Outflow=0.49 cfs 1,604 cf
Reach 6R: Swale	Avg. Flow Depth=0.33' Max Vel=1.92 fps Inflow=1.49 cfs 4,738 cf n=0.030 L=167.0' S=0.0105 '/' Capacity=35.05 cfs Outflow=1.46 cfs 4,731 cf
Reach 7R: Swale	Avg. Flow Depth=0.47' Max Vel=1.73 fps Inflow=2.30 cfs 7,401 cf n=0.030 L=259.0' S=0.0058 '/' Capacity=26.05 cfs Outflow=2.17 cfs 7,383 cf
	Avg. Flow Depth=0.36' Max Vel=1.21 fps Inflow=2.17 cfs 7,383 cf Pipe n=0.040 L=6.0' S=0.0050 '/' Capacity=13.65 cfs Outflow=2.17 cfs 7,383 cf
Pond 4R: Pipe to Infiltratio	n Pond Peak Elev=250.55' Storage=37 cf Inflow=2.10 cfs 6,804 cf 6.0" Round Culvert x 2.00 n=0.012 L=55.0' S=0.0051 '/' Outflow=2.09 cfs 6,802 cf
Pond P1: Infiltration Pond	Peak Elev=249.64' Storage=3,263 cf Inflow=5.60 cfs 19,337 cf Discarded=1.95 cfs 19,333 cf Primary=0.00 cfs 0 cf Outflow=1.95 cfs 19,333 cf
Link AP1: Wetlands	Inflow=0.89 cfs 5,561 cf Primary=0.89 cfs 5,561 cf

Total Runoff Area = 292,946 sf Runoff Volume = 24,943 cf Average Runoff Depth = 1.02" 70.61% Pervious = 206,849 sf 29.39% Impervious = 86,097 sf

Summary for Subcatchment 201: to Under Sidewalk Unit

Runoff = 1.09 cfs @ 12.09 hrs, Volume= 3,434 cf, Depth> 2.32" Routed to Pond P1 : Infiltration Pond

A	rea (sf)	CN	Description				
	13,365	3,365 98 Paved parking, HSG B					
	3,474	61	>75% Gras	s cover, Go	ood, HSG B		
	909	98	Roofs, HSG	БВ			
	17,748	91	Weighted A	verage			
	3,474 19.57% Pervious Area				a		
	14,274 80.43% Impervious Area				rea		
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 202: To RGT 1

Runoff = 0.66 cfs @ 12.09 hrs, Volume= 2,075 cf, Depth> 1.39" Routed to Reach 3R : Swale

A	rea (sf)	CN	Description					
	4,676	98	Paved park	ing, HSG B	В			
	9,245	61	>75% Ġras	s cover, Go	ood, HSG B			
	4,026	98						
	17,947	79	Weighted A	verage				
	9,245 51.51% Pervious Area							
	8,702	4	48.49% Impervious Area					
Та	l e e este	Clana	Valasity	Consolity	Description			
Tc (min)	Length	Slope		Capacity	1			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment 203: To RGT 2

Runoff = 0.86 cfs @ 12.09 hrs, Volume= 2,649 cf, Depth> 1.82" Routed to Reach 2R : Swale

Α	rea (sf)	CN	Description				
	7,022	,022 98 Paved parking, HSG B					
	6,324	61	>75% Gras	s cover, Go	bod, HSG B		
	4,161	98	Roofs, HSO	βB			
	17,507	85	Weighted A	verage			
	6,324 36.12% Pervious Area				1		
	11,183	(63.88% Imp	pervious Ar	ea		
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Subcatchment 204: To RGT 3

Runoff = 0.43 cfs @ 12.09 hrs, Volume= 1,326 cf, Depth> 1.74" Routed to Reach 1R : Swale

	Area (sf)	CN	Description						
	3,638		Paved parking, HSG B						
	3,409	61	>75% Gras	s cover, Go	ood, HSG B				
	2,100	98	Roofs, HSC	Э В					
	9,147	9,147 84 Weighted Average							
	3,409		37.27% Pervious Area						
	5,738		62.73% Impervious Area						
Т	5	Slope	,	Capacity	Description				
(min) (feet)	(ft/ft) (ft/sec)	(cfs)					
6.0)				Direct Entry,				
(min	3,409 5,738 c Length) (feet)		37.27% Pei 62.73% Imp e Velocity	rvious Area pervious Ar	ea Description				

Summary for Subcatchment 205: To RGT 4

Runoff = 0.40 cfs @ 12.09 hrs, Volume= 1,294 cf, Depth> 2.51" Routed to Reach 5R : Swale

Α	rea (sf)	CN	Description						
	4,135	98	1 07						
	816	61	>75% Gras	s cover, Go	ood, HSG B				
	1,232	98	Roofs, HSC	βB					
	6,183 93 Weighted Average								
	816	• •							
	5,367	367 86.80% Impervious Area							
_		<u> </u>							
ŢĊ	Length	Slope	,	Capacity	1				
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment 206: To RGT 5

Runoff = 0.88 cfs @ 12.09 hrs, Volume= 2,713 cf, Depth> 1.74" Routed to Reach 6R : Swale

A	rea (sf)	CN I	Description					
	7,564	98 Paved parking, HSG B						
	7,317	61 :	>75% Gras	s cover, Go	od, HSG B			
	3,839	98	Roofs, HSC	βB				
	18,720	18,720 84 Weighted Average						
	7,317	4	39.09% Pei	vious Area				
	11,403	(60.91% Imp	pervious Ar	a			
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment 207: To RGT 6

Runoff = 0.76 cfs @ 12.09 hrs, Volume= 2,356 cf, Depth> 1.45" Routed to Reach 7R : Swale

Are	ea (sf)	CN	Description						
	6,428	98							
	9,636	61	>75% Gras	s cover, Go	od, HSG B				
	3,381	98	Roofs, HSC	6 B					
1	9,445 80 Weighted Average								
	9,636								
	9,809		50.44% Impervious Area						
_									
	Length	Slope	,	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment 208: To Swale

Runoff = 0.13 cfs @ 12.10 hrs, Volume= 421 cf, Depth> 1.03" Routed to Reach 6R : Swale

A	rea (sf)	CN	Description						
	1,547	98	Paved parking, HSG B						
	3,372	61	-75% Grass cover, Good, HSG B						
	4,919	73	Weighted A	verage					
	3,372		68.55% Per	vious Area	3				
	1,547		31.45% Impervious Area						
_									
Тс	Length	Slope	,	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				
					-				

Summary for Subcatchment 209: To Swale

Runoff = 0.09 cfs @ 12.10 hrs, Volume= 312 cf, Depth> 0.97" Routed to Reach 5R : Swale

Α	rea (sf)	CN	Description					
	1,115	98	Paved park	ing, HSG B	В			
	2,743	61	75% Grass cover, Good, HSG B					
	3,858	72	Weighted A	verage				
	2,743		71.10% Per	vious Area	а			
	1,115		28.90% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)				
6.0					Direct Entry,			

Summary for Subcatchment 210: To Swale

Runoff = 0.06 cfs @ 12.09 hrs, Volume= 174 cf, Depth> 1.39" Routed to Reach 1R : Swale

) CN	Description						
98	Paved parking, HSG B						
61	>75% Gras	>75% Grass cover, Good, HSG B					
5 79	Weighted A	verage					
	51.89% Per	vious Area					
724 48.11% Impervious Area							
	,	Capacity (cfs)	Description				
			Direct Entry,				
	98 61 5 79 h Slop	98 Paved parki 61 >75% Grass 79 Weighted A 51.89% Per 48.11% Imp h Slope Velocity	98 Paved parking, HSG E 61 >75% Grass cover, Go 79 Weighted Average 51.89% Pervious Area 48.11% Impervious Ar h Slope Velocity Capacity				

Summary for Subcatchment 211: To Swale

Runoff = 0.11 cfs @ 12.10 hrs, Volume= 364 cf, Depth> 1.08" Routed to Reach 2R : Swale

Paved parking, HSG B						
•75% Grass cover, Good, HSG B						
35.99% Impervious Area						

Summary for Subcatchment 212: To Swale

Runoff = 0.07 cfs @ 12.10 hrs, Volume= 233 cf, Depth> 1.03" Routed to Reach 3R : Swale

A	rea (sf)	CN	Description					
	894	98	Paved park	ing, HSG B	3			
	1,825	61	>75% Gras	s cover, Go	ood, HSG B			
	2,719	73	Weighted Average					
	1,825		67.12% Pervious Area					
	894		32.88% Impervious Area					
Та	l e ve exte	Clana	Volesity	Consolity	Description			
Tc	Length	Slope	,	Capacity	1			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment 213: Direct to Infiltration Pond

Runoff = 0.36 cfs @ 12.12 hrs, Volume= 1,719 cf, Depth> 0.47" Routed to Pond P1 : Infiltration Pond

A	rea (sf)	CN [Description				
	1,912	98 F	Paved park	ing, HSG B	В		
	39,571	61 >	>75% Ġras	s cover, Go	lood, HSG B		
	2,227	39 >	>75% Gras	s cover, Go	lood, HSG A		
	43,710	61 \	Weighted Average				
	41,798	ę	95.63% Per	vious Area	а		
	1,912	2	4.37% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	I I I I I I I I I I I I I I I I I I I		
6.0					Direct Entry,		

Summary for Subcatchment 214: Direct to Wetland

Runoff = 0.89 cfs @ 12.38 hrs, Volume= 5,561 cf, Depth> 0.55" Routed to Link AP1 : Wetlands

Α	rea (sf)	CN E	Description						
	7,280		Paved parking, HSG B						
	96,435	61 >	75% Gras	s cover, Go	ood, HSG B				
	9,806	55 V	Voods, Go	od, HSG B					
	3,519		Roofs, HSG						
	3,940			,	ood, HSG A				
	1,050	30 V	Voods, Go	od, HSG A					
1	22,030	63 V	Veighted A	verage					
	11,231	-		vious Area					
	10,799	8	.85% Impe	ervious Area	а				
Та	l a sa aith	Clana	Volocity	Consitu	Description				
Tc (min)	Length	Slope	Velocity (ft/sec)	Capacity	Description				
(min)	(feet)	(ft/ft)	/	(cfs)					
9.5	50	0.1600	0.09		Sheet Flow,				
0.4	4.4	0 1000	1 50		Woods: Dense underbrush n= 0.800 P2= 3.10"				
0.4	41	0.1000	1.58		Shallow Concentrated Flow,				
11.0	401	0 0075	Woodland Kv= 5.0 fps						
11.0	401	0.0075	0.61		Shallow Concentrated Flow,				
	400	T . 4 . 1			Short Grass Pasture Kv= 7.0 fps				
20.9	492	Total							

Summary for Subcatchment 215: To Swale

Runoff = 0.10 cfs @ 12.10 hrs, Volume= 313 cf, Depth> 1.08" Routed to Reach 7R : Swale

ea (sf)	CN I	Description				
1,179	98 I	Paved parki	ing, HSG B	3		
2,297	61 3	>75% Grass	s cover, Go	bod, HSG B		
3,476	74	Weighted Average				
2,297	(66.08% Pervious Area				
1,179	:	33.92% Impervious Area				
Length (feet)		Velocity (ft/sec)	Capacity (cfs)	Description		
				Direct Entry,		
	1,179 2,297 3,476 2,297 1,179 _ength	1,179 98 F 2,297 61 ≥ 3,476 74 \ 2,297 6 1,179 3 _ength Slope	1,179 98 Paved park 2,297 61 >75% Grass 3,476 74 Weighted A 2,297 66.08% Per 1,179 33.92% Imp _ength Slope Velocity	1,179 98 Paved parking, HSG E 2,297 61 >75% Grass cover, Go 3,476 74 Weighted Average 2,297 66.08% Pervious Area 1,179 33.92% Impervious Ar		

Summary for Reach 1R: Swale

10,652 sf, 60.66% Impervious, Inflow Depth > 1.69" for 2-yr event Inflow Area = Inflow = 0.48 cfs @ 12.09 hrs, Volume= 1.500 cf 0.48 cfs @ 12.09 hrs, Volume= Outflow = 1,499 cf, Atten= 0%, Lag= 0.1 min Routed to Reach 2R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.21 fps, Min. Travel Time= 0.2 min Avg. Velocity = 0.73 fps, Avg. Travel Time= 0.6 min Peak Storage= 5 cf @ 12.09 hrs Average Depth at Peak Storage= 0.12', Surface Width= 2.11' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 69.88 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 24.0' Slope= 0.0417 '/' Inlet Invert= 256.00', Outlet Invert= 255.00'

Summary for Reach 2R: Swale

[62] Hint: Exceeded Reach 1R OUTLET depth by 0.18' @ 12.13 hrs

 Inflow Area =
 32,191 sf, 59.32% Impervious, Inflow Depth >
 1.68" for 2-yr event

 Inflow =
 1.45 cfs @
 12.09 hrs, Volume=
 4,512 cf

 Outflow =
 1.40 cfs @
 12.11 hrs, Volume=
 4,503 cf, Atten= 3%, Lag= 1.3 min

 Routed to Reach 3R : Swale
 1.211 hrs, Volume=
 1.211 hrs, Volume=
 1.211 hrs, Volume=

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.05 fps, Min. Travel Time= 1.9 min Avg. Velocity = 0.67 fps, Avg. Travel Time= 5.7 min

Peak Storage= 157 cf @ 12.11 hrs Average Depth at Peak Storage= 0.30', Surface Width= 3.01' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.10 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 230.0' Slope= 0.0130 '/' Inlet Invert= 255.00', Outlet Invert= 252.00'

Summary for Reach 3R: Swale

[62] Hint: Exceeded Reach 2R OUTLET depth by 0.07' @ 12.15 hrs

Inflow Area =52,857 sf, 54.28% Impervious, Inflow Depth > 1.55" for 2-yr eventInflow =2.12 cfs @ 12.11 hrs, Volume=6,811 cfOutflow =2.10 cfs @ 12.12 hrs, Volume=6,804 cf, Atten= 1%, Lag= 0.8 minRouted to Pond 4R : Pipe to Infiltration Pond804 cf, Atten= 1%, Lag= 0.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.32 fps, Min. Travel Time= 1.1 min Avg. Velocity = 0.78 fps, Avg. Travel Time= 3.2 min

Peak Storage= 134 cf @ 12.12 hrs Average Depth at Peak Storage= 0.37', Surface Width= 3.36' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.66 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 149.0' Slope= 0.0134 '/' Inlet Invert= 252.00', Outlet Invert= 250.00'

Summary for Reach 5R: Swale

10,041 sf, 64.56% Impervious, Inflow Depth > 1.92" for 2-yr event Inflow Area = Inflow = 0.50 cfs @ 12.09 hrs, Volume= 1.606 cf = 0.49 cfs @ 12.10 hrs, Volume= Outflow 1,604 cf, Atten= 2%, Lag= 0.9 min Routed to Reach 6R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.82 fps, Min. Travel Time= 1.3 min Avg. Velocity = 0.56 fps, Avg. Travel Time= 4.1 min Peak Storage= 37 cf @ 12.10 hrs Average Depth at Peak Storage= 0.14', Surface Width= 2.22' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 52.35 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 139.0' Slope= 0.0234 '/' Inlet Invert= 257.00', Outlet Invert= 253.75'

Summary for Reach 6R: Swale

[62] Hint: Exceeded Reach 5R OUTLET depth by 0.18' @ 12.12 hrs

 Inflow Area =
 33,680 sf, 57.70% Impervious, Inflow Depth > 1.69" for 2-yr event

 Inflow =
 1.49 cfs @
 12.09 hrs, Volume=
 4,738 cf

 Outflow =
 1.46 cfs @
 12.11 hrs, Volume=
 4,731 cf, Atten= 2%, Lag= 1.1 min

 Routed to Reach 7R : Swale
 Swale
 1.46 cfs
 1.46 cfs

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.92 fps, Min. Travel Time= 1.5 min Avg. Velocity = 0.58 fps, Avg. Travel Time= 4.8 min

Peak Storage= 127 cf @ 12.11 hrs Average Depth at Peak Storage= 0.33', Surface Width= 3.14' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 35.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 167.0' Slope= 0.0105 '/' Inlet Invert= 253.75', Outlet Invert= 252.00'

