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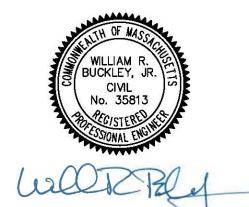
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Stormwater Management Plan "10 Commerce Boulevard" Wrentham, MA

April, 2023



Prepared for:

Edgewood Development Co., LLC 320 South Street Plainville, MA 02762

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1.0 Introduction

The 10 Commerce Boulevard project (Project) is the third and final phase of the development of the Wrentham Business Center. The first phase permitted in 2017 involved the construction of a 90,000 sf indoor recreational building on 40 Commerce Boulevard (Lot 2) and the construction/modification of Commerce Boulevard. The second phase was permitted in 2022 with the approval of a 180,000 sf warehouse located on 15 Commerce Boulevard (Lot 3). Phase 3 involves the construction of a 4,500 sf convenience store that includes six fuel pumps. The facility will be located on Lot 1, which is a 2.6 acre parcel located at the southwest intersection of Commerce Blvd and Route One.

Phase 1 was permitted in 2017 and constructed shortly thereafter and is operational. Phase 2 was permitted in 2022 and is currently being appealed. The design documents for Phase 2 were reviewed for the Town by Beals and Thomas, Inc. and Professional Services Corporation, PC and copies of the final storm water report are relevant to the current design and are available electronically at the BCG FTP site - https://personal.filesanywhere.com/fs/v.aspx?v=8e6e628c5d6273ab72a2

This report will discuss the entire Wrentham Business Center storm water design with an emphasis on Lot 1. Lot 1 was included in the Phase 1 and Phase 2 designs and the Phase 2 design should be reviewed for background,

2.0 Existing Conditions

The project area lies on a terrace that is located between Rabbit Hill Brook and Route One. Because the Wrentham Business Center site had been heavily developed before and during the time when it housed Basic Rental, and in conformance with the 2017 and 2022 Storm Water Reports for Phase 1 and Phase 2, we have used the condition that existed prior to the start of development in 2001, which is more conservative than using the existing condition. The site generally slopes toward Rabbit Hill Brook with a small section of the site flowing to a wetland to the south that is also part of the Rabbit Hill Brook drainage basin. There was a depression in the middle of the site that captures and infiltrates a large area, subarea EA, that at some point overflowed into subarea EC, which drained to Rabbit Hill Brook. That condition was not modeled and an assumption was made that runoff from all events was captured and infiltrated within the depression. This assumption was conservative in that any overflow to subarea EC would result in a higher existing condition rate and volume of runoff to the design point. This would have allowed the developed condition to have a higher rate and volume of runoff. As will be seen, this is not an issue since the post-development rates and volumes are 14-25% of the pre-development conditions. Lot 3 contains a storm water basin that was constructed as part of the 2001 design. Subsequent to that construction, the DEP regulations were modified and now BMPs are not allowed within 200' of a tributary to a surface water supply. The cranberry bogs to the east have been so designated which requires that the existing basin located on Lot 3 to be removed, which was assumed in the design.

The NRCS has logged the soils in the upland portion of the site as Hinckley and Udorthents, sandy; both A soils. The wetland soils were logged as Scarboro, a D soil **Appendix F.** The vegetation on the upland portion of the site was largely a pine forest. Stone walls and trails run throughout the site. BCG conducted soil evaluations throughout the site between 2001 and 2022 for the purpose of determining suitability for storm water and for on-site sewage disposal. The most recent soil evaluations in 2022 were conducted in accordance with Standard 3 of the DEP Stormwater Standards are based on the results of the soil investigations, we confirmed that Lot 1 is dominated by NRCS hydrologic Class A soils. The RAWLS table found in the DEP Stormwater Management Standards, Volume 3, Chapter 1 yields an infiltration rate of 8.27 inches per hour for Sand. The hydrogeologic investigation done in 2000 determined that the estimated high ground water in the area of the proposed storm water basin on Lot 1 is at about elevation 193.0' (Well CMW-4 3/30/2000).

Copies of the 2022 soil logs are included in **Appendix F**. The locations of the test pits are shown on Sheet 2.0 of the submitted plan.

3.0 Flood Condition Analyses and Flood Control

The storm water management system will consist of a system to capture and infiltrate the roof runoff from the building and fueling facility canopy before discharge into an infiltration system located south of the proposed building. The parking lot runoff will be captured through a catch basin/drain manhole system and conveyed to the street drain system that eventually discharges to the storm water basin on Lot 2 as was anticipated in the subdivision design and 2017 Storm Water Report. The separate system that will capture roof runoff from the proposed building and channel it to the infiltration system without it mixing with the parking lot runoff is an LID element.

The assumed existing land use conditions for the majority of project are woods, gravel and paved parking areas, and buildings. Lot 1 is vacant and consists of gravel, pavement, and concrete with some woods along the south lot line. Proposed land uses include buildings, driveways, sidewalks, parking lots, landscape, and woods. The land uses for existing and proposed conditions are summarized in **Tables 1a** and **1b**. Because the entire site was also once a gravel pit, the existing forested conditions are assumed to be fair. Again, these assumptions are the same as those in the 2017 and 2022 storm water designs. **Appendix A – Figure 1 Existing Conditions Subarea**.