Summary for Reach 7R: Swale

[62] Hint: Exceeded Reach 6R OUTLET depth by 0.16' @ 12.18 hrs

Inflow Area =56,601 sf, 53.74% Impervious, Inflow Depth >1.57" for 2-yr eventInflow =2.30 cfs @12.10 hrs, Volume=7,401 cfOutflow =2.17 cfs @12.13 hrs, Volume=7,383 cf, Atten= 5%, Lag= 1.8 minRouted to Reach 8R : Box Culvert to Infiltration Pond7,383 cf, Atten= 5%, Lag= 1.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.73 fps, Min. Travel Time= 2.5 min Avg. Velocity = 0.54 fps, Avg. Travel Time= 8.0 min

Peak Storage= 325 cf @ 12.13 hrs Average Depth at Peak Storage= 0.47' , Surface Width= 3.85' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 26.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 259.0' Slope= 0.0058 '/' Inlet Invert= 252.00', Outlet Invert= 250.50'

Summary for Reach 8R: Box Culvert to Infiltration Pond

[52] Hint: Inlet/Outlet conditions not evaluated

 Inflow Area =
 56,601 sf, 53.74% Impervious, Inflow Depth > 1.57" for 2-yr event

 Inflow =
 2.17 cfs @ 12.13 hrs, Volume=
 7,383 cf

 Outflow =
 2.17 cfs @ 12.13 hrs, Volume=
 7,383 cf, Atten= 0%, Lag= 0.1 min

 Routed to Pond P1 : Infiltration Pond
 56,601 sf, 53.74% Impervious, Inflow Depth > 1.57"

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.21 fps, Min. Travel Time= 0.1 min Avg. Velocity = 0.31 fps, Avg. Travel Time= 0.3 min

Peak Storage= 11 cf @ 12.13 hrs Average Depth at Peak Storage= 0.36' , Surface Width= 5.00' Bank-Full Depth= 1.50' Flow Area= 7.5 sf, Capacity= 13.65 cfs

60.0" W x 18.0" H Box Pipe n= 0.040 Earth, cobble bottom, clean sides Length= 6.0' Slope= 0.0050 '/' Inlet Invert= 250.00', Outlet Invert= 249.97'

Summary for Pond 4R: Pipe to Infiltration Pond

[62] Hint: Exceeded Reach 3R OUTLET depth by 0.18' @ 12.13 hrs

Inflow Are	a =	52,857 sf	, 54.28% Impervious,	Inflow Depth > 1.54" for 2-yr event				
Inflow	=	2.10 cfs @	12.12 hrs, Volume=	6,804 cf				
Outflow	=	2.09 cfs @	12.12 hrs, Volume=	6,802 cf, Atten= 0%, Lag= 0.3 min				
Primary	=	2.09 cfs @	12.12 hrs, Volume=	6,802 cf				
Routed to Pond P1 : Infiltration Pond								

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 250.55' @ 12.12 hrs Surf.Area= 110 sf Storage= 37 cf

Plug-Flow detention time= 0.5 min calculated for 6,802 cf (100% of inflow) Center-of-Mass det. time= 0.4 min (838.6 - 838.2)

Volume	Inve	ert Avail.Sto	orage Storag	e Description			
#1	250.0	00' 6	20 cf Custo	m Stage Data (P	rismatic)Listed below (Recalc)		
Elevation (feet)		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
250.00		22	0	0			
251.00		181	102	102			
252.00		855	518	620			
Device R	Routing	Invert	Outlet Devic	ces			
#1 P	Primary	250.00'	Inlet / Outlet	t Invert= 250.00' /	L= 55.0' Ke= 0.500 249.72' S= 0.0051 '/' Cc= 0.900 ooth interior, Flow Area= 1.23 sf		
Primary OutFlow Max=2.09 cfs @ 12.12 hrs HW=250.55' TW=249.42' (Dynamic Tailwater)							

Primary OutFlow Max=2.09 cfs @ 12.12 hrs HW=250.55' TW=249.42' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 2.09 cfs @ 2.94 fps)

Summary for Pond P1: Infiltration Pond

Inflow Area = 170,916 sf, 44.06% Impervious, Inflow Depth > 1.36" for 2-yr event Inflow = 5.60 cfs @ 12.12 hrs, Volume= 19.337 cf 1.95 cfs @ 12.47 hrs, Volume= 19,333 cf, Atten= 65%, Lag= 21.0 min Outflow = 1.95 cfs @ 12.47 hrs, Volume= Discarded = 19.333 cf Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Link AP1 : Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 249.64' @ 12.47 hrs Surf.Area= 8,757 sf Storage= 3,263 cf

Plug-Flow detention time= 9.0 min calculated for 19,333 cf (100% of inflow) Center-of-Mass det. time= 8.9 min (846.3 - 837.4)

Volume	Inver	t Avail.Sto	rage S	Storage	Description		
#1	249.25	' 24,64	49 cf 🕻	O cf Custom Stage Data (Prismatic)Listed below (Recalc)		w (Recalc)	
Elevatio (fee 249.2 250.0 251.0 251.7	et) 25 00 00	Surf.Area (sq-ft) 8,187 9,297 10,866 12,024	10		Cum.Store (cubic-feet) 0 6,557 16,638 24,649		
Device	Routing	Invert	Outlet	Device	s		
#1	Primary	249.90'				road-Crested Recta	
#2	2 Discarded 24		Coef. 8.270	Èngĺisł in/hr E	n) 2.68 2.70 2. xfiltration over	0.80 1.00 1.20 1.4 70 2.64 2.63 2.64 Surface area Elevation = 247.00'	

Discarded OutFlow Max=1.95 cfs @ 12.47 hrs HW=249.64' (Free Discharge) **2=Exfiltration** (Controls 1.95 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=249.25' TW=0.00' (Dynamic Tailwater) **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link AP1: Wetlands

Inflow Area	a =	292,946 sf, 29.39% Impervious, Inflow Depth > 0.23" for 2-yr ev	/ent
Inflow	=	0.89 cfs @ 12.38 hrs, Volume= 5,561 cf	
Primary	=	0.89 cfs @ 12.38 hrs, Volume= 5,561 cf, Atten= 0%, Lag	= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

PostDevelopment Prepared by Howard Stein Hudson <u>HydroCAD® 10.10-6a_s/n M05607_© 2020 Hyd</u>	<i>Type III 24-hr 10-yr Rainfall=4.92"</i> Printed 4/8/2022 IroCAD Software Solutions LLC Page 62
Runoff by SCS TR	4.00 hrs, dt=0.01 hrs, 2401 points x 3 -20 method, UH=SCS, Weighted-CN method - Pond routing by Dyn-Stor-Ind method
Subcatchment 201: to Under Sidewalk Un	it Runoff Area=17,748 sf 80.43% Impervious Runoff Depth>3.90" Tc=6.0 min CN=91 Runoff=1.79 cfs 5,769 cf
Subcatchment 202: To RGT 1	Runoff Area=17,947 sf 48.49% Impervious Runoff Depth>2.73" Tc=6.0 min CN=79 Runoff=1.32 cfs 4,083 cf
Subcatchment 203: To RGT 2	Runoff Area=17,507 sf 63.88% Impervious Runoff Depth>3.29" Tc=6.0 min CN=85 Runoff=1.54 cfs 4,801 cf
Subcatchment 204: To RGT 3	Runoff Area=9,147 sf 62.73% Impervious Runoff Depth>3.19" Tc=6.0 min CN=84 Runoff=0.78 cfs 2,435 cf
Subcatchment 205: To RGT 4	Runoff Area=6,183 sf 86.80% Impervious Runoff Depth>4.12" Tc=6.0 min CN=93 Runoff=0.65 cfs 2,121 cf
Subcatchment 206: To RGT 5	Runoff Area=18,720 sf 60.91% Impervious Runoff Depth>3.19" Tc=6.0 min CN=84 Runoff=1.60 cfs 4,983 cf
Subcatchment 207: To RGT 6	Runoff Area=19,445 sf 50.44% Impervious Runoff Depth>2.82" Tc=6.0 min CN=80 Runoff=1.48 cfs 4,570 cf
Subcatchment208: To Swale	Runoff Area=4,919 sf 31.45% Impervious Runoff Depth>2.22" Tc=6.0 min CN=73 Runoff=0.29 cfs 908 cf
Subcatchment 209: To Swale	Runoff Area=3,858 sf 28.90% Impervious Runoff Depth>2.13" Tc=6.0 min CN=72 Runoff=0.22 cfs 686 cf
Subcatchment210: To Swale	Runoff Area=1,505 sf 48.11% Impervious Runoff Depth>2.73" Tc=6.0 min CN=79 Runoff=0.11 cfs 342 cf
Subcatchment211: To Swale	Runoff Area=4,032 sf 35.99% Impervious Runoff Depth>2.30" Tc=6.0 min CN=74 Runoff=0.25 cfs 772 cf
Subcatchment212: To Swale	Runoff Area=2,719 sf 32.88% Impervious Runoff Depth>2.22" Tc=6.0 min CN=73 Runoff=0.16 cfs 502 cf
Subcatchment 213: Direct to Infiltration	Runoff Area=43,710 sf 4.37% Impervious Runoff Depth>1.32" Tc=6.0 min CN=61 Runoff=1.41 cfs 4,805 cf
Subcatchment 214: Direct to Wetland	Runoff Area=122,030 sf 8.85% Impervious Runoff Depth>1.45" low Length=492' Tc=20.9 min CN=63 Runoff=2.92 cfs 14,744 cf
Subcatchment215: To Swale	Runoff Area=3,476 sf 33.92% Impervious Runoff Depth>2.30" Tc=6.0 min CN=74 Runoff=0.21 cfs 666 cf
	Avg. Flow Depth=0.17' Max Vel=2.69 fps Inflow=0.89 cfs 2,777 cf 24.0' S=0.0417 '/' Capacity=69.88 cfs Outflow=0.89 cfs 2,777 cf

PostDevelopment Prepared by Howard Stein Hudson <u>HydroCAD® 10.10-6a</u> s/n M05607 © 202	Type III 24-hr 10-yr Rainfall=4.92"Printed 4/8/2022O HydroCAD Software Solutions LLCPage 63
Reach 2R: Swale n=0.030	Avg. Flow Depth=0.42' Max Vel=2.44 fps Inflow=2.68 cfs 8,350 cf L=230.0' S=0.0130 '/' Capacity=39.10 cfs Outflow=2.61 cfs 8,338 cf
Reach 3R: Swale n=0.030	Avg. Flow Depth=0.52' Max Vel=2.78 fps Inflow=4.07 cfs 12,922 cf L=149.0' S=0.0134 '/' Capacity=39.66 cfs Outflow=4.03 cfs 12,912 cf
Reach 5R: Swale n=0.030	Avg. Flow Depth=0.20' Max Vel=2.17 fps Inflow=0.86 cfs 2,807 cf L=139.0' S=0.0234 '/' Capacity=52.35 cfs Outflow=0.85 cfs 2,804 cf
Reach 6R: Swale n=0.030	Avg. Flow Depth=0.45' Max Vel=2.28 fps Inflow=2.74 cfs 8,694 cf L=167.0' S=0.0105 '/' Capacity=35.05 cfs Outflow=2.70 cfs 8,685 cf
Reach 7R: Swale n=0.030	Avg. Flow Depth=0.65' Max Vel=2.06 fps Inflow=4.36 cfs 13,920 cf L=259.0' S=0.0058 '/' Capacity=26.05 cfs Outflow=4.18 cfs 13,896 cf
	Avg. Flow Depth=0.54' Max Vel=1.54 fps Inflow=4.18 cfs 13,896 cf L=6.0' S=0.0050 '/' Capacity=13.65 cfs Outflow=4.18 cfs 13,895 cf
Pond 4R: Pipe to Infiltration Pond 15.0" Round C	Peak Elev=250.80' Storage=69 cf Inflow=4.03 cfs 12,912 cf ulvert x 2.00 n=0.012 L=55.0' S=0.0051 '/' Outflow=4.03 cfs 12,910 cf
Pond P1: Infiltration Pond Discarded=2.4	Peak Elev=250.11' Storage=7,635 cf Inflow=11.24 cfs 37,379 cf 6 cfs 33,776 cf Primary=2.67 cfs 3,596 cf Outflow=5.13 cfs 37,372 cf
Link AP1: Wetlands	Inflow=5.59 cfs 18,340 cf Primary=5.59 cfs 18,340 cf

Total Runoff Area = 292,946 sf Runoff Volume = 52,185 cf Average Runoff Depth = 2.14" 70.61% Pervious = 206,849 sf 29.39% Impervious = 86,097 sf

Summary for Subcatchment 201: to Under Sidewalk Unit

Runoff = 1.79 cfs @ 12.08 hrs, Volume= 5,769 cf, Depth> 3.90" Routed to Pond P1 : Infiltration Pond

A	rea (sf)	CN	Description				
	13,365	98	Paved park	ing, HSG B			
	3,474	61	>75% Gras	s cover, Go	od, HSG B		
	909	98	Roofs, HSG	βB			
	17,748	91	Weighted Average				
	3,474		19.57% Per	vious Area			
	14,274	i	80.43% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Subcatchment 202: To RGT 1

Runoff = 1.32 cfs @ 12.09 hrs, Volume= 4,083 cf, Depth> 2.73" Routed to Reach 3R : Swale

A	rea (sf)	CN	Description					
	4,676	98	Paved park	ing, HSG B	В			
	9,245	61	>75% Ġras	s cover, Go	ood, HSG B			
	4,026	98	Roofs, HSC	БВ				
	17,947	79	Weighted A	verage				
	9,245		51.51% Pei	vious Area	а			
	8,702		48.49% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	1			
6.0					Direct Entry,			

Summary for Subcatchment 203: To RGT 2

Runoff = 1.54 cfs @ 12.09 hrs, Volume= 4,801 cf, Depth> 3.29" Routed to Reach 2R : Swale

Α	rea (sf)	CN I	Description						
	7,022	98 I	Paved park	ing, HSG B	В				
	6,324	61 >	>75% Gras	s cover, Go	ood, HSG B				
	4,161	98 I	Roofs, HSG	БВ					
	17,507	85 \	Neighted A	verage					
	6,324	3	36.12% Per	vious Area	a				
	11,183	6	63.88% Imp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	1				
6.0					Direct Entry,				

Summary for Subcatchment 204: To RGT 3

Runoff = 0.78 cfs @ 12.09 hrs, Volume= 2,435 cf, Depth> 3.19" Routed to Reach 1R : Swale

A	rea (sf)	CN	Description						
	3,638	98	Paved park	ing, HSG B	1				
	3,409	61	>75% Gras	s cover, Go	ood, HSG B				
	2,100	98	Roofs, HSC	βB					
	9,147	84	Weighted Average						
	3,409	;	37.27% Pei	vious Area					
	5,738		62.73% Imp	pervious Ar	ea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0	(1		()	()	Direct Entry,				

Summary for Subcatchment 205: To RGT 4

Runoff = 0.65 cfs @ 12.08 hrs, Volume= 2,121 cf, Depth> 4.12" Routed to Reach 5R : Swale

A	rea (sf)	CN	Description						
	4,135	98	Paved park	ing, HSG B	3				
	816	61	>75% Gras	s cover, Go	ood, HSG B				
	1,232	98	Roofs, HSC	βB					
	6,183	93	3 Weighted Average						
	816		13.20% Pervious Area						
	5,367		86.80% Imp	pervious Ar	rea				
_									
Tc	Length	Slope	,	Capacity					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment 206: To RGT 5

Runoff = 1.60 cfs @ 12.09 hrs, Volume= 4,983 cf, Depth> 3.19" Routed to Reach 6R : Swale

Α	rea (sf)	CN I	Description						
	7,564	98	Paved park	ing, HSG B	3				
	7,317	61 :	>75% Gras	s cover, Go	ood, HSG B				
	3,839	98	Roofs, HSC	βB					
	18,720	84	Weighted Average						
	7,317	4	39.09% Pei	vious Area	a				
	11,403	(60.91% Imp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)					
6.0					Direct Entry,				

Summary for Subcatchment 207: To RGT 6

Runoff = 1.48 cfs @ 12.09 hrs, Volume= 4,570 cf, Depth> 2.82" Routed to Reach 7R : Swale