Table 1a – Summary of Existing Land Uses

Subarea	Total Area (acres)	Land use	Area (acres)
EA	6.58	Roofs, HSG A	0.04
		Paved parking, HSG A	0.61
		Woods, fair, HSG A	5.93
EB	2.06	Woods, fair, HSG A	2.06
EC	23.40	Roofs, HSG A	0.04
		Paved parking, HSG A	7.11
		Woods, fair, HSG A	14.15
		Woods, fair, HSG D	2.10
Total:	32.04	Total:	32.04

As was previously described, the developed area of study is divided into two watersheds that discharge toward Rabbit Hill Brook to the east and to the south property line Appendix A –Developed Conditions Subarea. One of the goals of the storm water design is to ensure that there is no increase in the rate of runoff from the site to the abutting properties and downstream resources. For proposed conditions with significant grading changes, the sub-watersheds are divided in to six separate subareas as well as four future buildings. Subarea DA, which includes Lot 1, drains to basin C3 on Lot 2 largely through the existing trunk drain. Subarea DB consists of sheet flow off to the south property line. Subareas DA and DB have not changed from the 2017 design. Subareas DC and DD are largely located on Lot 3 and were modified in Phase 2. Subarea DC sheet flows directly to Rabbit Hill Brook through the wooded area that contains wetlands and a drainage ditch. Subarea DD flows to the proposed infiltration basin on Lot 3 through a combination of sheet flow and the proposed drainage system to be constructed on Lot 3.

The Lot 1 parking area will be served by a standard catch basin/drain manhole system that will discharge the runoff into the infiltration basin on Lot 2 through the drain system in Commerce Boulevard. In compliance with the Town of Wrentham standards we have integrated LID into the design by separating the clean roof runoff from the parking lot runoff. This separate drain system eliminates the need to treat the roof runoff prior to discharge to the basin. All runoff from the parking lot will be pretreated by a water quality inlet designed to accept to the 2-inch storm event, as well as deep sump catch basins with oil traps, which then discharge to the trunk drain into the infiltration basin on Lot 2. The infiltration system for roof runoff has been designed to capture and infiltrate all storms up to the 100-year event. The same is true for the existing storm water basin on Lot 2 and the proposed storm water basin on Lot 3. This is necessary to eliminate any discharge to the Zone A and results in a significant reduction in rate and volume of runoff from the site. **Table 2 & Appendix A**

Table 1b – Summary of Proposed Land Uses

	Table 10	Summary of Froposcu Land Oscs	
Subarea Area (acres)		Land use	Area (acres)
DA	8.13	Paved roads w/curbs & sewer, HSG A	0.69
		Paved parking, HSG A	2.36
		Grass cover >75%, good, HSG A	2.75
		Water surface, 0% imp, HSG A	0.44
		Woods, fair, HSG A	1.89
Building & Canopy	0.21	Unconnected roofs, HSG A	0.21
DB	1.31	Woods, fair, HSG A	1.21
		Grass cover >75%, good, HSG A	0.10
DC	8.50	Grass cover >75%, good, HSG A	0.43
		Woods, fair, HSG A	5.97
		Woods, fair, HSG D	2.10
DD	11.71	Grass cover >75%, good, HSG A	2.70
		Unconnected roofs, HSG A	4.12
		Paved parking, HSG A	4.28
		Water surface, HSG A	0.61
Bldg A	1.09	Roofs, HSG A	1.09
Bldg B	1.09	Roofs, HSG A	1.09
Total:	32.04	Total:	32.04

The closed parking lot and roadway drainage systems were designed using the Rational Method with the capability of handling a 25-year storm event in accordance with the Wrentham standards. The schematic layout and calculation sheets are included in **Appendix D** of this report.

The pre- and post-developed conditions based on the land uses in **Tables 1a** and **1b** are summarized in **Table 2** and detailed calculations can be found in **Appendix A**.

Table 2: Summary of Peak Runoff (cfs) at the Study Points

Condition		2-inch	2-inch	2-year	2-year	10-year	10-year	50-year	50-year	100-year	100-year	
	COII	uition	(cfs)	(ac-ft)	(cfs)	(ac-ft)	(cfs)	(ac-ft)	(cfs)	(ac-ft)	(cfs)	(ac-ft)
Existing Conditions		South Property Line	0.0	0.000	0.0	0.000	0.0	0.015	0.5	0.114	1.4	0.204
		Rabbit Hill Brook	0.1	0.093	3.4	0.733	15.7	2.229	41.8	5.319	59.2	7.396
		Total	0.1	0.093	3.4	0.733	15.7	2.244	42.3	5.433	60.6	7.600
		South Property Line	0.0	0.000	0.0	0.000	0.0	0.010	0.3	0.072	0.9	0.130
Developed Conditions		Rabbit Hill Brook	0.0	0.000	0.1	0.052	1.2	0.339	6.5	1.106	10.7	1.679
		Total	0.0	0.000	0.1	0.052	1.2	0.349	6.8	1.178	11.6	1.809

The 2-inch, 2-year, 10-year, 50-year, and 100-year flood elevations in the storm water infiltration system for roof runoff is summarized in **Table 3**. The detailed flood routing calculations are attached in **Appendix A**. The infiltration rates used were those outlined in the RAWLs Table in the DEP Stormwater Management Standards for Sand and were discussed in **Section 2.0 Existing Conditions**. Infiltration in the existing Lot 2 infiltration basin and the proposed Lot 1 subsurface system is substantial and will satisfy the Required Recharge Volume.

Table 3: Summary of Stormwater Basin Flood Elevations

Condition	2-inch	2-year	10-year	50-year	100-year	
Lot 1 Infiltration	212.6'	213.0'	213.3'	213.8'	214.2'	
System Elevation	212.0	213.0	213.3	213.0		
Lot 1 Infiltration						
System Storage	0.001 ac-ft	0.005 ac-ft	0.012 ac-ft	0.025 ac-ft	0.033 ac-ft	
Volume						

4.0 Stormwater Management

The site is not located in a groundwater recharge zone (Zone I, II, III), nor are there private drinking water wells around the project site. Rabbit Hill Brook is in the Zone A as a tributary for the Wading River surface water supply. The DEP Stormwater Standards apply to this project, as do the Wrentham Subdivision Rules and Regulations and the Wrentham Board of Health Stormwater Regulations, and the project design is based on the latest edition of these documents.

<u>DEP STORMWATER MANAGEMENT STANDARDS</u>

Standard #1: NO UNTREATED DISCHARGE OR EROSION TO WETLANDS

No untreated stormwater from the proposed project area will be discharged to resource areas. Runoff from all new pavement on Lot 1 will be routed through a water quality structure and deep sump catch basins equipped with "Snout" water quality elbows, then to manholes, and finally to the above-ground storm water basin on Lot 2.

Standard #2: PEAK RATE ATTENUATION

Stormwater controls have been designed for 2-inch, 2-year, 10-year, 50-year, and 100-year storms according to both state and local regulations. The post-development peak discharge rates with flood control do not exceed pre-development rates on the site at the downgradient discharge points. See **Table 2 and Appendix A** for details.

Standard #3: STORMWATER RECHARGE

1) The proposed project is located on a plot with hydrologic class A and D soils based on the NRCS soil map. All of Lot 1 and the majority of the property are in A soils. The target depth factor for an A soil is 0.60 inches. Soil textural analyses

have been conducted in the areas where recharge is proposed and the soils were found to be Sand. Storm water recharge for the parking areas will take place within the infiltration basin on Lot 2 and for the roof runoff in the storm water infiltration system located south of the building. The calculations for the recharge volumes are located in **Appendix B**.

- 2) The infiltration BMP that will be used will be the above-ground storm water basin on Lot 2 and the subsurface infiltration system on Lot 1. The calculation for the recharge volume is located in **Appendix B**.
- 3) Due to the use of the multi-stage treatment train, the actual TSS removal is 93% and the standard is met. **Table 4 & Appendix B**
- 4) Using the RAWLS rate for Sand for the subsurface infiltration system shows that the drawdown of the Required Recharge Volume will take 2 hours. The drawdown of the 10-year storm will take 4 hours and the 100-year storm will take 7 hours. All events meet the required 72 hours dewatering standard **Appendix B**.
- 5) Capture area adjustment is not necessary since all of the infiltration will take place within the infiltration basin on Lot 2 or the subsurface infiltration system on Lot 1 and 100% of the impervious area will be directed to the facilities, which meets the 65% requirement.
- 6) Mounding analysis is not necessary because the bottom of the roof infiltration system is more than 4' above the estimated high ground water, but is included due to the local requirement. The mount will be about 19' below the proposed system.

Standard # 4: WATER QUALITY

- 1) The required water quality volume is based on 8.52 acres of impervious area and 1.0 inch water quality depth, which yields a water quality volume of 0.43 acrefeet.
- 2) The BMPs used for Lot 1 to enhance water quality includes a water quality structure and deep sump catch basins on the site. All of the runoff from the parking areas will go through deep sump catch basins and a water quality structure prior to passage into the street drain system and to the infiltration basin on Lot 2. Runoff from the roof will be piped separately to the infiltration system south of the building. The estimated overall TSS removal will be 93%, which meets the 80% standard **Appendix B & Table 4.**
- 3) The Lot 2 Infiltration Basin is being used to fulfill the requirements of Standards 3 and 4 it must handle the larger of the water quality volumes. The required

Water Quality Volume is 0.83 ac-ft and the basin has a storage volume of 2.73 ac-ft. Therefore, the standard is met. **Appendix B**

4) The water quality inlet was sized using Contech Cascade Maximum Treatment Flow Rate **Sheet 5.3 & Appendix B**

Table 4: Summary of TSS Removal

Cascade Stream						
Impo	ervious Area =	0.77	acres			
Runoff depth t	to be treated =	1.77	inches (2" storr	n)		
Runoff volume to be treated =		0.1136	ac-ft			
	TSS Removal	Starting TSS	Amount	Remaining		
BMP	Rate	Load	Removed	Load		
Deep Sump and						
Hooded CB	0.25	1.00	0.25	0.75		
Cascade CS-8	0.5	0.75	0.38	0.38		
Infiltration Basin (Lot 2)	0.8	0.38	0.30	0.08		
	TOTA	L TSS REMOV	ED =	93	%	

Standard # 5: LAND USES WITH HIGHER POTENTIAL POLLUTION LOADS

The facility will consist of a refueling facility which is considered to have a high potential pollutant load. The recommended BMPs for this land use are identified in Chapter 1, P.14, Table LUHPPL of the Massachusetts Stormwater Handbook, which have been used in the design. Recommended pretreatment includes deep sump catch basins and proprietary separators, which are included in the treatment train. Treatment and infiltration recommendations include infiltration basins, and subsurface structures; all of which are included in the treatment train for runoff from Lot 1.

Standard #6: CRITICAL AREAS

Rabbit Hill Brook is part of the Zone A to the Wading River surface water supply. There is no discharge from the project to the brook from any developed areas since the storm water basins can accommodate the 100-year event without overflow. The closest BMP to the river is about 256' and is located on Lot 3. Table CA6, Vol.1, Ch 1., p.18 was used to choose the specific BMPs for Lot 1. The pretreatment BMP is a deep sump catch basin. The treatment BMP includes a water quality structure and infiltration basin. The infiltration BMPs include the subsurface infiltration system on Lot 1.

Standard #7: REDEVELOPMENT

The proposed activity is a combination development/redevelopment project, but all standards for new development will be met.

Standard #8: CONSTRUCTION PERIOD CONTROLS

Silt sock barriers will be installed at the downgradient limit of work before any excavation starts. A stone pad shall be spread at the entrance from the existing roadway to the project site to prevent mud from escaping the site during construction. Silt sacks will be used in the catch basins during construction.