Α	rea (sf)	CN	Description						
	6,428	98	Paved park	ing, HSG B					
	9,636	61	>75% Ġras	s cover, Go	od, HSG B				
	3,381	98	Roofs, HSO	βB					
	19,445	80	Weighted A	verage					
	9,636		49.56% Pei	vious Area					
	9,809	:	50.44% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment 208: To Swale

Runoff = 0.29 cfs @ 12.09 hrs, Volume= 908 cf, Depth> 2.22" Routed to Reach 6R : Swale

A	rea (sf)	CN I	Description		
	1,547	98	Paved park	ing, HSG B	В
	3,372	61 :	>75% Gras	s cover, Go	ood, HSG B
	4,919	73	Neighted A	verage	
	3,372	(58.55% Per	vious Area	a
	1,547	:	31.45% Imp	pervious Are	rea
_				. .	
Tc	Length	Slope	,	Capacity	•
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,
					-

Summary for Subcatchment 209: To Swale

Runoff = 0.22 cfs @ 12.09 hrs, Volume= 686 cf, Depth> 2.13" Routed to Reach 5R : Swale

Α	rea (sf)	CN	Description						
	1,115	98	Paved park	ing, HSG B	3				
	2,743	61	>75% Gras	s cover, Go	ood, HSG B				
	3,858	72	Weighted A	verage					
	2,743		71.10% Per	vious Area	a				
	1,115		28.90% Imp	pervious Are	rea				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)					
6.0					Direct Entry,				

Summary for Subcatchment 210: To Swale

Runoff = 0.11 cfs @ 12.09 hrs, Volume= 342 cf, Depth> 2.73" Routed to Reach 1R : Swale

Are	ea (sf)	CN I	Description						
	724	98 I	Paved park	ing, HSG B					
	781	61 >	>75% Ġras	s cover, Go	bod, HSG B				
	1,505	79	Veighted A	verage					
	781	Į	51.89% Per	vious Area					
	724	4	48.11% Impervious Area						
Tc I (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment 211: To Swale

Runoff = 0.25 cfs @ 12.09 hrs, Volume= 772 cf, Depth> 2.30" Routed to Reach 2R : Swale

rea (sf)	CN [Description		
1,451	98 F	Paved park	ing, HSG B	}
2,581	61 >	75% Gras	s cover, Go	bod, HSG B
4,032	74 \	Veighted A	verage	
2,581	6	64.01% Per	vious Area	
1,451	3	85.99% Imp	pervious Are	ea
Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
				Direct Entry,
	1,451 2,581 4,032 2,581 1,451 Length	1,451 98 F 2,581 61 > 4,032 74 V 2,581 6 1,451 3 Length Slope	1,451 98 Paved park 2,581 61 >75% Grass 4,032 74 Weighted A 2,581 64.01% Per 1,451 35.99% Imp Length Slope Velocity	1,45198Paved parking, HSG E2,58161>75% Grass cover, Go4,03274Weighted Average2,58164.01% Pervious Area1,45135.99% Impervious ArLengthSlopeVelocityCapacity

Summary for Subcatchment 212: To Swale

Runoff = 0.16 cfs @ 12.09 hrs, Volume= 502 cf, Depth> 2.22" Routed to Reach 3R : Swale

sf) CN	Description	Description						
94 98	Paved park	ing, HSG B	3					
25 61	>75% Gras	s cover, Go	ood, HSG B					
19 73	Weighted A	Weighted Average						
25	67.12% Per	vious Area	a					
94	32.88% Imp	ervious Ar	ea					
•		Capacity (cfs)	Description					
			Direct Entry,					
	94 98 25 61 19 73 25 94 ngth Slo	94 98 Paved park 25 61 >75% Grass 19 73 Weighted A 25 67.12% Per 94 32.88% Imp ngth Slope Velocity	9498Paved parking, HSG E2561>75% Grass cover, Gr1973Weighted Average2567.12% Pervious Area9432.88% Impervious AreangthSlopeVelocity					

Summary for Subcatchment 213: Direct to Infiltration Pond

Runoff = 1.41 cfs @ 12.10 hrs, Volume= 4,805 cf, Depth> 1.32" Routed to Pond P1 : Infiltration Pond

A	rea (sf)	CN	Description						
	1,912	98	Paved park	ing, HSG B	3				
;	39,571	61	>75% Gras	s cover, Go	bod, HSG B				
	2,227	39	>75% Gras	s cover, Go	bod, HSG A				
	43,710	61	61 Weighted Average						
	41,798		95.63% Per	vious Area					
	1,912		4.37% Impe	ervious Area	а				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment 214: Direct to Wetland

Runoff = 2.92 cfs @ 12.33 hrs, Volume= 14,744 cf, Depth> 1.45" Routed to Link AP1 : Wetlands

A	rea (sf)	CN [Description		
	7,280	98 F	Paved park	ing, HSG B	
	96,435	61 >	•75% Gras	s cover, Go	ood, HSG B
	9,806	55 V	Voods, Go	od, HSG B	
	3,519		Roofs, HSG		
	3,940				ood, HSG A
	1,050	<u> 30 </u>	Voods, Go	od, HSG A	
1	22,030	63 V	Veighted A	verage	
1	11,231	ç	91.15% Pei	vious Area	
	10,799	8	3.85% Imp€	ervious Area	а
Та	Longeth	Clana	Valasity	Conseitu	Description
Tc (min)	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.5	50	0.1600	0.09		Sheet Flow,
0.4	4.4	0 4 0 0 0	4 50		Woods: Dense underbrush n= 0.800 P2= 3.10"
0.4	41	0.1000	1.58		Shallow Concentrated Flow,
11.0	404	0 0075	0.64		Woodland Kv= 5.0 fps
11.0	401	0.0075	0.61		Shallow Concentrated Flow,
	400	- · ·			Short Grass Pasture Kv= 7.0 fps
20.9	492	Total			

Summary for Subcatchment 215: To Swale

Runoff = 0.21 cfs @ 12.09 hrs, Volume= 666 cf, Depth> 2.30" Routed to Reach 7R : Swale

a (sf)	CN E	Description			
,179	98 F	Paved parking, HSG B			
2,297	61 >	>75% Grass cover, Good, HSG B			
,476	74 V	4 Weighted Average			
2,297	6	66.08% Pervious Area			
,179	33.92% Impervious Area				
ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
				Direct Entry,	
,	,179 ,297 ,476 ,297 ,179 ength	,179 98 F ,297 61 > ,476 74 V ,297 6 ,179 3 ength Slope	,179 98 Paved parki ,297 61 >75% Grass ,476 74 Weighted A ,297 66.08% Per ,179 33.92% Imp ength Slope Velocity	,179 98 Paved parking, HSG E ,297 61 >75% Grass cover, Go ,476 74 Weighted Average ,297 66.08% Pervious Area ,179 33.92% Impervious Ar	

Summary for Reach 1R: Swale

10,652 sf, 60.66% Impervious, Inflow Depth > 3.13" for 10-yr event Inflow Area = Inflow = 0.89 cfs @ 12.09 hrs, Volume= 2.777 cf 0.89 cfs @ 12.09 hrs, Volume= Outflow = 2,777 cf, Atten= 0%, Lag= 0.1 min Routed to Reach 2R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.69 fps, Min. Travel Time= 0.1 min Avg. Velocity = 0.81 fps, Avg. Travel Time= 0.5 min Peak Storage= 8 cf @ 12.09 hrs Average Depth at Peak Storage= 0.17', Surface Width= 2.36' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 69.88 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 24.0' Slope= 0.0417 '/' Inlet Invert= 256.00', Outlet Invert= 255.00'

Summary for Reach 2R: Swale

[62] Hint: Exceeded Reach 1R OUTLET depth by 0.25' @ 12.12 hrs

 Inflow Area =
 32,191 sf, 59.32% Impervious, Inflow Depth > 3.11" for 10-yr event

 Inflow =
 2.68 cfs @
 12.09 hrs, Volume=
 8,350 cf

 Outflow =
 2.61 cfs @
 12.11 hrs, Volume=
 8,338 cf, Atten= 3%, Lag= 1.1 min

 Routed to Reach 3R : Swale
 Swale
 10 - yr event

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.44 fps, Min. Travel Time= 1.6 min Avg. Velocity = 0.78 fps, Avg. Travel Time= 4.9 min

Peak Storage= 245 cf @ 12.11 hrs Average Depth at Peak Storage= 0.42' , Surface Width= 3.59' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.10 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 230.0' Slope= 0.0130 '/' Inlet Invert= 255.00', Outlet Invert= 252.00'

Summary for Reach 3R: Swale

[62] Hint: Exceeded Reach 2R OUTLET depth by 0.10' @ 12.13 hrs

 Inflow Area =
 52,857 sf, 54.28% Impervious, Inflow Depth > 2.93" for 10-yr event

 Inflow =
 4.07 cfs @
 12.10 hrs, Volume=
 12,922 cf

 Outflow =
 4.03 cfs @
 12.11 hrs, Volume=
 12,912 cf, Atten= 1%, Lag= 0.7 min

 Routed to Pond 4R : Pipe to Infiltration Pond
 Pond
 Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.78 fps, Min. Travel Time= 0.9 min Avg. Velocity = 0.91 fps, Avg. Travel Time= 2.7 min

Peak Storage= 216 cf @ 12.11 hrs Average Depth at Peak Storage= 0.52' , Surface Width= 4.09' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.66 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 149.0' Slope= 0.0134 '/' Inlet Invert= 252.00', Outlet Invert= 250.00'

Summary for Reach 5R: Swale

10,041 sf, 64.56% Impervious, Inflow Depth > 3.35" for 10-yr event Inflow Area = Inflow = 0.86 cfs @ 12.09 hrs, Volume= 2.807 cf = 0.85 cfs @ 12.10 hrs, Volume= Outflow 2,804 cf, Atten= 1%, Lag= 0.7 min Routed to Reach 6R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.17 fps, Min. Travel Time= 1.1 min Avg. Velocity = 0.63 fps, Avg. Travel Time= 3.7 min Peak Storage= 55 cf @ 12.10 hrs Average Depth at Peak Storage= 0.20', Surface Width= 2.49' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 52.35 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 139.0' Slope= 0.0234 '/' Inlet Invert= 257.00', Outlet Invert= 253.75'

Summary for Reach 6R: Swale

[62] Hint: Exceeded Reach 5R OUTLET depth by 0.25' @ 12.11 hrs

 Inflow Area =
 33,680 sf, 57.70% Impervious, Inflow Depth > 3.10" for 10-yr event

 Inflow =
 2.74 cfs @
 12.09 hrs, Volume=
 8,694 cf

 Outflow =
 2.70 cfs @
 12.11 hrs, Volume=
 8,685 cf, Atten= 2%, Lag= 0.9 min

 Routed to Reach 7R : Swale
 Swale
 10 - yr event

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.28 fps, Min. Travel Time= 1.2 min Avg. Velocity = 0.68 fps, Avg. Travel Time= 4.1 min

Peak Storage= 198 cf @ 12.11 hrs Average Depth at Peak Storage= 0.45', Surface Width= 3.75' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 35.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 167.0' Slope= 0.0105 '/' Inlet Invert= 253.75', Outlet Invert= 252.00'

Summary for Reach 7R: Swale

[62] Hint: Exceeded Reach 6R OUTLET depth by 0.21' @ 12.16 hrs

Inflow Area = 56,601 sf, 53.74% Impervious, Inflow Depth > 2.95" for 10-yr event Inflow = 4.36 cfs @ 12.10 hrs, Volume= 13,920 cf Outflow = 4.18 cfs @ 12.12 hrs, Volume= 13,896 cf, Atten= 4%, Lag= 1.5 min Routed to Reach 8R : Box Culvert to Infiltration Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.06 fps, Min. Travel Time= 2.1 min Avg. Velocity = 0.64 fps, Avg. Travel Time= 6.7 min

Peak Storage= 525 cf @ 12.12 hrs Average Depth at Peak Storage= 0.65' , Surface Width= 4.74' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 26.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 259.0' Slope= 0.0058 '/' Inlet Invert= 252.00', Outlet Invert= 250.50'

Summary for Reach 8R: Box Culvert to Infiltration Pond

[52] Hint: Inlet/Outlet conditions not evaluated [61] Hint: Exceeded Reach 7R outlet invert by 0.04' @ 12.13 hrs

 Inflow Area =
 56,601 sf, 53.74% Impervious, Inflow Depth > 2.95" for 10-yr event

 Inflow =
 4.18 cfs @
 12.12 hrs, Volume=
 13,896 cf

 Outflow =
 4.18 cfs @
 12.13 hrs, Volume=
 13,895 cf, Atten= 0%, Lag= 0.0 min

 Routed to Pond P1 : Infiltration Pond
 Pond
 Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.54 fps, Min. Travel Time= 0.1 min Avg. Velocity = 0.37 fps, Avg. Travel Time= 0.3 min

Peak Storage= 16 cf @ 12.13 hrs Average Depth at Peak Storage= 0.54' , Surface Width= 5.00' Bank-Full Depth= 1.50' Flow Area= 7.5 sf, Capacity= 13.65 cfs

60.0" W x 18.0" H Box Pipe n= 0.040 Earth, cobble bottom, clean sides Length= 6.0' Slope= 0.0050 '/' Inlet Invert= 250.00', Outlet Invert= 249.97'

Summary for Pond 4R: Pipe to Infiltration Pond

[62] Hint: Exceeded Reach 3R OUTLET depth by 0.29' @ 12.12 hrs

Inflow Area	a =	52,857 sf,	54.28% Impervious,	Inflow Depth > 2.93" for 10-yr event				
Inflow	=	4.03 cfs @	12.11 hrs, Volume=	12,912 cf				
Outflow	=	4.03 cfs @	12.12 hrs, Volume=	12,910 cf, Atten= 0%, Lag= 0.3 min				
Primary	=	4.03 cfs @	12.12 hrs, Volume=	12,910 cf				
Routed to Pond P1 : Infiltration Pond								

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 250.80' @ 12.12 hrs Surf.Area= 150 sf Storage= 69 cf

Plug-Flow detention time= 0.4 min calculated for 12,904 cf (100% of inflow) Center-of-Mass det. time= 0.3 min (820.2 - 819.9)

<u>Volume</u> #1	Invo 250.0		0 0	escription	ismatic)Listed below (Recalc)		
#1	200.0	0 0		Slaye Dala (Pr	ISINALICICIES DEIOW (RECAIC)		
Elevatior (feet	•	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
250.00)	22	0	0			
251.00)	181	102	102			
252.00)	855	518	620			
Device	Routing	Invert	Outlet Devices				
#1	Primary	250.00'	Inlet / Outlet Inv	vert= 250.00' / 2	L= 55.0' Ke= 0.500 249.72' S= 0.0051 '/' Cc= 0.900 poth interior, Flow Area= 1.23 sf		
Primary OutFlow Max=4 02 cfs @ 12 12 hrs HW=250 80' TW=249 79' (Dynamic Tailwater)							

Primary OutFlow Max=4.02 cfs @ 12.12 hrs HW=250.80' TW=249.79' (Dynamic Tailwater) -1=Culvert (Barrel Controls 4.02 cfs @ 3.43 fps)

Summary for Pond P1: Infiltration Pond

[61] Hint: Exceeded Reach 8R outlet invert by 0.14' @ 12.33 hrs

Inflow Area = 170,916 sf, 44.06% Impervious, Inflow Depth > 2.62" for 10-yr event Inflow 11.24 cfs @ 12.11 hrs, Volume= = 37,379 cf Outflow = 5.13 cfs @ 12.33 hrs, Volume= 37,372 cf, Atten= 54%, Lag= 13.3 min 2.46 cfs @ 12.33 hrs, Volume= Discarded = 33,776 cf 2.67 cfs @ 12.33 hrs, Volume= Primary = 3,596 cf Routed to Link AP1 : Wetlands

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 250.11' @ 12.33 hrs Surf.Area= 9,477 sf Storage= 7,635 cf

Plug-Flow detention time= 16.6 min calculated for 37,372 cf (100% of inflow) Center-of-Mass det. time= 16.5 min (837.7 - 821.2)