A Draft Stormwater Pollution Prevention Plan has been developed in accordance with the EPA General Permit for Construction Activities. A final SWPPP will be prepared once the construction schedule is finalized and the contractors are chosen. A copy of the Draft SWPPP is included in **Appendix E**

Standard #9: OPERATION AND MAINTENANCE PLAN

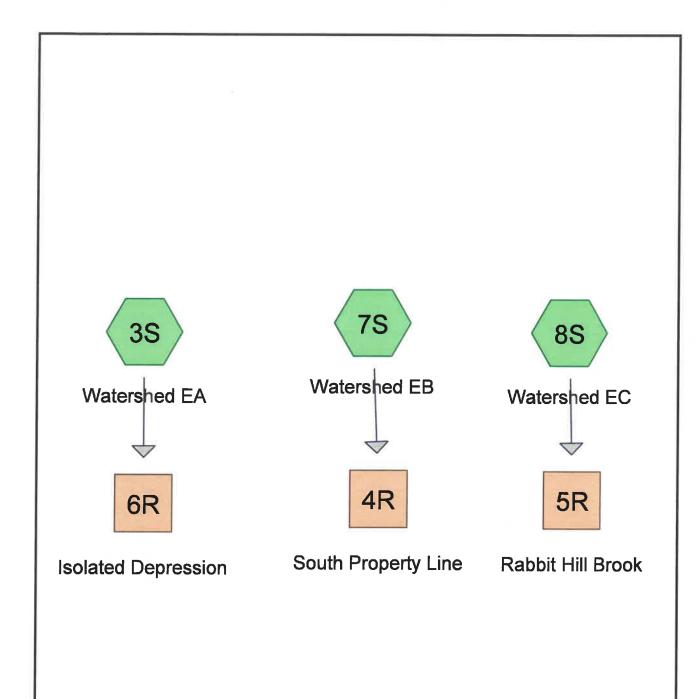
See Appendix C for details.

Standard # 10: ILLICIT DISCHARGES TO DRAINAGE SYSTEM

I certify to the best of my professional knowledge, information and belief that there are no illicit discharges to the stormwater management system, including wastewater discharges and discharges of stormwater contaminated by contact with process wastes, raw materials, toxic pollutants, hazardous substances, oil, or grease. The proposed systems as shown on the referenced plans do not allow entry of any illicit discharges into the system and there are no connections between the stormwater and wastewater management systems.

Further, I certify that the stormwater management system as shown on the referenced
plans will be maintained in accordance with the conditions of the Long Term Pollution
Prevention Plan.

APPENDIX A – Pre- and Post-Development Analysis and Infiltration Systems Designs

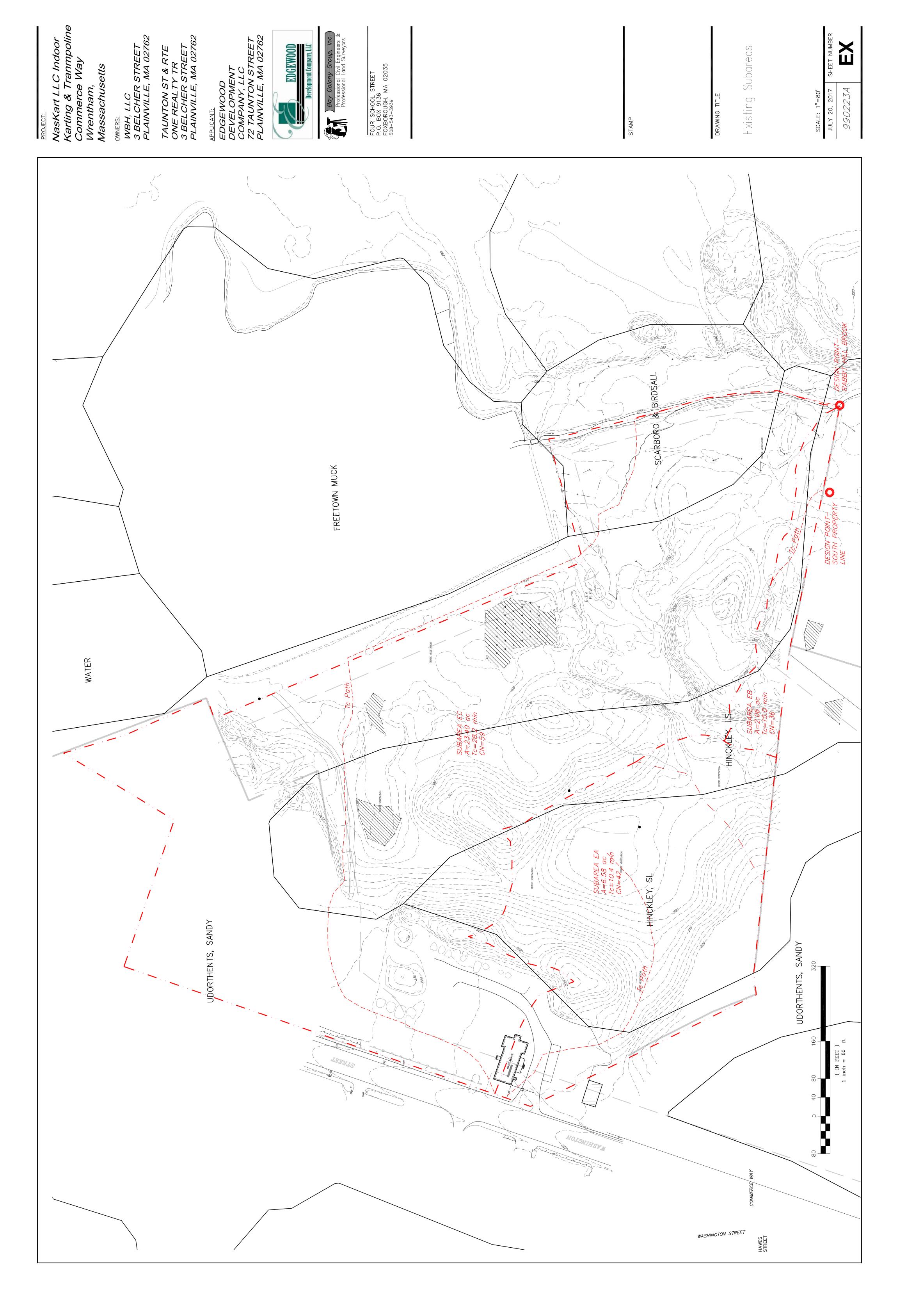












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Page 2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
7.720	98	Paved parking, HSG A (3S, 8S)
0.080	98	Roofs, HSG A (3S, 8S)
22.140	36	Woods, Fair, HSG A (3S, 7S, 8S)
2.100	79	Woods, Fair, HSG D (8S)
32.040	54	TOTAL AREA

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
29.940	HSG A	3S, 7S, 8S
0.000	HSG B	
0.000	HSG C	
2.100	HSG D	8S
0.000	Other	
32.040		TOTAL AREA

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Ground Covers (all nodes)

 HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
7.720	0.000	0.000	0.000	0.000	7.720	Paved parking	3S, 8S
0.080	0.000	0.000	0.000	0.000	0.080	Roofs	3S, 8S
22.140	0.000	0.000	2.100	0.000	24.240	Woods, Fair	3S, 7S, 8S
29.940	0.000	0.000	2.100	0.000	32.040	TOTAL AREA	

Wrentham Business Park - BoH Supplement NRCC 24-hr C 2-Inch Rainfall=2.00"

990223 Ex-Alt

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Runoff Area=6.580 ac 9.88% Impervious Runoff Depth=0.00" Subcatchment 3S: Watershed EA Flow Length=725' Tc=10.4 min CN=42 Runoff=0.0 cfs 0.000 af

Subcatchment 7S: Watershed EB Runoff Area=2.060 ac 0.00% Impervious Runoff Depth=0.00"

Flow Length=228' Tc=15.0 min CN=36 Runoff=0.0 cfs 0.000 af

Runoff Area=23.400 ac 30.56% Impervious Runoff Depth>0.05" Subcatchment 8S: Watershed EC

Flow Length=2,467' Tc=28.2 min CN=59 Runoff=0.1 cfs 0.093 af

Reach 4R: South Property Line Inflow=0.0 cfs 0.000 af

Outflow=0.0 cfs 0.000 af

Inflow=0.1 cfs 0.093 af Reach 5R: Rabbit Hill Brook

Outflow=0.1 cfs 0.093 af

Reach 6R: Isolated Depression Inflow=0.0 cfs 0.000 af

Outflow=0.0 cfs 0.000 af

Total Runoff Area = 32.040 ac Runoff Volume = 0.093 af Average Runoff Depth = 0.03" 75.66% Pervious = 24.240 ac 24.34% Impervious = 7.800 ac

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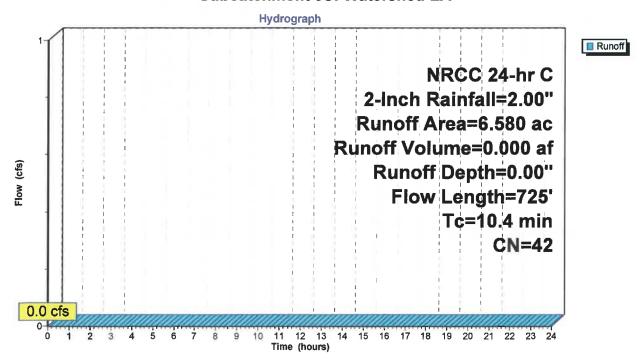
Summary for Subcatchment 3S: Watershed EA

Runoff = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-lnch Rainfall=2.00"

	Area	(ac) C	N Des	cription			
	0.	040 9	98 Root	fs, HSG A			
	0.	610	8 Pave	ed parking	, HSG A		
	5.	.930		ds, Fair, Ě			
-	6.	580 4	12 Weid	hted Aver	age		
		930	•	2% Pervio	•		
	0.	650	9.88	% Impervi	ous Area		
				•			
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	· · · · · · · · · · · · · · · · · · ·	
-	1.2	50	0.0050	0.69		Sheet Flow,	
						Smooth surfaces n= 0.011 P2= 3.20"	
	1.1	140	0.0100	2.03		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
	8.1	535	0.0480	1.10		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
_	10.4	725	Total				

Subcatchment 3S: Watershed EA



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Summary for Subcatchment 7S: Watershed EB

Runoff

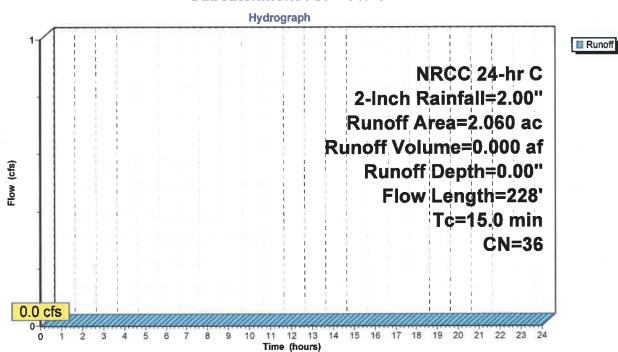
0.0 cfs @ 0.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-Inch Rainfall=2.00"