Volume	Invert	t Avail.Sto	rage	Storage	Description		
#1	249.25	' 24,64	49 cf	Custom	Stage Data (P	rismatic)Listed below	w (Recalc)
Elevation (feet) 249.25 250.00 251.00 251.70		urf.Area (sq-ft) 8,187 9,297 10,866 12,024	(cubic ((Store <u>-feet)</u> 0 6,557 0,082 8,011	Cum.Store (cubic-feet) 0 6,557 16,638 24,649		
Device F	Routing	Invert	Outle	t Devices	6		
#1 F	Primary	249.90'	10.0'	long x 2	22.0' breadth B	road-Crested Recta	Ingular Weir
#2 [Discarded 249.25'		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63 8.270 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 247.00' Phase-In= 0.01'				

Discarded OutFlow Max=2.46 cfs @ 12.33 hrs HW=250.11' (Free Discharge) **2=Exfiltration** (Controls 2.46 cfs)

Primary OutFlow Max=2.67 cfs @ 12.33 hrs HW=250.11' TW=0.00' (Dynamic Tailwater) **1=Broad-Crested Rectangular Weir** (Weir Controls 2.67 cfs @ 1.24 fps)

Summary for Link AP1: Wetlands

Inflow Are	a =	292,946 sf, 29.39% Impervious, Ir	nflow Depth > 0.75"	for 10-yr event
Inflow	=	5.59 cfs @ 12.33 hrs, Volume=	18,340 cf	
Primary	=	5.59 cfs @ 12.33 hrs, Volume=	18,340 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

PostDevelopment Prepared by Howard Stein Hudson <u>HydroCAD® 10.10-6a_s/n M05607_© 2020 Hyd</u>	<i>Type III 24-hr 50-yr Rainfall=7.42"</i> Printed 4/8/2022 IroCAD Software Solutions LLC Page 89
Runoff by SCS TR	4.00 hrs, dt=0.01 hrs, 2401 points x 3 -20 method, UH=SCS, Weighted-CN method - Pond routing by Dyn-Stor-Ind method
Subcatchment 201: to Under Sidewalk Un	it Runoff Area=17,748 sf 80.43% Impervious Runoff Depth>6.35" Tc=6.0 min CN=91 Runoff=2.83 cfs 9,387 cf
Subcatchment 202: To RGT 1	Runoff Area=17,947 sf 48.49% Impervious Runoff Depth>4.97" Tc=6.0 min CN=79 Runoff=2.38 cfs 7,426 cf
Subcatchment 203: To RGT 2	Runoff Area=17,507 sf 63.88% Impervious Runoff Depth>5.65" Tc=6.0 min CN=85 Runoff=2.58 cfs 8,242 cf
Subcatchment 204: To RGT 3	Runoff Area=9,147 sf 62.73% Impervious Runoff Depth>5.53" Tc=6.0 min CN=84 Runoff=1.33 cfs 4,219 cf
Subcatchment 205: To RGT 4	Runoff Area=6,183 sf 86.80% Impervious Runoff Depth>6.58" Tc=6.0 min CN=93 Runoff=1.00 cfs 3,391 cf
Subcatchment 206: To RGT 5	Runoff Area=18,720 sf 60.91% Impervious Runoff Depth>5.53" Tc=6.0 min CN=84 Runoff=2.72 cfs 8,634 cf
Subcatchment 207: To RGT 6	Runoff Area=19,445 sf 50.44% Impervious Runoff Depth>5.08" Tc=6.0 min CN=80 Runoff=2.63 cfs 8,229 cf
Subcatchment208: To Swale	Runoff Area=4,919 sf 31.45% Impervious Runoff Depth>4.29" Tc=6.0 min CN=73 Runoff=0.57 cfs 1,761 cf
Subcatchment 209: To Swale	Runoff Area=3,858 sf 28.90% Impervious Runoff Depth>4.18" Tc=6.0 min CN=72 Runoff=0.44 cfs 1,345 cf
Subcatchment210: To Swale	Runoff Area=1,505 sf 48.11% Impervious Runoff Depth>4.97" Tc=6.0 min CN=79 Runoff=0.20 cfs 623 cf
Subcatchment 211: To Swale	Runoff Area=4,032 sf 35.99% Impervious Runoff Depth>4.41" Tc=6.0 min CN=74 Runoff=0.48 cfs 1,480 cf
Subcatchment 212: To Swale	Runoff Area=2,719 sf 32.88% Impervious Runoff Depth>4.29" Tc=6.0 min CN=73 Runoff=0.31 cfs 973 cf
Subcatchment 213: Direct to Infiltration	Runoff Area=43,710 sf 4.37% Impervious Runoff Depth>3.00" Tc=6.0 min CN=61 Runoff=3.48 cfs 10,945 cf
Subcatchment214: Direct to Wetland	Runoff Area=122,030 sf 8.85% Impervious Runoff Depth>3.20" low Length=492' Tc=20.9 min CN=63 Runoff=6.89 cfs 32,571 cf
Subcatchment 215: To Swale	Runoff Area=3,476 sf 33.92% Impervious Runoff Depth>4.41" Tc=6.0 min CN=74 Runoff=0.41 cfs 1,276 cf
	Avg. Flow Depth=0.23' Max Vel=3.17 fps Inflow=1.53 cfs 4,841 cf 24.0' S=0.0417 '/' Capacity=69.88 cfs Outflow=1.53 cfs 4,841 cf

PostDevelopment Prepared by Howard Steir HydroCAD® 10.10-6a s/n M05	Type III 24-hr 50-yr Raint n Hudson Printed 5607 © 2020 HydroCAD Software Solutions LLC	fall=7.42″ 4/8/2022 Page 90
Reach 2R: Swale	Avg. Flow Depth=0.55' Max Vel=2.84 fps Inflow=4.59 cfs n=0.030 L=230.0' S=0.0130 '/' Capacity=39.10 cfs Outflow=4.50 cfs	
Reach 3R: Swale	Avg. Flow Depth=0.68' Max Vel=3.24 fps Inflow=7.16 cfs n=0.030 L=149.0' S=0.0134 '/' Capacity=39.66 cfs Outflow=7.11 cfs	
Reach 5R: Swale	Avg. Flow Depth=0.26' Max Vel=2.53 fps Inflow=1.44 cf n=0.030 L=139.0' S=0.0234 '/' Capacity=52.35 cfs Outflow=1.43 cf	
Reach 6R: Swale	Avg. Flow Depth=0.59' Max Vel=2.64 fps Inflow=4.71 cfs n=0.030 L=167.0' S=0.0105 '/' Capacity=35.05 cfs Outflow=4.65 cfs	
Reach 7R: Swale	Avg. Flow Depth=0.85' Max Vel=2.40 fps Inflow=7.65 cfs n=0.030 L=259.0' S=0.0058 '/' Capacity=26.05 cfs Outflow=7.40 cfs	
	nfiltration Avg. Flow Depth=0.79' Max Vel=1.87 fps Inflow=7.40 cfs ipe n=0.040 L=6.0' S=0.0050 '/' Capacity=13.65 cfs Outflow=7.40 cfs	
Pond 4R: Pipe to Infiltration 15.0	n Pond Peak Elev=251.15' Storage=136 cf Inflow=7.11 cfs 0" Round Culvert x 2.00 n=0.012 L=55.0' S=0.0051 '/' Outflow=7.07 cfs	,
Pond P1: Infiltration Pond Discar	Peak Elev=250.48' Storage=11,191 cf Inflow=20.53 cfs rded=2.86 cfs 50,532 cf Primary=11.90 cfs 17,303 cf Outflow=14.76 cfs	
Link AP1: Wetlands	Inflow=17.91 cfs Primary=17.91 cfs	,

Total Runoff Area = 292,946 sf Runoff Volume = 100,502 cf Average Runoff Depth = 4.12" 70.61% Pervious = 206,849 sf 29.39% Impervious = 86,097 sf

Summary for Subcatchment 201: to Under Sidewalk Unit

Runoff = 2.83 cfs @ 12.08 hrs, Volume= 9,387 cf, Depth> 6.35" Routed to Pond P1 : Infiltration Pond

A	rea (sf)	CN I	Description				
	13,365	98	Paved park	ing, HSG B			
	3,474	61 3	>75% Ġras	s cover, Go	od, HSG B		
	909	98	Roofs, HSC	βB			
	17,748	91	Neighted A	verage			
	3,474	·	19.57% Pervious Area				
	14,274	ä	80.43% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Subcatchment 202: To RGT 1

Runoff = 2.38 cfs @ 12.09 hrs, Volume= 7,426 cf, Depth> 4.97" Routed to Reach 3R : Swale

A	rea (sf)	CN I	Description					
	4,676	98	Paved park	ing, HSG B	В			
	9,245	61 3	>75% Gras	s cover, Go	lood, HSG B			
	4,026	98	Roofs, HSC	БВ				
	17,947	79	Neighted A	verage				
	9,245	!	51.51% Pervious Area					
	8,702	4	48.49% Imp	pervious Ar	rea			
Та	Longth	Clana	Valaaitu	Consoitu	Description			
Tc (min)	Length (feet)	Slope	Velocity (ft/sec)	Capacity	· · · · · · · · · · · · · · · · · · ·			
<u>(min)</u>	(ieet)	(ft/ft)	(it/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment 203: To RGT 2

Runoff = 2.58 cfs @ 12.09 hrs, Volume= 8,242 cf, Depth> 5.65" Routed to Reach 2R : Swale

A	rea (sf)	CN I	Description					
	7,022	98	Paved park	ing, HSG B	3			
	6,324	61 :	>75% Gras	s cover, Go	bod, HSG B			
	4,161	98	Roofs, HSG	БВ				
	17,507	85	Neighted A	verage				
	6,324		36.12% Pervious Area					
	11,183	(63.88% Imp	pervious Ar	ea			
-		01		0				
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment 204: To RGT 3

Runoff = 1.33 cfs @ 12.09 hrs, Volume= 4,219 cf, Depth> 5.53" Routed to Reach 1R : Swale

/	Area (sf)	CN	Description				
	3,638		Paved park				
	3,409	61	>75% Gras	s cover, Go	ood, HSG B		
	2,100	98	Roofs, HSC	Э В			
	9,147	84	Weighted A	verage			
	3,409		37.27% Pei	rvious Area			
	5,738		62.73% Imp	pervious Ar	ea		
Тс	5	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
6.0					Direct Entry,		
(min)	(feet)	Slope (ft/ft)	,	Capacity (cfs)	•		

Summary for Subcatchment 205: To RGT 4

Runoff = 1.00 cfs @ 12.08 hrs, Volume= 3,391 cf, Depth> 6.58" Routed to Reach 5R : Swale

A	rea (sf)	CN	Description		
	4,135		Paved park		
	816	61	>75% Gras	s cover, Go	ood, HSG B
	1,232	98	Roofs, HSC	βB	
	6,183	93	Weighted A	verage	
	816		13.20% Pei	vious Area	a
	5,367		86.80% Imp	pervious Ar	rea
Tc	Length	Slope		Capacity	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment 206: To RGT 5

Runoff = 2.72 cfs @ 12.09 hrs, Volume= 8,634 cf, Depth> 5.53" Routed to Reach 6R : Swale

A	rea (sf)	CN	Description				
	7,564	98	Paved park	ing, HSG B	В		
	7,317	61	>75% Gras	s cover, Go	ood, HSG B		
	3,839	98	Roofs, HSO	БВ			
	18,720	84	Weighted A	verage			
	7,317	;	39.09% Pervious Area				
	11,403		60.91% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	1		
6.0					Direct Entry,		

Summary for Subcatchment 207: To RGT 6

Runoff = 2.63 cfs @ 12.09 hrs, Volume= 8,229 cf, Depth> 5.08" Routed to Reach 7R : Swale

Α	rea (sf)	CN	Description			
	6,428	98	Paved park	ing, HSG B		
	9,636	61	>75% Ġras	s cover, Go	od, HSG B	
	3,381	98	Roofs, HSO	βB		
	19,445	80	Weighted A	verage		
	9,636		49.56% Pei	vious Area		
	9,809	:	50.44% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
6.0					Direct Entry,	

Summary for Subcatchment 208: To Swale

Runoff = 0.57 cfs @ 12.09 hrs, Volume= 1,761 cf, Depth> 4.29" Routed to Reach 6R : Swale

(sf) C	CN D	escription		
547	98 P	aved parki	ng, HSG B	
372	61 >	75% Ġrass	s cover, Go	ood, HSG B
919	73 W	/eighted A	verage	
372	6	8.55% Per	vious Area	
547	3	1.45% Imp	ervious Are	ea
ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
				Direct Entry,
	547 372 919 372 547 ength	547 98 P 372 61 > 919 73 M 372 6 547 3 547 3 3 3	547 98 Paved parki 372 61 >75% Grass 919 73 Weighted A 372 68.55% Per 547 31.45% Imp ength Slope Velocity	54798Paved parking, HSG B37261>75% Grass cover, Go91973Weighted Average37268.55% Pervious Area54731.45% Impervious AreaengthSlopeVelocity

Summary for Subcatchment 209: To Swale

Runoff = 0.44 cfs @ 12.09 hrs, Volume= 1,345 cf, Depth> 4.18" Routed to Reach 5R : Swale

Α	rea (sf)	CN	Description		
	1,115	98	Paved park	ing, HSG B	В
	2,743	61	>75% Gras	s cover, Go	ood, HSG B
	3,858	72	Weighted A	verage	
	2,743		71.10% Per	vious Area	а
	1,115		28.90% Imp	pervious Are	rea
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	
6.0					Direct Entry,

Summary for Subcatchment 210: To Swale

Runoff = 0.20 cfs @ 12.09 hrs, Volume= 623 cf, Depth> 4.97" Routed to Reach 1R : Swale

sf) CN	Description			
24 98	Paved park	ing, HSG B	3	
81 61	>75% Ġras	s cover, Go	bod, HSG B	
05 79	Weighted A	verage		
81	51.89% Per	vious Area	l	
24	48.11% Impervious Area			
0		Capacity (cfs)	Description	
			Direct Entry,	
	24 98 81 61 05 79 81 24 ngth Slo	24 98 Paved park 81 61 >75% Gras 05 79 Weighted A 81 51.89% Per 24 48.11% Imp ngth Slope Velocity	2498Paved parking, HSG E8161>75% Grass cover, Go0579Weighted Average8151.89% Pervious Area2448.11% Impervious ArngthSlopeVelocity	

Summary for Subcatchment 211: To Swale

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 1,480 cf, Depth> 4.41" Routed to Reach 2R : Swale

A	rea (sf)	CN I	Description		
	1,451	98 I	Paved park	ing, HSG B	3
	2,581	61 >	-75% Gras	s cover, Go	bod, HSG B
	4,032	74 \	Veighted A	verage	
	2,581	6	64.01% Per	vious Area	
	1,451	(35.99% Imp	pervious Are	ea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment 212: To Swale

Runoff = 0.31 cfs @ 12.09 hrs, Volume= 973 cf, Depth> 4.29" Routed to Reach 3R : Swale

sf) CN	Description			
94 98	Paved park	ing, HSG B	3	
25 61	>75% Gras	s cover, Go	ood, HSG B	
19 73	Weighted A	verage		
25	67.12% Per	vious Area	a	
94	32.88% Impervious Area			
0		Capacity (cfs)	Description	
			Direct Entry,	
	94 98 25 61 19 73 25 94 ngth Slo	94 98 Paved park 25 61 >75% Gras 19 73 Weighted A 25 67.12% Per 94 32.88% Imp 19 Slope	98 Paved parking, HSG B 25 61 >75% Grass cover, G 719 73 Weighted Average 25 67.12% Pervious Area 32.88% Impervious Area 94 32.88% Impervious Area	

Summary for Subcatchment 213: Direct to Infiltration Pond

Runoff = 3.48 cfs @ 12.09 hrs, Volume= 10,945 cf, Depth> 3.00" Routed to Pond P1 : Infiltration Pond

Α	rea (sf)	CN I	Description			
	1,912	98	Paved park	ing, HSG B		
	39,571	61 :	>75% Gras	s cover, Go	od, HSG B	
	2,227	39 :	>75% Gras	s cover, Go	od, HSG A	
	43,710	61	Neighted A	verage		
	41,798	9	95.63% Per	vious Area		
	1,912	4	4.37% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
6.0					Direct Entry,	

Summary for Subcatchment 214: Direct to Wetland

Runoff = 6.89 cfs @ 12.29 hrs, Volume= 32,571 cf, Depth> 3.20" Routed to Link AP1 : Wetlands

Α	rea (sf)	CN E	Description		
	7,280	98 F	aved park	ing, HSG B	5
	96,435	61 >	75% Gras	s cover, Go	ood, HSG B
	9,806	55 V	Voods, Go	od, HSG B	
	3,519		Roofs, HSC		
	3,940				ood, HSG A
	1,050	30 V	Voods, Go	od, HSG A	
1	22,030		Veighted A		
1	11,231	-	-	vious Area	
	10,799	8	.85% Impe	ervious Area	а
Тс	Longth	Slope	Volocity	Capacity	Description
(min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	(cfs)	Description
	. ,		/	(013)	Chaot Flow
9.5	50	0.1600	0.09		Sheet Flow, Woods: Dense underbrush n= 0.800 P2= 3.10"
0.4	41	0.1000	1.58		
0.4	41	0.1000	1.50		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
11.0	401	0.0075	0.61		Shallow Concentrated Flow,
11.0	-01	0.0070	0.01		Short Grass Pasture Kv= 7.0 fps
20.9	492	Total			

Summary for Subcatchment 215: To Swale

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 1,276 cf, Depth> 4.41" Routed to Reach 7R : Swale

ea (sf)	CN I	Description		
1,179	98 I	Paved parki	ing, HSG B	3
2,297	61 3	>75% Grass	s cover, Go	bod, HSG B
3,476	74	Neighted A	verage	
2,297	(6.08% Per	vious Area	1
1,179	:	33.92% Imp	pervious Are	ea
Length (feet)		Velocity (ft/sec)	Capacity (cfs)	Description
				Direct Entry,
	1,179 2,297 3,476 2,297 1,179 _ength	1,179 98 F 2,297 61 ≥ 3,476 74 \ 2,297 6 1,179 3 _ength Slope	1,179 98 Paved park 2,297 61 >75% Grass 3,476 74 Weighted A 2,297 66.08% Per 1,179 33.92% Imp _ength Slope Velocity	1,17998Paved parking, HSG E2,29761>75% Grass cover, Go3,47674Weighted Average2,29766.08% Pervious Area1,17933.92% Impervious Ar_engthSlopeVelocityCapacity

Summary for Reach 1R: Swale

10,652 sf, 60.66% Impervious, Inflow Depth > 5.45" for 50-yr event Inflow Area = Inflow = 1.53 cfs @ 12.09 hrs, Volume= 4.841 cf 1.53 cfs @ 12.09 hrs, Volume= 4,841 cf, Atten= 0%, Lag= 0.1 min Outflow = Routed to Reach 2R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 3.17 fps, Min. Travel Time= 0.1 min Avg. Velocity = 0.94 fps, Avg. Travel Time= 0.4 min Peak Storage= 12 cf @ 12.09 hrs Average Depth at Peak Storage= 0.23', Surface Width= 2.66' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 69.88 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 24.0' Slope= 0.0417 '/' Inlet Invert= 256.00', Outlet Invert= 255.00'

Summary for Reach 2R: Swale

[62] Hint: Exceeded Reach 1R OUTLET depth by 0.32' @ 12.12 hrs

 Inflow Area =
 32,191 sf, 59.32% Impervious, Inflow Depth > 5.43" for 50-yr event

 Inflow =
 4.59 cfs @
 12.09 hrs, Volume=
 14,563 cf

 Outflow =
 4.50 cfs @
 12.10 hrs, Volume=
 14,548 cf, Atten= 2%, Lag= 1.0 min

 Routed to Reach 3R : Swale
 5.43
 5.43
 5.43

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.84 fps, Min. Travel Time= 1.4 min Avg. Velocity = 0.91 fps, Avg. Travel Time= 4.2 min

Peak Storage= 365 cf @ 12.10 hrs Average Depth at Peak Storage= 0.55', Surface Width= 4.25' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.10 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 230.0' Slope= 0.0130 '/' Inlet Invert= 255.00', Outlet Invert= 252.00'

Summary for Reach 3R: Swale

[62] Hint: Exceeded Reach 2R OUTLET depth by 0.13' @ 12.12 hrs

Inflow Area =52,857 sf, 54.28% Impervious, Inflow Depth > 5.21" for 50-yr eventInflow =7.16 cfs @12.10 hrs, Volume=22,947 cfOutflow =7.11 cfs @12.11 hrs, Volume=22,933 cf, Atten= 1%, Lag= 0.6 minRouted to Pond 4R : Pipe to Infiltration PondPondPond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 3.24 fps, Min. Travel Time= 0.8 min Avg. Velocity = 1.06 fps, Avg. Travel Time= 2.3 min

Peak Storage= 327 cf @ 12.11 hrs Average Depth at Peak Storage= 0.68', Surface Width= 4.92' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.66 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 149.0' Slope= 0.0134 '/' Inlet Invert= 252.00', Outlet Invert= 250.00'

Summary for Reach 5R: Swale

10,041 sf, 64.56% Impervious, Inflow Depth > 5.66" for 50-yr event Inflow Area = Inflow = 1.44 cfs @ 12.09 hrs. Volume= 4.737 cf 1.43 cfs @ 12.10 hrs, Volume= Outflow = 4,733 cf, Atten= 1%, Lag= 0.6 min Routed to Reach 6R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.53 fps, Min. Travel Time= 0.9 min Avg. Velocity = 0.73 fps, Avg. Travel Time= 3.2 min Peak Storage= 78 cf @ 12.10 hrs Average Depth at Peak Storage= 0.26', Surface Width= 2.81' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 52.35 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 139.0' Slope= 0.0234 '/' Inlet Invert= 257.00', Outlet Invert= 253.75'

Summary for Reach 6R: Swale

[62] Hint: Exceeded Reach 5R OUTLET depth by 0.33' @ 12.11 hrs

 Inflow Area =
 33,680 sf, 57.70% Impervious, Inflow Depth > 5.39" for 50-yr event

 Inflow =
 4.71 cfs @
 12.09 hrs, Volume=
 15,127 cf

 Outflow =
 4.65 cfs @
 12.10 hrs, Volume=
 15,115 cf, Atten= 1%, Lag= 0.8 min

 Routed to Reach 7R : Swale
 5.39"
 5.39"
 5.39"

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.64 fps, Min. Travel Time= 1.1 min Avg. Velocity = 0.80 fps, Avg. Travel Time= 3.5 min

Peak Storage= 294 cf @ 12.10 hrs Average Depth at Peak Storage= 0.59', Surface Width= 4.45' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 35.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 167.0' Slope= 0.0105 '/' Inlet Invert= 253.75', Outlet Invert= 252.00'

Summary for Reach 7R: Swale

[62] Hint: Exceeded Reach 6R OUTLET depth by 0.28' @ 12.15 hrs

Inflow Area = 56,601 sf, 53.74% Impervious, Inflow Depth > 5.22" for 50-yr event Inflow = 7.65 cfs @ 12.10 hrs, Volume= 24,620 cf Outflow = 7.40 cfs @ 12.12 hrs, Volume= 24,587 cf, Atten= 3%, Lag= 1.3 min Routed to Reach 8R : Box Culvert to Infiltration Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.40 fps, Min. Travel Time= 1.8 min Avg. Velocity = 0.76 fps, Avg. Travel Time= 5.7 min

Peak Storage= 799 cf @ 12.12 hrs Average Depth at Peak Storage= 0.85' , Surface Width= 5.75' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 26.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 259.0' Slope= 0.0058 '/' Inlet Invert= 252.00', Outlet Invert= 250.50'

Summary for Reach 8R: Box Culvert to Infiltration Pond

[52] Hint: Inlet/Outlet conditions not evaluated [61] Hint: Exceeded Reach 7R outlet invert by 0.29' @ 12.12 hrs

 Inflow Area =
 56,601 sf, 53.74% Impervious, Inflow Depth > 5.21" for 50-yr event

 Inflow =
 7.40 cfs @
 12.12 hrs, Volume=
 24,587 cf

 Outflow =
 7.40 cfs @
 12.12 hrs, Volume=
 24,586 cf, Atten= 0%, Lag= 0.0 min

 Routed to Pond P1 : Infiltration Pond
 Pond
 Pond
 Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 1.87 fps, Min. Travel Time= 0.1 min Avg. Velocity = 0.45 fps, Avg. Travel Time= 0.2 min

Peak Storage= 24 cf @ 12.12 hrs Average Depth at Peak Storage= 0.79', Surface Width= 5.00' Bank-Full Depth= 1.50' Flow Area= 7.5 sf, Capacity= 13.65 cfs

60.0" W x 18.0" H Box Pipe n= 0.040 Earth, cobble bottom, clean sides Length= 6.0' Slope= 0.0050 '/' Inlet Invert= 250.00', Outlet Invert= 249.97'



Summary for Pond 4R: Pipe to Infiltration Pond

[62] Hint: Exceeded Reach 3R OUTLET depth by 0.47' @ 12.12 hrs

Inflow Are	a =	52,857 sf, 54.28% Impervious, Inflow Depth >	5.21" for 50-yr event				
Inflow	=	7.11 cfs @ 12.11 hrs, Volume= 22,933 cf	-				
Outflow	=	7.07 cfs @ 12.11 hrs, Volume= 22,930 cf	, Atten= 1%, Lag= 0.5 min				
Primary	=	7.07 cfs @ 12.11 hrs, Volume= 22,930 cf	-				
Routed to Pond P1 : Infiltration Pond							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 251.15' @ 12.11 hrs Surf.Area= 281 sf Storage= 136 cf

Plug-Flow detention time= 0.4 min calculated for 22,930 cf (100% of inflow) Center-of-Mass det. time= 0.3 min (803.9 - 803.6)

<u>Volume</u> #1	<u>Inv</u> 250.0		0 0	escription	iamatia) istad halaw (Pasala)	
#1	200.0	0		Slage Dala (Ph	ismatic)Listed below (Recalc)	
Elevatio (feet	•••	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
250.0	0	22	0	0		
251.0	0	181	102	102		
252.0	0	855	518	620		
Device	Routing	Invert	Outlet Devices			
#1	Primary	Imary 250.00' 15.0'' Round Culvert X 2.00 L= 55.0' Ke= 0.500 Inlet / Outlet Invert= 250.00' / 249.72' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf				
Primary OutFlow Max=7.06 cfs @ 12.11 hrs. HW=251.15' TW=250.35' (Dynamic Tailwater)						

Primary OutFlow Max=7.06 cfs @ 12.11 hrs HW=251.15' TW=250.35' (Dynamic Tailwater) -1=Culvert (Barrel Controls 7.06 cfs @ 3.92 fps)

Summary for Pond P1: Infiltration Pond

[61] Hint: Exceeded Reach 8R outlet invert by 0.51' @ 12.19 hrs

Inflow Area =	170,916 sf	, 44.06% Impervious,	Inflow Depth > 4.76" for 50-yr event				
Inflow =	20.53 cfs @	12.11 hrs, Volume=	67,847 cf				
Outflow =	14.76 cfs @	12.19 hrs, Volume=	67,835 cf,Atten= 28%,Lag= 5.0 min				
Discarded =	2.86 cfs @	12.19 hrs, Volume=	50,532 cf				
Primary =	11.90 cfs @	12.19 hrs, Volume=	17,303 cf				
Routed to Link AP1 : Wetlands							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 250.48' @ 12.19 hrs Surf.Area= 10,049 sf Storage= 11,191 cf

Plug-Flow detention time= 15.6 min calculated for 67,807 cf (100% of inflow) Center-of-Mass det. time= 15.5 min (822.0 - 806.5)

Volume	Inver	rt Avail.Sto	rage Sto	rage Description		
#1	249.25	5' 24,64	49 cf Cu	stom Stage Data (P	Prismatic)Listed below	v (Recalc)
Elevatic (fee 249.2 250.0 251.0 251.7	25 00 00	Surf.Area (sq-ft) 8,187 9,297 10,866 12,024	Inc.Sto (cubic-fee 6,55 10,08 8,07	(cubic-feet) 0 0 57 6,557 32 16,638		
Device	Routing	Invert	Outlet De	evices		
#1	Primary	249.90'	10.0' lon	g x 22.0' breadth E	Broad-Crested Recta	Ingular Weir
#2	Discarded	249.25'	Coef. (Ei 8.270 in /	ngĺish) 2.68 2.70 2 / hr Exfiltration over	0.80 1.00 1.20 1.40 2.70 2.64 2.63 2.64 • Surface area Elevation = 247.00'	

Discarded OutFlow Max=2.86 cfs @ 12.19 hrs HW=250.48' (Free Discharge) **2=Exfiltration** (Controls 2.86 cfs)

Primary OutFlow Max=11.89 cfs @ 12.19 hrs HW=250.48' TW=0.00' (Dynamic Tailwater) **1=Broad-Crested Rectangular Weir** (Weir Controls 11.89 cfs @ 2.05 fps)

Summary for Link AP1: Wetlands

Inflow Are	a =	292,946 sf, 29.39% Impervious, Inflow Depth > 2	04" for 50-yr event
Inflow	=	17.91 cfs @ 12.22 hrs, Volume= 49,874 cf	
Primary	=	17.91 cfs @ 12.22 hrs, Volume= 49,874 cf,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

PostDevelopment Prepared by Howard Stein Hudson <u>HydroCAD® 10.10-6a_s/n M05607_© 2020 Hyd</u>	<i>Type III 24-hr 100-yr Rainfall=8.86"</i> Printed 4/8/2022 droCAD Software Solutions LLC Page 116
Runoff by SCS TR	4.00 hrs, dt=0.01 hrs, 2401 points x 3 -20 method, UH=SCS, Weighted-CN method - Pond routing by Dyn-Stor-Ind method
Subcatchment 201: to Under Sidewalk Un	it Runoff Area=17,748 sf 80.43% Impervious Runoff Depth>7.77" Tc=6.0 min CN=91 Runoff=3.42 cfs 11,489 cf
Subcatchment 202: To RGT 1	Runoff Area=17,947 sf 48.49% Impervious Runoff Depth>6.31" Tc=6.0 min CN=79 Runoff=3.00 cfs 9,433 cf
Subcatchment 203: To RGT 2	Runoff Area=17,507 sf 63.88% Impervious Runoff Depth>7.04" Tc=6.0 min CN=85 Runoff=3.18 cfs 10,269 cf
Subcatchment 204: To RGT 3	Runoff Area=9,147 sf 62.73% Impervious Runoff Depth>6.92" Tc=6.0 min CN=84 Runoff=1.64 cfs 5,273 cf
Subcatchment 205: To RGT 4	Runoff Area=6,183 sf 86.80% Impervious Runoff Depth>8.01" Tc=6.0 min CN=93 Runoff=1.21 cfs 4,127 cf
Subcatchment 206: To RGT 5	Runoff Area=18,720 sf 60.91% Impervious Runoff Depth>6.92" Tc=6.0 min CN=84 Runoff=3.36 cfs 10,791 cf
Subcatchment 207: To RGT 6	Runoff Area=19,445 sf 50.44% Impervious Runoff Depth>6.43" Tc=6.0 min CN=80 Runoff=3.30 cfs 10,418 cf
Subcatchment 208: To Swale	Runoff Area=4,919 sf 31.45% Impervious Runoff Depth>5.57" Tc=6.0 min CN=73 Runoff=0.74 cfs 2,285 cf
Subcatchment 209: To Swale	Runoff Area=3,858 sf 28.90% Impervious Runoff Depth>5.45" Tc=6.0 min CN=72 Runoff=0.57 cfs 1,752 cf
Subcatchment 210: To Swale	Runoff Area=1,505 sf 48.11% Impervious Runoff Depth>6.31" Tc=6.0 min CN=79 Runoff=0.25 cfs 791 cf
Subcatchment211: To Swale	Runoff Area=4,032 sf 35.99% Impervious Runoff Depth>5.70" Tc=6.0 min CN=74 Runoff=0.62 cfs 1,914 cf
Subcatchment212: To Swale	Runoff Area=2,719 sf 32.88% Impervious Runoff Depth>5.57" Tc=6.0 min CN=73 Runoff=0.41 cfs 1,263 cf
Subcatchment213: Direct to Infiltration	Runoff Area=43,710 sf 4.37% Impervious Runoff Depth>4.11" Tc=6.0 min CN=61 Runoff=4.81 cfs 14,962 cf
Subcatchment214: Direct to Wetland	Runoff Area=122,030 sf 8.85% Impervious Runoff Depth>4.34" low Length=492' Tc=20.9 min CN=63 Runoff=9.43 cfs 44,098 cf
Subcatchment215: To Swale	Runoff Area=3,476 sf 33.92% Impervious Runoff Depth>5.70" Tc=6.0 min CN=74 Runoff=0.53 cfs 1,650 cf
	Avg. Flow Depth=0.26' Max Vel=3.38 fps Inflow=1.89 cfs 6,064 cf 24.0' S=0.0417 '/' Capacity=69.88 cfs Outflow=1.89 cfs 6,063 cf