Area	(ac) C	N Des	cription		
2.	.060 3	6 Woo	ds, Fair, H	ISG A	
2.	.060	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.3	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
2.7	178	0.0500	1.12		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
15.0	228	Total			

Subcatchment 7S: Watershed EB



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Summary for Subcatchment 8S: Watershed EC

Runoff = 0.1 cfs @ 14.65 hrs, Volume=

0.093 af, Depth> 0.05"

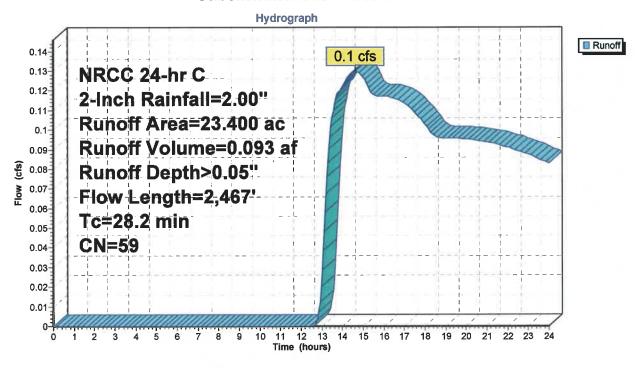
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-lnch Rainfall=2.00"

Area	(ac) C	N Des	cription			
0.	.040	8 Roo	fs, HSG A			
7.	.110 9	8 Pave	ed parking	, HSG A		
14.	.150 3		ds, Fair, F			
2.	.100 7	79 Woo	ds, Fair, F	ISG D		
23.	.400 5	9 Weig	ghted Aver	age		
	.250	69.4	4% Pervio	us Area		
7.	.150	30.5	6% Imper	ious Area		
_				• "	B - 4 44	
Tc	Length	Slope	Velocity		Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
1.2	50	0.0050	0.69		Sheet Flow,	
					Smooth surfaces n= 0.011 P2= 3.20"	
3.7	580	0.0170	2.65		Shallow Concentrated Flow,	
0.4	407	0.0400	4.00		Paved Kv= 20.3 fps	
8.1	487	0.0400	1.00		Shallow Concentrated Flow,	
8.5	E 4 2	0.0050	1.06		Woodland Kv= 5.0 fps	
0.0	542	0.0050	1.06		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps	
5.2	350	0.0500	1.12		Shallow Concentrated Flow,	
5.2	330	0.0300	1.12		Woodland Kv= 5.0 fps	
1.5	458	0.0040	5.06	91.16	Trap/Vee/Rect Channel Flow,	
1.0	400	0.0040	5.00	51.10	Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'	
					n= 0.022 Earth, clean & straight	
28.2	2,467	Total				
20.2	2,407	IOIAI				

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Subcatchment 8S: Watershed EC



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Summary for Reach 4R: South Property Line

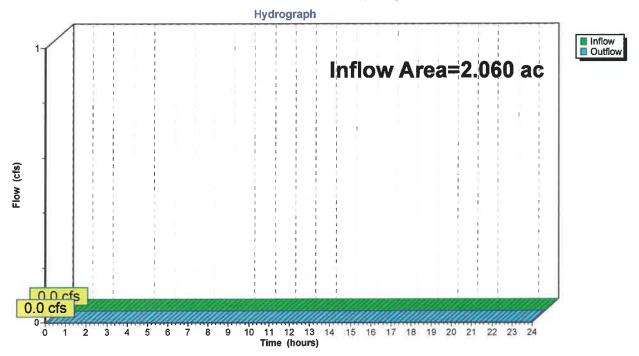
2.060 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Inch event Inflow Area =

0.00 hrs, Volume= 0.000 af Inflow 0.0 cfs @

0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min Outflow 0.0 cfs @

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 4R: South Property Line



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Summary for Reach 5R: Rabbit Hill Brook

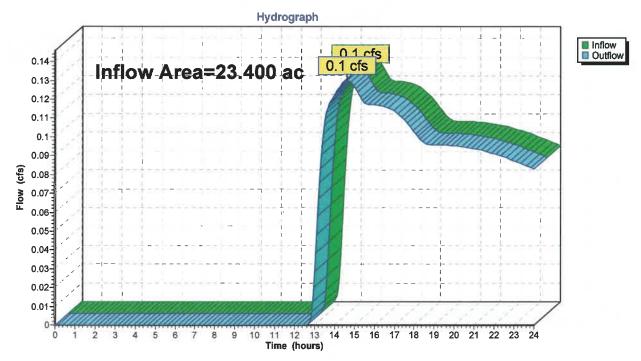
Inflow Area = 23.400 ac, 30.56% Impervious, Inflow Depth > 0.05" for 2-Inch event

Inflow = 0.1 cfs @ 14.65 hrs, Volume= 0.093 af

Outflow = 0.1 cfs @ 14.65 hrs, Volume= 0.093 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 5R: Rabbit Hill Brook



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Summary for Reach 6R: Isolated Depression

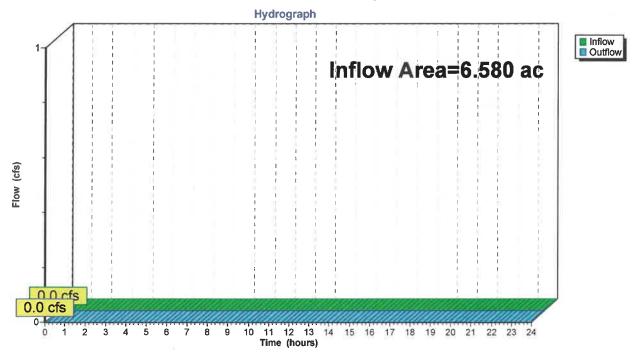
Inflow Area = 6.580 ac, 9.88% Impervious, Inflow Depth = 0.00" for 2-Inch event

Inflow = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Outflow = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 6R: Isolated Depression



Wrentham Business Park - BoH Supplement NRCC 24-hr C 2-Year Rainfall=3.22"

990223 Ex-Alt

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: Watershed EA Runoff Area=6.580 ac 9.88% Impervious Runoff Depth>0.01" Flow Length=725' Tc=10.4 min CN=42 Runoff=0.0 cfs 0.008 af

Flow Length-725 TC-18.4 ItiliT CN-42 Runon-0.0 CIS 0.000 at

Subcatchment 7S: Watershed EB Runoff Area=2.060 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=228' Tc=15.0 min CN=36 Runoff=0.0 cfs 0.000 af

Subcatchment8S: Watershed EC Runoff Area=23.400 ac 30.56% Impervious Runoff Depth>0.38"

Flow Length=2,467' Tc=28.2 min CN=59 Runoff=3.4 cfs 0.733 af

Reach 4R: South Property Line Inflow=0.0 cfs 0.000 af Outflow=0.0 cfs 0.000 af

Outliow=0.0 cis 0.000 ai

Reach 5R: Rabbit Hill Brook Inflow=3.4 cfs 0.733 af

Outflow=3.4 cfs 0.733 af

Reach 6R: Isolated Depression Inflow=0.0 cfs 0.008 af

Outflow=0.0 cfs 0.008 af

Total Runoff Area = 32.040 ac Runoff Volume = 0.741 af Average Runoff Depth = 0.28" 75.66% Pervious = 24.240 ac 24.34% Impervious = 7.800 ac

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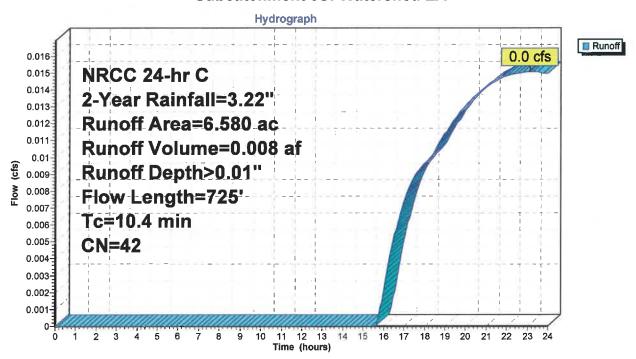
Summary for Subcatchment 3S: Watershed EA

Runoff = 0.0 cfs @ 23.13 hrs, Volume= 0.008 af, Depth> 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

12	Area	(ac) C	N Des	cription		
-	0.	040 9	98 Roo	fs, HSG A		
	0.	610 9	98 Pave	ed parking	, HSG A	
5.930 36 Woods, Fair, HSG A						
	6.	580 4	42 Weig	ghted Aver	age	
	5.	930	90.1	2% Pervio	us Area	
	0.	650	9.88	% Impervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	1.2	50	0.0050	0.69		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 3.20"
	1.1	140	0.0100	2.03		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	8.1	535	0.0480	1.10		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	10.4	725	Total			

Subcatchment 3S: Watershed EA



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Summary for Subcatchment 7S: Watershed EB

Runoff

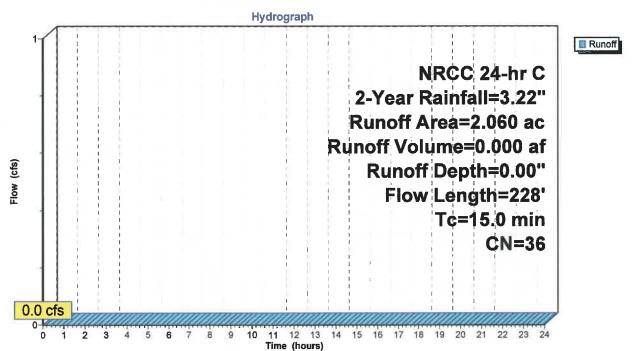
0.0 cfs @ 0.00 hrs, Volume=

0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

Area	(ac) C	N Des	cription	_	
2.	.060 3	6 Woo	ds, Fair, H	ISG A	
2.	.060	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.3	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
2.7	178	0.0500	1.12		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
15.0	228	Total			

Subcatchment 7S: Watershed EB



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Summary for Subcatchment 8S: Watershed EC

3.4 cfs @ 12.53 hrs, Volume= Runoff

0.733 af, Depth> 0.38"

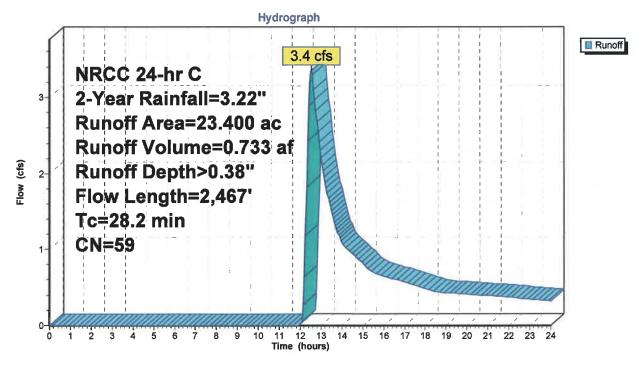
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