PostDevelopment Prepared by Howard Steir HydroCAD® 10.10-6a s/n M05		<i>I-hr 100-yr Rainfall=8.86"</i> Printed 4/8/2022 Page 117				
Reach 2R: Swale	Avg. Flow Depth=0.61' Max Vel=3.00 f n=0.030 L=230.0' S=0.0130 '/' Capacity=39.10 cfs					
Reach 3R: Swale	Avg. Flow Depth=0.76' Max Vel=3.43 n=0.030 L=149.0' S=0.0134 '/' Capacity=39.66 cfs	•				
Reach 5R: Swale	Avg. Flow Depth=0.29' Max Vel=2.69 n=0.030 L=139.0' S=0.0234 '/' Capacity=52.35 c	•				
Reach 6R: Swale	Avg. Flow Depth=0.66' Max Vel=2.80 f n=0.030 L=167.0' S=0.0105 '/' Capacity=35.05 cfs	•				
Reach 7R: Swale	Avg. Flow Depth=0.95' Max Vel=2.54 1 n=0.030 L=259.0' S=0.0058 '/' Capacity=26.05 cfs	•				
Reach 8R: Box Culvert to Infiltration 60.0" x 18.0" Box Pipe Avg. Flow Depth=0.92' Max Vel=2.02 fps Inflow=9.28 cfs 30,967 cf L=6.0' S=0.0050 '/' Capacity=13.65 cfs Outflow=9.28 cfs 30,966 cf						
Pond 4R: Pipe to Infiltratio 15.	n Pond Peak Elev=251.35' Storage=207 0" Round Culvert x 2.00 n=0.012 L=55.0' S=0.0051 '/					
Pond P1: Infiltration Pond Discar	Peak Elev=250.65' Storage=12,957 ded=3.06 cfs 59,642 cf Primary=17.33 cfs 26,664 cf					
Link AP1: Wetlands		Inflow=25.21 cfs 70,762 cf Primary=25.21 cfs 70,762 cf				

Total Runoff Area = 292,946 sf Runoff Volume = 130,513 cf Average Runoff Depth = 5.35" 70.61% Pervious = 206,849 sf 29.39% Impervious = 86,097 sf

Summary for Subcatchment 201: to Under Sidewalk Unit

Page 118

Runoff 3.42 cfs @ 12.08 hrs, Volume= 11,489 cf, Depth> 7.77" = Routed to Pond P1 : Infiltration Pond

A	rea (sf)	CN	Description					
	13,365	98	Paved parking, HSG B					
	3,474	61	>75% Grass cover, Good, HSG B					
	909	98	Roofs, HSC	βB				
	17,748	91	91 Weighted Average					
	3,474		19.57% Pervious Area					
	14,274		80.43% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment 202: To RGT 1

Runoff = 3.00 cfs @ 12.09 hrs, Volume= 9,433 cf, Depth> 6.31" Routed to Reach 3R : Swale

Ar	rea (sf)	CN	Description					
	4,676	98	Paved parking, HSG B					
	9,245	61	>75% Gras	s cover, Go	od, HSG B			
	4,026	98	Roofs, HSG B					
	17,947	79	79 Weighted Average					
	9,245	:	51.51% Pervious Area					
	8,702	4	48.49% Impervious Area					
Та	Longth	Slope	Volocity	Conocity	Description			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	(ieer)	(11/11)		(015)				
6.0					Direct Entry,			

Summary for Subcatchment 203: To RGT 2

Runoff = 3.18 cfs @ 12.08 hrs, Volume= 10,269 cf, Depth> 7.04" Routed to Reach 2R : Swale

Α	rea (sf)	CN	Description						
	7,022	98	Paved park	ing, HSG B	3				
	6,324	61	>75% Gras	s cover, Go	bod, HSG B				
	4,161	98	Roofs, HSO	βB					
	17,507	85	Weighted A	verage					
	6,324		36.12% Pervious Area						
	11,183	(63.88% Imp	pervious Ar	ea				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment 204: To RGT 3

Runoff = 1.64 cfs @ 12.08 hrs, Volume= 5,273 cf, Depth> 6.92" Routed to Reach 1R : Swale

/	Area (sf)	CN	Description						
	3,638		Paved park						
	3,409	61	>75% Gras	s cover, Go	ood, HSG B				
	2,100	98	Roofs, HSC	Э В					
	9,147	84	Weighted A	verage					
	3,409		37.27% Pervious Area						
	5,738		62.73% Imp	pervious Ar	ea				
Тс	5	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft	ft) (ft/sec) (cfs)						
6.0					Direct Entry,				
(min)	(feet)	Slope (ft/ft)	,		•				

Summary for Subcatchment 205: To RGT 4

Runoff = 1.21 cfs @ 12.08 hrs, Volume= 4,127 cf, Depth> 8.01" Routed to Reach 5R : Swale

A	rea (sf)	CN	Description						
	4,135	98	Paved park	ing, HSG B	3				
	816	61	>75% Gras	s cover, Go	ood, HSG B				
	1,232	98	Roofs, HSC	βB					
	6,183	93	Weighted A	verage					
	816		13.20% Pervious Area						
	5,367		86.80% Imp	pervious Ar	rea				
_									
Tc	Length	Slope	,	Capacity					
<u>(min)</u>	(feet)	(ft/ft)	ft) (ft/sec) (cfs)						
6.0					Direct Entry,				

Summary for Subcatchment 206: To RGT 5

Runoff = 3.36 cfs @ 12.08 hrs, Volume= 10,791 cf, Depth> 6.92" Routed to Reach 6R : Swale

A	rea (sf)	CN	Description						
	7,564	98	Paved park	ing, HSG B	В				
	7,317	61	>75% Gras	s cover, Go	ood, HSG B				
	3,839	98	Roofs, HSO	БВ					
	18,720	84	Weighted A	verage					
	7,317	;	39.09% Pei	vious Area	a				
	11,403		60.91% Imp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	1				
6.0					Direct Entry,				

Summary for Subcatchment 207: To RGT 6

Runoff = 3.30 cfs @ 12.09 hrs, Volume= 10,418 cf, Depth> 6.43" Routed to Reach 7R : Swale

A	rea (sf)	CN I	Description						
	6,428	98	Paved park	ing, HSG B	3				
	9,636	61 3	>75% Gras	s cover, Go	ood, HSG B				
	3,381	98	Roofs, HSG	6 B					
	19,445	80	Neighted A	verage					
	9,636	4	49.56% Pervious Area						
	9,809	:	50.44% Imp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment 208: To Swale

Runoff = 0.74 cfs @ 12.09 hrs, Volume= 2,285 cf, Depth> 5.57" Routed to Reach 6R : Swale

A	rea (sf)	CN I	Description						
	1,547	98	Paved park	ing, HSG B	В				
	3,372	61 :	>75% Gras	s cover, Go	ood, HSG B				
	4,919	73	Neighted A	verage					
	3,372	(68.55% Pervious Area						
	1,547	:	31.45% Imp	pervious Are	rea				
_				. .					
Tc	Length	Slope	,	Capacity	•				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				
					-				

Summary for Subcatchment 209: To Swale

Runoff = 0.57 cfs @ 12.09 hrs, Volume= 1,752 cf, Depth> 5.45" Routed to Reach 5R : Swale

ea (sf)	CN [Description					
1,115	98 F	Paved park	ing, HSG B	3			
2,743	61 >	-75% Ġras	s cover, Go	bod, HSG B			
3,858	72 \	Veighted A	verage				
2,743	7	71.10% Pervious Area					
1,115	2	28.90% Imp	ervious Ar	ea			
Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
				Direct Entry,			
	1,115 2,743 3,858 2,743 1,115 _ength	1,115 98 F 2,743 61 ≥ 3,858 72 V 2,743 7 1,115 2 ∟ength Slope	1,115 98 Paved park 2,743 61 >75% Grass 3,858 72 Weighted A 2,743 71.10% Per 1,115 28.90% Imp Length Slope Velocity	1,11598Paved parking, HSG E2,74361>75% Grass cover, Go3,85872Weighted Average2,74371.10% Pervious Area1,11528.90% Impervious ArLengthSlopeVelocityCapacity			

Summary for Subcatchment 210: To Swale

Runoff = 0.25 cfs @ 12.09 hrs, Volume= 791 cf, Depth> 6.31" Routed to Reach 1R : Swale

sf) CN	Description	Description						
24 98	Paved park	ing, HSG B	3					
81 61	>75% Ġras	s cover, Go	bod, HSG B					
05 79	Weighted A	verage						
81	51.89% Per	vious Area	l					
24	48.11% Imp	pervious Ar	ea					
0		Capacity (cfs)	Description					
			Direct Entry,					
	24 98 81 61 05 79 81 24 ngth Slo	24 98 Paved park 81 61 >75% Gras 05 79 Weighted A 81 51.89% Per 24 48.11% Imp ngth Slope Velocity	2498Paved parking, HSG E8161>75% Grass cover, Go0579Weighted Average8151.89% Pervious Area2448.11% Impervious ArngthSlopeVelocity					

Summary for Subcatchment 211: To Swale

Runoff = 0.62 cfs @ 12.09 hrs, Volume= 1,914 cf, Depth> 5.70" Routed to Reach 2R : Swale

rea (sf)	CN [Description					
1,451	98 F	Paved park	ing, HSG B	}			
2,581	61 >	-75% Gras	s cover, Go	bod, HSG B			
4,032	74 \	Veighted A	verage				
2,581	6	64.01% Per	vious Area				
1,451	3	85.99% Imp	pervious Are	ea			
Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
				Direct Entry,			
	1,451 2,581 4,032 2,581 1,451 Length	1,451 98 F 2,581 61 > 4,032 74 V 2,581 6 1,451 3 Length Slope	1,451 98 Paved park 2,581 61 >75% Grass 4,032 74 Weighted A 2,581 64.01% Per 1,451 35.99% Imp Length Slope Velocity	1,45198Paved parking, HSG E2,58161>75% Grass cover, Go4,03274Weighted Average2,58164.01% Pervious Area1,45135.99% Impervious ArLengthSlopeVelocityCapacity			

Summary for Subcatchment 212: To Swale

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 1,263 cf, Depth> 5.57" Routed to Reach 3R : Swale

sf) CN	Description	Description						
94 98	Paved park	ing, HSG B	3					
25 61	>75% Gras	s cover, Go	ood, HSG B					
19 73	Weighted A	verage						
25	67.12% Per	vious Area	a					
94	32.88% Imp	pervious Are	rea					
0		Capacity (cfs)	Description					
			Direct Entry,					
	94 98 25 61 19 73 25 94 ngth Slo	94 98 Paved park 25 61 >75% Gras 19 73 Weighted A 25 67.12% Per 94 32.88% Imp 19 Slope	98 Paved parking, HSG B 25 61 >75% Grass cover, G 719 73 Weighted Average 25 67.12% Pervious Area 32.88% Impervious Area 94 32.88% Impervious Area					

Summary for Subcatchment 213: Direct to Infiltration Pond

Runoff = 4.81 cfs @ 12.09 hrs, Volume= 14,962 cf, Depth> 4.11" Routed to Pond P1 : Infiltration Pond

A	rea (sf)	CN I	Description						
	1,912	98	Paved park	ing, HSG B					
	39,571	61 :	>75% Gras	s cover, Go	ood, HSG B				
	2,227	39 :	>75% Gras	s cover, Go	ood, HSG A				
	43,710	61	Neighted A	verage					
	41,798	9	95.63% Pervious Area						
	1,912	4	4.37% Impe	ervious Are	а				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment 214: Direct to Wetland

Runoff = 9.43 cfs @ 12.29 hrs, Volume= 44,098 cf, Depth> 4.34" Routed to Link AP1 : Wetlands

A	rea (sf)	CN [Description							
	7,280	98 F	Paved parking, HSG B							
	96,435	61 >	•75% Gras	s cover, Go	ood, HSG B					
	9,806	55 V	Voods, Go	od, HSG B						
	3,519		Roofs, HSG							
	3,940			,	ood, HSG A					
	1,050	30 V	Voods, Go	od, HSG A						
	22,030		Veighted A							
1	11,231	-	-	vious Area						
	10,799	8	8.85% Impe	ervious Are	а					
То	Longth	Slope	Volocity	Capacity	Description					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
				(013)	Shoot Elow					
9.5	50	0.1600	0.09		Sheet Flow, Woods: Dense underbrush n= 0.800 P2= 3.10"					
0.4	41	0.1000	1.58							
0.4	41	0.1000	1.50		Shallow Concentrated Flow, Woodland, Ky= 5.0 fps					
11.0	401	0.0075	Woodland Kv= 5.0 fps							
11.0	+01	0.0075	0.61 Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps							
20.9	492	Total								
20.9	432	iotai								

Summary for Subcatchment 215: To Swale

Runoff = 0.53 cfs @ 12.09 hrs, Volume= 1,650 cf, Depth> 5.70" Routed to Reach 7R : Swale

a (sf)	CN E	Description				
,179	98 F	aved parki	ing, HSG B	3		
,297	61 >	>75% Grass cover, Good, HSG B				
,476	74 V	Veighted A	verage			
,297	6	6.08% Per	vious Area	l		
,179	3	33.92% Impervious Area				
ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
				Direct Entry,		
	,179 ,297 ,476 ,297 ,179 ength	,179 98 F ,297 61 > ,476 74 V ,297 6 ,179 3 ength Slope	,179 98 Paved parki ,297 61 >75% Grass ,476 74 Weighted A ,297 66.08% Per ,179 33.92% Imp ength Slope Velocity	,17998Paved parking, HSG E,29761>75% Grass cover, Go,47674Weighted Average,29766.08% Pervious Area,17933.92% Impervious ArengthSlopeVelocity		

Summary for Reach 1R: Swale

10,652 sf, 60.66% Impervious, Inflow Depth > 6.83" for 100-yr event Inflow Area = Inflow = 1.89 cfs @ 12.09 hrs, Volume= 6.064 cf 1.89 cfs @ 12.09 hrs, Volume= Outflow = 6,063 cf, Atten= 0%, Lag= 0.1 min Routed to Reach 2R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 3.38 fps, Min. Travel Time= 0.1 min Avg. Velocity = 1.00 fps, Avg. Travel Time= 0.4 min Peak Storage= 13 cf @ 12.09 hrs Average Depth at Peak Storage= 0.26', Surface Width= 2.80' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 69.88 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 24.0' Slope= 0.0417 '/' Inlet Invert= 256.00', Outlet Invert= 255.00'

Summary for Reach 2R: Swale

[62] Hint: Exceeded Reach 1R OUTLET depth by 0.36' @ 12.11 hrs

 Inflow Area =
 32,191 sf, 59.32% Impervious, Inflow Depth > 6.80" for 100-yr event

 Inflow =
 5.69 cfs @
 12.09 hrs, Volume=
 18,246 cf

 Outflow =
 5.58 cfs @
 12.10 hrs, Volume=
 18,228 cf, Atten= 2%, Lag= 0.9 min

 Routed to Reach 3R : Swale
 5.58 cfs
 12.10 hrs, Volume=
 18,228 cf, Atten= 2%, Lag= 0.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 3.00 fps, Min. Travel Time= 1.3 min Avg. Velocity = 0.97 fps, Avg. Travel Time= 4.0 min

Peak Storage= 427 cf @ 12.10 hrs Average Depth at Peak Storage= 0.61', Surface Width= 4.56' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.10 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 230.0' Slope= 0.0130 '/' Inlet Invert= 255.00', Outlet Invert= 252.00'

Summary for Reach 3R: Swale

[62] Hint: Exceeded Reach 2R OUTLET depth by 0.15' @ 12.12 hrs

 Inflow Area =
 52,857 sf, 54.28% Impervious, Inflow Depth > 6.57" for 100-yr event

 Inflow =
 8.95 cfs @
 12.10 hrs, Volume=
 28,924 cf

 Outflow =
 8.90 cfs @
 12.10 hrs, Volume=
 28,908 cf, Atten= 1%, Lag= 0.5 min

 Routed to Pond 4R : Pipe to Infiltration Pond
 Pipe to Infiltration Pond
 Pipe to Infiltration Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 3.43 fps, Min. Travel Time= 0.7 min Avg. Velocity = 1.13 fps, Avg. Travel Time= 2.2 min

Peak Storage= 386 cf @ 12.10 hrs Average Depth at Peak Storage= 0.76', Surface Width= 5.31' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 39.66 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 149.0' Slope= 0.0134 '/' Inlet Invert= 252.00', Outlet Invert= 250.00'

Summary for Reach 5R: Swale

10,041 sf, 64.56% Impervious, Inflow Depth > 7.03" for 100-yr event Inflow Area = Inflow = 1.77 cfs @ 12.08 hrs, Volume= 5.880 cf 1.76 cfs @ 12.10 hrs, Volume= Outflow = 5,875 cf, Atten= 1%, Lag= 0.6 min Routed to Reach 6R : Swale Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.69 fps, Min. Travel Time= 0.9 min Avg. Velocity = 0.78 fps, Avg. Travel Time= 3.0 min Peak Storage= 91 cf @ 12.10 hrs Average Depth at Peak Storage= 0.29', Surface Width= 2.96' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 52.35 cfs 1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 139.0' Slope= 0.0234 '/' Inlet Invert= 257.00', Outlet Invert= 253.75'

Innet Invert- 257.00°, Oddet Invert- 255.75

Summary for Reach 6R: Swale

[62] Hint: Exceeded Reach 5R OUTLET depth by 0.37' @ 12.11 hrs

 Inflow Area =
 33,680 sf, 57.70% Impervious, Inflow Depth > 6.75" for 100-yr event

 Inflow =
 5.85 cfs @
 12.09 hrs, Volume=
 18,951 cf

 Outflow =
 5.78 cfs @
 12.10 hrs, Volume=
 18,937 cf, Atten= 1%, Lag= 0.7 min

 Routed to Reach 7R : Swale
 12.10 hrs, Volume=
 18,937 cf, Atten= 1%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.80 fps, Min. Travel Time= 1.0 min Avg. Velocity = 0.86 fps, Avg. Travel Time= 3.2 min

Peak Storage= 345 cf @ 12.10 hrs Average Depth at Peak Storage= 0.66' , Surface Width= 4.79' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 35.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 167.0' Slope= 0.0105 '/' Inlet Invert= 253.75', Outlet Invert= 252.00'

Summary for Reach 7R: Swale

[62] Hint: Exceeded Reach 6R OUTLET depth by 0.30' @ 12.15 hrs

Inflow Area = 56,601 sf, 53.74% Impervious, Inflow Depth > 6.57" for 100-yr event Inflow = 9.57 cfs @ 12.09 hrs, Volume= 31,004 cf Outflow = 9.28 cfs @ 12.12 hrs, Volume= 30,967 cf, Atten= 3%, Lag= 1.2 min Routed to Reach 8R : Box Culvert to Infiltration Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.54 fps, Min. Travel Time= 1.7 min Avg. Velocity = 0.81 fps, Avg. Travel Time= 5.3 min

Peak Storage= 946 cf @ 12.12 hrs Average Depth at Peak Storage= 0.95' , Surface Width= 6.23' Bank-Full Depth= 1.50' Flow Area= 7.9 sf, Capacity= 26.05 cfs

1.50' x 1.50' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 2.5 '/' Top Width= 9.00' Length= 259.0' Slope= 0.0058 '/' Inlet Invert= 252.00', Outlet Invert= 250.50'

Summary for Reach 8R: Box Culvert to Infiltration Pond

[52] Hint: Inlet/Outlet conditions not evaluated [61] Hint: Exceeded Reach 7R outlet invert by 0.42' @ 12.12 hrs

 Inflow Area =
 56,601 sf, 53.74% Impervious, Inflow Depth > 6.57" for 100-yr event

 Inflow =
 9.28 cfs @
 12.12 hrs, Volume=
 30,967 cf

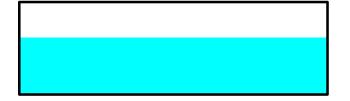
 Outflow =
 9.28 cfs @
 12.12 hrs, Volume=
 30,966 cf, Atten= 0%, Lag= 0.0 min

 Routed to Pond P1 : Infiltration Pond
 Pond
 Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Max. Velocity= 2.02 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.49 fps, Avg. Travel Time= 0.2 min

Peak Storage= 28 cf @ 12.12 hrs Average Depth at Peak Storage= 0.92', Surface Width= 5.00' Bank-Full Depth= 1.50' Flow Area= 7.5 sf, Capacity= 13.65 cfs

60.0" W x 18.0" H Box Pipe n= 0.040 Earth, cobble bottom, clean sides Length= 6.0' Slope= 0.0050 '/' Inlet Invert= 250.00', Outlet Invert= 249.97'



Summary for Pond 4R: Pipe to Infiltration Pond

[62] Hint: Exceeded Reach 3R OUTLET depth by 0.60' @ 12.12 hrs

Inflow Area	=	52,857 sf,	, 54.28% Impervious,	Inflow Depth > 6.56" for 100-yr event	
Inflow	=	8.90 cfs @	12.10 hrs, Volume=	28,908 cf	
Outflow	=	8.79 cfs @	12.12 hrs, Volume=	28,905 cf, Atten= 1%, Lag= 0.8 min	
Primary	=	8.79 cfs @	12.12 hrs, Volume=	28,905 cf	
Routed to Pond P1 : Infiltration Pond					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 251.35' @ 12.12 hrs Surf.Area= 418 sf Storage= 207 cf

Plug-Flow detention time= 0.4 min calculated for 28,905 cf (100% of inflow) Center-of-Mass det. time= 0.3 min (797.5 - 797.2)

Volume	Inve	ert Avail.Sto	orage S	Storage D	Description	
#1	250.0	00' 6	20 cf C	Sustom S	Stage Data (P	rismatic)Listed below (Recalc)
Elevation (feet)		Surf.Area (sq-ft)	Inc.S (cubic-f		Cum.Store (cubic-feet)	
250.00		22		0	0	
251.00		181		102	102	
252.00		855		518	620	
Device R	outing	Invert	Outlet	Devices		
#1 Primary 250.00' 15.0" Round Culvert X 2.00 L= 55.0' Ke= 0.500 Inlet / Outlet Invert= 250.00' / 249.72' S= 0.0051 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf						
Primary OutFlow Max=8 78 cfs @ 12 12 hrs HW=251 35' TW=250 57' (Dynamic Tailwater)						

Primary OutFlow Max=8.78 cfs @ 12.12 hrs HW=251.35' TW=250.57' (Dynamic Tailwater) -1=Culvert (Barrel Controls 8.78 cfs @ 4.12 fps)

Summary for Pond P1: Infiltration Pond

[61] Hint: Exceeded Reach 8R outlet invert by 0.68' @ 12.17 hrs

Inflow Area =	170,916 sf, 44.	.06% Impervious,	Inflow Depth > 6.06"	for 100-yr event	
Inflow =	25.97 cfs @ 12.1	11 hrs, Volume=	86,321 cf	-	
Outflow =	20.38 cfs @ 12.1	17 hrs, Volume=	86,306 cf, Atter	n= 22%, Lag= 4.1 min	
Discarded =	3.06 cfs @ 12.1	17 hrs, Volume=	59,642 cf	-	
Primary =	17.33 cfs @ 12.1	17 hrs, Volume=	26,664 cf		
Routed to Link AP1 : Wetlands					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 250.65' @ 12.17 hrs Surf.Area= 10,321 sf Storage= 12,957 cf

Plug-Flow detention time= 15.5 min calculated for 86,271 cf (100% of inflow) Center-of-Mass det. time= 15.4 min (816.0 - 800.6)

Volume	Invei	rt Avail.Sto	rage Storag	ge Description		
#1	249.25	5' 24,64	49 cf Custo	om Stage Data (P	rismatic)Listed below	w (Recalc)
Elevatio (fee 249.2 250.0 251.0 251.7	t) 25 00 00	Surf.Area (sq-ft) 8,187 9,297 10,866 12,024	Inc.Store (cubic-feet) 0 6,557 10,082 8,011	Cum.Store (cubic-feet) 0 6,557 16,638 24,649		
Device	Routing	Invert	Outlet Devi	ces		
#1	Primary	249.90'	10.0' long	x 22.0' breadth B	road-Crested Recta	angular Weir
#2	Discardec	l 249.25'	Coef. (Engl 8.270 in/hr	ish) 2.68 2.70 2. Exfiltration over	0.80 1.00 1.20 1.4 70 2.64 2.63 2.64 Surface area Elevation = 247.00'	

Discarded OutFlow Max=3.05 cfs @ 12.17 hrs HW=250.65' (Free Discharge) **2=Exfiltration** (Controls 3.05 cfs)

Primary OutFlow Max=17.31 cfs @ 12.17 hrs HW=250.65' TW=0.00' (Dynamic Tailwater) **1=Broad-Crested Rectangular Weir** (Weir Controls 17.31 cfs @ 2.30 fps)

Summary for Link AP1: Wetlands

Inflow Are	a =	292,946 sf,	, 29.39% Impervious,	Inflow Depth >	2.90"	for 100-yr event
Inflow	=	25.21 cfs @	12.20 hrs, Volume=	70,762 c	f	
Primary	=	25.21 cfs @	12.20 hrs, Volume=	70,762 c	f, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Stage-Area-Storage for Pond 4R: Pipe to Infiltration Pond

	.			- <i>i</i>	O (
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	<u>(sq-ft)</u>	(cubic-feet)
250.00 250.01	22 24	0 0	250.53 250.54	106 108	34 35
250.01	24	0	250.54	109	36
250.02	23	1	250.56	103	37
250.04	28	1	250.57	113	38
250.05	30	1	250.58	114	40
250.06	32	2	250.59	116	41
250.07	33	2	250.60	117	42
250.08	35	2	250.61	119	43
250.09	36	3	250.62	121	44
250.10	38	3	250.63	122	45
250.11	39	3	250.64	124	47
250.12	41	4	250.65	125	48
250.13	43	4	250.66	127	49
250.14	44	5	250.67	129	50
250.15	46	5	250.68	130	52
250.16 250.17	47 49	6 6	250.69 250.70	132 133	53 54
250.17	49 51	7	250.70	135	56
250.10	52	7	250.71	136	57
250.20	54	8	250.72	138	58
250.21	55	8	250.74	140	60
250.22	57	9	250.75	141	61
250.23	59	9	250.76	143	63
250.24	60	10	250.77	144	64
250.25	62	10	250.78	146	66
250.26	63	11	250.79	148	67
250.27	65	12	250.80	149	68
250.28	67	12	250.81	151	70
250.29	68	13	250.82	152	71
250.30	70	14	250.83	154	73
250.31 250.32	71 73	14 15	250.84 250.85	156 157	75 76
250.32	73	16	250.85	159	78
250.34	76	10	250.87	160	70
250.35	78	17	250.88	162	81
250.36	79	18	250.89	164	83
250.37	81	19	250.90	165	84
250.38	82	20	250.91	167	86
250.39	84	21	250.92	168	88
250.40	86	22	250.93	170	89
250.41	87	22	250.94	171	91
250.42	89	23	250.95	173	93
250.43	90	24	250.96	175	94
250.44 250.45	92 94	25 26	250.97 250.98	176 178	96 98
250.45	94 95	20 27	250.98	178	100
250.40	97	28	251.00	181	100
250.48	98	20	251.00	188	102
250.49	100	30	251.02	194	105
250.50	102	31	251.03	201	107
250.51	103	32	251.04	208	109
250.52	105	33	251.05	215	111
			I		

Stage-Area-Storage for Pond 4R: Pipe to Infiltration Pond (continued)

ElevationSurfaceStorage (cubic-feet)ElevationSurfaceStorage (cubic-feet) 251.06 221 114 251.59 579 326 251.07 228 116 251.60 585 331 251.08 235 118 251.61 592 337 251.09 242 121 251.62 599 343 251.11 248 123 251.63 606 349 251.11 2262 128 251.66 612 355 251.12 269 131 251.66 626 368 251.14 275 133 251.67 633 374 251.15 282 136 251.68 639 380 251.16 289 139 251.69 646 387 251.17 296 142 251.70 653 393 251.18 302 145 251.71 666 407 251.20 316 151 251.75 687 427 251.21 323 154 251.75 687 427 251.22 329 158 251.76 693 434 251.24 343 164 251.77 700 441 251.25 350 168 251.78 707 448 251.24 343 164 251.77 700 444 251.25 350 168 251.80 720 462 251.29 376 182						
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$	251.34	410	202	251.87	767	514
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251.54 545 298						
251.55 552 303						
251.56 558 309						
251.57 565 314						
251.58 572 320						
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Stage-Area-Storage for Pond P1: Infiltration Pond

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Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
249.25	8,187	0 82	249.78 249.79	8,971	4,547
249.26 249.27	8,202 8,217	02 164	249.79	8,986 9,001	4,637 4,727
249.27	8,231	246	249.80	9,001	4,727 4,817
249.20	8,246	329	249.82	9,010	4,907
249.30	8,261	411	249.82	9,031	4,907 4,997
249.30	8,276	494	249.83	9,040	5,088
249.32	8,291	577	249.85	9,000	5,179
249.33	8,305	660	249.86	9,090	5,269
249.34	8,320	743	249.87	9,105	5,360
249.35	8,335	826	249.88	9,119	5,452
249.36	8,350	910	249.89	9,134	5,543
249.37	8,365	993	249.90	9,149	5,634
249.38	8,379	1,077	249.91	9,164	5,726
249.39	8,394	1,161	249.92	9,179	5,817
249.40	8,409	1,245	249.93	9,193	5,909
249.41	8,424	1,329	249.94	9,208	6,001
249.42	8,439	1,413	249.95	9,223	6,093
249.43	8,453	1,498	249.96	9,238	6,186
249.44	8,468	1,582	249.97	9,253	6,278
249.45	8,483	1,667	249.98	9,267	6,371
249.46	8,498	1,752	249.99	9,282	6,464
249.47	8,513	1,837	250.00	9,297	6,557
249.48	8,527	1,922	250.01	9,313	6,650
249.49	8,542	2,008	250.02	9,328	6,743
249.50 249.51	8,557 8,572	2,093 2,179	250.03 250.04	9,344 9,360	6,836 6,930
249.51	8,587	2,179	250.04	9,300	7,023
249.52	8,601	2,204 2,350	250.05	9,391	7,023
249.54	8,616	2,436	250.00	9,407	7,211
249.55	8,631	2,523	250.08	9,423	7,305
249.56	8,646	2,609	250.09	9,438	7,400
249.57	8,661	2,696	250.10	9,454	7,494
249.58	8,675	2,782	250.11	9,470	7,589
249.59	8,690	2,869	250.12	9,485	7,683
249.60	8,705	2,956	250.13	9,501	7,778
249.61	8,720	3,043	250.14	9,517	7,873
249.62	8,735	3,130	250.15	9,532	7,969
249.63	8,749	3,218	250.16	9,548	8,064
249.64	8,764	3,305	250.17	9,564	8,160
249.65	8,779	3,393	250.18	9,579	8,255
249.66	8,794	3,481	250.19	9,595	8,351
249.67	8,809	3,569	250.20	9,611	8,447
249.68	8,823	3,657	250.21	9,626	8,543
249.69	8,838	3,746	250.22	9,642	8,640
249.70 249.71	8,853 8,868	3,834 3,923	250.23 250.24	9,658 9,674	8,736 8,833
249.71	8,883	4,011	250.24	9,689	8,930
249.72	8,897	4,100	250.25	9,705	9,027
249.74	8,912	4,189	250.20	9,721	9,124
249.75	8,927	4,279	250.28	9,736	9,221
249.76	8,942	4,368	250.29	9,752	9,319
249.77	8,957	4,457	250.30	9,768	9,416
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Stage-Area-Storage for Pond P1: Infiltration Pond (continued)

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250.339,8159,710250.8610,64615,132250.349,8309,808250.8710,66215,239250.359,8469,907250.8810,67815,345250.369,86210,005250.8910,69315,452250.379,87810,104250.9010,70915,559250.389,89310,203250.9110,72515,666250.399,90910,302250.9210,74015,774250.409,92510,401250.9310,75615,881250.419,94010,500250.9410,77215,989
250.349,8309,808250.8710,66215,239250.359,8469,907250.8810,67815,345250.369,86210,005250.8910,69315,452250.379,87810,104250.9010,70915,559250.389,89310,203250.9110,72515,666250.399,90910,302250.9210,74015,774250.409,92510,401250.9310,75615,881250.419,94010,500250.9410,77215,989
250.359,8469,907250.8810,67815,345250.369,86210,005250.8910,69315,452250.379,87810,104250.9010,70915,559250.389,89310,203250.9110,72515,666250.399,90910,302250.9210,74015,774250.409,92510,401250.9310,75615,881250.419,94010,500250.9410,77215,989
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250.379,87810,104250.9010,70915,559250.389,89310,203250.9110,72515,666250.399,90910,302250.9210,74015,774250.409,92510,401250.9310,75615,881250.419,94010,500250.9410,77215,989
250.389,89310,203250.9110,72515,666250.399,90910,302250.9210,74015,774250.409,92510,401250.9310,75615,881250.419,94010,500250.9410,77215,989
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250.409,92510,401250.9310,75615,881250.419,94010,500250.9410,77215,989
250.41 9,940 10,500 250.94 10,772 15,989
250.42 9,956 10,600 250.95 10,788 16,097
250.43 9,972 10,699 250.96 10,803 16,205
250.44 9,987 10,799 250.97 10,819 16,313
250.45 10,003 10,899 250.98 10,835 16,421
250.46 10,019 10,999 250.99 10,850 16,529 10,014 10,999 250.99 10,850 16,529
250.47 10,034 11,099 251.00 10,866 16,638
250.48 10,050 11,200 251.01 10,883 16,747
250.49 10,066 11,300 251.02 10,899 16,856
250.50 10,082 11,401 251.03 10,916 16,965
250.51 10,097 11,502 251.04 10,932 17,074
250.52 10,113 11,603 251.05 10,949 17,183 250.52 10,400 14,704 254.00 10,949 17,183
250.53 10,129 11,704 251.06 10,965 17,293
250.54 10,144 11,806 251.07 10,982 17,403
250.55 10,160 11,907 251.08 10,998 17,513 250.56 10,176 12,000 251.08 10,998 17,513
250.56 10,176 12,009 251.09 11,015 17,623
250.57 10,191 12,111 251.10 11,031 17,733 250.58 10.207 12,212 251.11 11,048 17,842
250.58 10,207 12,213 251.11 11,048 17,843 250.50 10.222 12.215 251.12 11.065 17,054
250.59 10,223 12,315 251.12 11,065 17,954 250.60 10.238 12,417 251.12 11,065 17,954
250.60 10,238 12,417 251.13 11,081 18,065 250.61 10.254 12,520 251.14 11,008 18,175
250.61 10,254 12,520 251.14 11,098 18,175 250.62 10.270 12,622 251.15 11,114 18,287
250.6210,27012,622251.1511,11418,287250.6310,28512,725251.1611,13118,398
250.6310,28512,725251.1611,13118,398250.6410,30112,828251.1711,14718,509
250.65 10,317 12,931 251.18 11,164 18,621
250.66 10,317 12,931 251.16 11,104 16,021 250.66 10,333 13,034 251.19 11,180 18,732
250.67 10,348 13,138 251.20 11,197 18,844
250.68 10,364 13,241 251.21 11,213 18,956
250.69 10,380 13,345 251.22 11,230 19,069
250.70 10,395 13,449 251.23 11,246 19,181
250.71 10,411 13,553 251.24 11,263 19,293
250.72 10,427 13,657 251.25 11,280 19,406
250.73 10,442 13,761 251.26 11,296 19,519
250.74 10,458 13,866 251.27 11,313 19,632
250.75 10,474 13,971 251.28 11,329 19,745
250.76 10,489 14,075 251.29 11,346 19,859
250.77 10,505 14,180 251.30 11,362 19,972
250.78 10,521 14,285 251.31 11,379 20,086
250.79 10,537 14,391 251.32 11,395 20,200
250.80 10,552 14,496 251.33 11,412 20,314
250.81 10,568 14,602 251.34 11,428 20,428
250.82 10,584 14,708 251.35 11,445 20,542
250.83 10,599 14,813 251.36 11,462 20,657

Stage-Area-Storage for Pond P1: Infiltration Pond (continued)

Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)
251.37	11,478	20,772
251.38	11,495	20,887
251.39	11,511	21,002
251.40	11,528	21,117
251.41	11,544	21,232
251.42	11,561	21,348
251.43	11,577	21,463
251.44	11,594	21,579
251.45	11,610	21,695
251.46	11,627	21,811
251.47	11,644	21,928
251.48	11,660	22,044
251.49	11,677	22,161
251.50	11,693	22,278
251.51	11,710	22,395
251.52	11,726	22,512
251.53	11,743	22,629
251.54	11,759	22,747
251.55	11,776	22,865
251.56	11,792	22,982
251.57	11,809	23,100
251.58	11,825	23,219
251.59	11,842	23,337
251.60	11,859	23,455
251.61	11,875	23,574
251.62	11,892	23,693
251.63	11,908	23,812
251.64	11,925	23,931
251.65	11,941	24,050
251.66	11,958	24,170
251.67	11,974	24,290
251.68	11,991	24,409
251.69	12,007	24,529
251.70	12,024	24,649

Elevation Surface Storage Elevation Surface Storage (feet) (sq-ft) (cubic-feet) (feet) (sq-ft) (cubic-feet) 250.00 250.53 250.01 250.54 250.55 250.02 250.03 250.56 250.57 250.04 250.05 250.58 2 2 250.06 250.59 250.07 250.60 250.08 250.61 250.09 250.62 250.63 250.10 250.11 250.64 250.12 250.65 250.13 250.66 250.14 250.67 250.15 250.68 250.16 250.69 250.17 250.70 250.18 250.71 250.19 250.72 250.20 250.73 250.21 250.74 250.22 250.75 250.23 250.76 250.24 250.77 250.25 250.78 250.26 250.79 250.27 250.80 250.28 250.81 250.29 250.82 250.30 250.83 250.31 250.84 250.32 250.85 250.33 250.86 250.87 250.34 250.35 250.88 250.36 250.89 250.90 250.37 250.38 250.91 250.39 250.92 250.40 250.93 250.41 250.94 250.42 250.95 250.43 250.96 250.97 250.44 250.98 250.45 250.46 250.99 250.47 251.00 250.48 251.01 250.49 251.02 250.50 251.03 250.51 251.04 250.52 251.05

Stage-Area-Storage for Pond 4R: Pipe to Infiltration Pond

Stage-Area-Storage for Pond 4R: Pipe to Infiltration Pond (continued)

			·		-
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
251.06	221	114	251.59	579	326
251.07	228	116	251.60	585	331
251.08	235	118	251.61	592	337
251.09	242	121	251.62	599	343
251.10	248	123	251.63	606	349
251.11	255	125	251.64	612	355
251.12 251.13	262 269	128 131	251.65 251.66	619 626	362 368
251.13	269 275	133	251.60	633	300 374
251.14	275	135	251.68	639	374 380
251.15	289	130	251.69	646	387
251.10	296	142	251.00	653	393
251.18	302	145	251.70	660	400
251.10	309	148	251.72	666	407
251.20	316	151	251.73	673	413
251.21	323	154	251.74	680	420
251.22	329	158	251.75	687	427
251.23	336	161	251.76	693	434
251.24	343	164	251.77	700	441
251.25	350	168	251.78	707	448
251.26	356	171	251.79	713	455
251.27	363	175	251.80	720	462
251.28	370	179	251.81	727	469
251.29	376	182	251.82	734	477
251.30	383	186	251.83	740	484
251.31	390	190	251.84	747	491
251.32	397	194	251.85	754	499
251.33	403	198	251.86	761	506
251.34	410	202	251.87	767	514
251.35	417	206	251.88	774	522
251.36	424	210	251.89	781	530
251.37	430	215	251.90	788	537
251.38	437	219	251.91	794	545
251.39	444	223	251.92	801	553
251.40 251.41	451 457	228 232	251.93 251.94	808 815	561 569
251.41	457 464	232 237	251.94	815	578
251.42	404	237	251.95	828	586
251.43	478	242	251.90	835	594
251.45	484	240	251.98	842	603
251.46	491	256	251.99	848	611
251.47	498	261	252.00	855	620
251.48	505	266	202.00		020
251.49	511	271			
251.50	518	276			
251.51	525	281			
251.52	531	287			
251.53	538	292			
251.54	545	298			
251.55	552	303			
251.56	558	309			
251.57	565	314			
251.58	572	320			
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Surface Elevation Surface Elevation Storage Storage (feet) (cubic-feet) (feet) (sq-ft) (cubic-feet) (sq-ft) 249.25 8,187 249.78 8,971 4,547 0 249.26 8,202 82 249.79 8,986 4,637 249.27 8,217 164 249.80 9,001 4,727 249.28 8,231 246 249.81 9,016 4,817 8,246 329 249.82 9,031 4,907 249.29 249.30 8,261 411 249.83 9.045 4,997 249.31 8,276 494 249.84 9,060 5,088 249.32 8,291 577 249.85 9,075 5,179 249.33 8,305 660 249.86 9,090 5,269 249.34 8,320 743 249.87 9,105 5,360 249.35 826 249.88 8,335 9,119 5,452 249.36 8,350 910 249.89 9,134 5,543 249.37 8,365 993 249.90 9,149 5.634 249.38 8,379 1,077 249.91 9,164 5,726 249.39 8,394 1,161 249.92 9,179 5,817 249.40 8,409 1,245 249.93 9,193 5,909 249.41 8,424 1,329 249.94 9,208 6,001 249.42 8,439 1,413 249.95 9,223 6,093 249.43 8,453 1,498 249.96 9,238 6,186 249.44 8,468 1,582 249.97 9,253 6,278 249.45 8,483 1,667 249.98 9,267 6,371 8,498 1,752 249.99 9,282 6,464 249.46 8,513 1,837 9,297 6,557 249.47 250.00 249.48 8,527 1,922 250.01 9,313 6.650 249.49 8,542 2,008 250.02 9,328 6,743 249.50 8,557 2,093 250.03 9,344 6.836 2,179 6,930 249.51 8,572 250.04 9,360 2,264 9.375 7.023 249.52 8,587 250.05 249.53 8,601 2,350 250.06 9,391 7,117 8,616 7,211 249.54 2,436 250.07 9.407 7,305 9,423 249.55 8,631 2,523 250.08 249.56 8,646 2,609 250.09 9,438 7,400 250.10 8,661 2,696 9,454 7,494 249.57 250.11 9,470 249.58 8,675 2,782 7,589 8,690 2,869 250.12 9,485 249.59 7,683 250.13 9.501 7.778 249.60 8,705 2,956 249.61 8,720 3.043 250.14 9,517 7,873 250.15 9,532 7,969 249.62 8,735 3,130 249.63 8,749 3,218 250.16 9,548 8,064 249.64 8,764 3,305 250.17 9,564 8,160 8,255 249.65 8,779 3,393 250.18 9,579 249.66 8,794 3,481 250.19 9.595 8,351 250.20 9.611 8.447 249.67 8,809 3,569 8,543 249.68 8,823 3.657 250.21 9.626 249.69 8,838 3,746 250.22 9,642 8,640 8,736 249.70 8,853 3,834 250.23 9,658 3,923 8.833 249.71 8,868 250.24 9,674 4,011 9,689 8,930 249.72 8,883 250.25 4,100 8,897 250.26 9,705 9,027 249.73 4,189 9,124 249.74 8,912 250.27 9,721 249.75 8,927 4,279 250.28 9,736 9,221 249.76 8,942 4,368 250.29 9,752 9,319

249.77

8,957

4,457

250.30

9,768

9,416

Stage-Area-Storage for Pond P1: Infiltration Pond

Stage-Area-Storage for Pond P1: Infiltration Pond (continued)

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250.70 10,395 13,449 251.23 11,246 19,181					
250.71 10,411 13,553 251.24 11,263 19,293					
250.72 10,427 13,657 251.25 11,280 19,406					
250.73 10,442 13,761 251.26 11,296 19,519					
250.74 10,458 13,866 251.27 11,313 19,632					
250.75 10,474 13,971 251.28 11,329 19,745					
250.76 10,489 14,075 251.29 11,346 19,859					
250.77 10,505 14,180 251.30 11,362 19,972	•				
250.78 10,521 14,285 251.31 11,379 20,086					
250.79 10,537 14,391 251.32 11,395 20,200					
250.80 10,552 14,496 251.33 11,412 20,314					
250.81 10,568 14,602 251.34 11,428 20,428				,	
250.82 10,584 14,708 251.35 11,445 20,542					
250.83 10,599 14,813 251.36 11,462 20,657					
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Stage-Area-Storage for Pond P1: Infiltration Pond (continued)

Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)
251.37	11,478	20,772
251.38	11,495	20,887
251.39	11,511	21,002
251.40	11,528	21,117
251.41	11,544	21,232
251.42	11,561	21,348
251.43	11,577	21,463
251.44	11,594	21,579
251.45	11,610	21,695
251.46	11,627	21,811
251.47	11,644	21,928
251.48	11,660	22,044
251.49	11,677	22,161
251.50	11,693	22,278
251.51	11,710	22,395
251.52	11,726	22,512
251.53	11,743	22,629
251.54	11,759	22,747
251.55	11,776	22,865
251.56	11,792	22,982
251.57	11,809	23,100
251.58	11,825	23,219
251.59	11,842	23,337
251.60	11,859	23,455
251.61	11,875	23,574
251.62	11,892	23,693
251.63	11,908	23,812
251.64	11,925	23,931
251.65	11,941	24,050
251.66	11,958	24,170
251.67	11,974	24,290
251.68	11,991	24,409
251.69	12,007	24,529
251.70	12,024	24,649



Appendix G – Mounding Calculations

Sheldon West - Infiltration Basin 1

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

	use consistent units (e.g. feet & days or inches & hours)	Conversion Table inch/hour feet/	/day
R	Recharge (infiltration) rate (feet/day)	0.67	1.33
Sy	Specific yield, Sy (dimensionless, between 0 and 1)		
<mark>з</mark> к	Horizontal hydraulic conductivity, Kh (feet/day)*	2.00	4.00 In the report accompanying this spreadsheet
x C	1/2 length of basin (x direction, in feet)		(USGS SIR 2010-5102), vertical soil permeability
y y	1/2 width of basin (y direction, in feet)	hours days	
) t	duration of infiltration period (days)	36	1.50 hydraulic conductivity (ft/d).
) hi(0)	initial thickness of saturated zone (feet)		

16.058 h(max) 1.058 Δh(max) Ground- Distance from

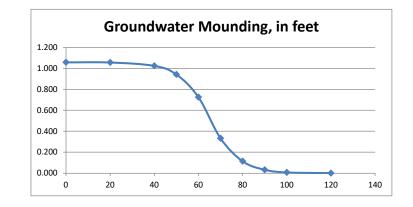
Input Values 0.2830 0.260 2.83 65.000 31.000 1.000 15.000

water center of basin Mounding, in in x direction, in feet feet

ieet		ieet	
	1.058		0
	1.057		20
	1.025		40
	0.942		50
	0.725		60
	0.333		70
	0.114		80
	0.032		90
	0.007		100
	0.001		120

maximum thickness of saturated zone (beneath center of basin at end of infiltration period) maximum groundwater mounding (beneath center of basin at end of infiltration period)

Re-Calculate Now



Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.