Area	(ac) C	N Desc	cription		
0.	.040 9	8 Roof	fs, HSG A		
7.	.110 9	8 Pave	ed parking	, HSG A	
14.	.150 3		ds, Fair, F		
2.	.100 7		ds, Fair, F		
23.	.400 5	59 Weig	hted Aver	age	
	250	•	4% Pervio		
	150			ious Area	
			· / · · · · · · · · · · · · · · · · · ·		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
1.2	50	0.0050	0.69	1	Sheet Flow,
	00	0.0000	0.00		Smooth surfaces n= 0.011 P2= 3.20"
3.7	580	0.0170	2.65		Shallow Concentrated Flow,
0.,	000	0.0170	2.00		Paved Kv= 20.3 fps
8.1	487	0.0400	1.00		Shallow Concentrated Flow,
0.1	401	0.0-100	1.00		Woodland Kv= 5.0 fps
8.5	542	0.0050	1.06		Shallow Concentrated Flow,
0.0	072	0.0000	1.00		Grassed Waterway Kv= 15.0 fps
5.2	350	0.0500	1.12		Shallow Concentrated Flow,
5.2	550	0.0000	1.12		Woodland Kv= 5.0 fps
1.5	458	0.0040	5.06	91.16	
1.5	430	0.0040	3.00	91,10	Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'
					n= 0.022 Earth, clean & straight
	0.40=	T . (.)			II- U.UZZ Eartii, Clean & Straight
28.2	2,467	Total			

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Subcatchment 8S: Watershed EC



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Summary for Reach 4R: South Property Line

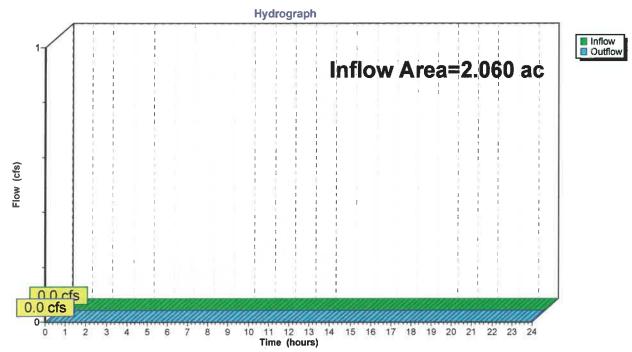
Inflow Area = 2.060 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event

Inflow 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min Outflow 0.0 cfs @

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 4R: South Property Line



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Summary for Reach 5R: Rabbit Hill Brook

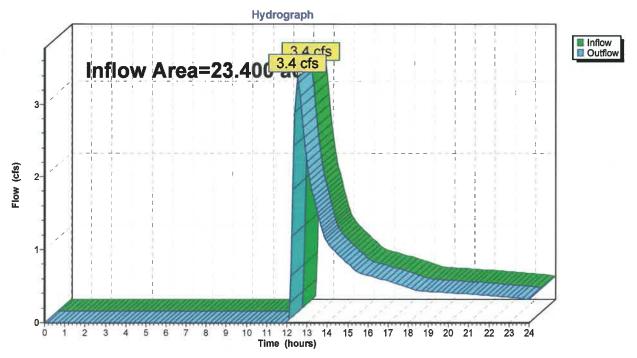
Inflow Area = 23.400 ac, 30.56% Impervious, Inflow Depth > 0.38" for 2-Year event

Inflow = 3.4 cfs @ 12.53 hrs, Volume= 0.733 af

Outflow = 3.4 cfs @ 12.53 hrs, Volume= 0.733 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 5R: Rabbit Hill Brook



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Summary for Reach 6R: Isolated Depression

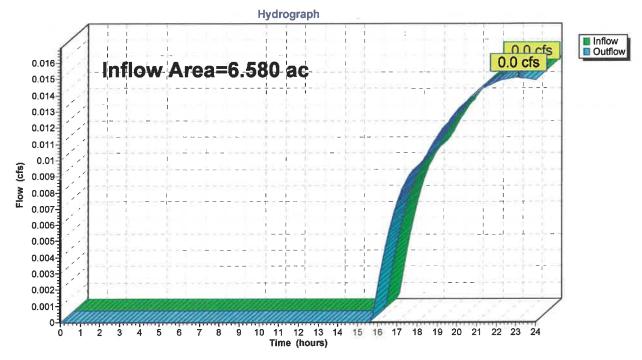
Inflow Area = 6.580 ac, 9.88% Impervious, Inflow Depth > 0.01" for 2-Year event

Inflow = 0.0 cfs @ 23.13 hrs, Volume= 0.008 af

Outflow = 0.0 cfs @ 23.13 hrs, Volume= 0.008 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 6R: Isolated Depression



Wrentham Business Park - BoH Supplement NRCC 24-hr C 10-Year Rainfall=4.86"

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment3S: Watershed EA Runoff Area=6.580 ac 9.88% Impervious Runoff Depth>0.27"

Flow Length=725' Tc=10.4 min CN=42 Runoff=0.4 cfs 0.151 af

Subcatchment 7S: Watershed EB Runoff Area=2.060 ac 0.00% Impervious Runoff Depth>0.09"

Flow Length=228' Tc=15.0 min CN=36 Runoff=0.0 cfs 0.015 af

Subcatchment 8S: Watershed EC Runoff Area=23.400 ac 30.56% Impervious Runoff Depth>1.14"

Flow Length=2,467' Tc=28.2 min CN=59 Runoff=15.7 cfs 2.229 af

Reach 4R: South Property Line Inflow=0.0 cfs 0.015 af

Outflow=0.0 cfs 0.015 af

Reach 5R: Rabbit Hill Brook Inflow=15.7 cfs 2.229 af

Outflow=15.7 cfs 2.229 af

Reach 6R: Isolated Depression Inflow=0.4 cfs 0.151 af

Outflow=0.4 cfs 0.151 af

Total Runoff Area = 32.040 ac Runoff Volume = 2.395 af Average Runoff Depth = 0.90" 75.66% Pervious = 24.240 ac 24.34% Impervious = 7.800 ac

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Summary for Subcatchment 3S: Watershed EA

Runoff =

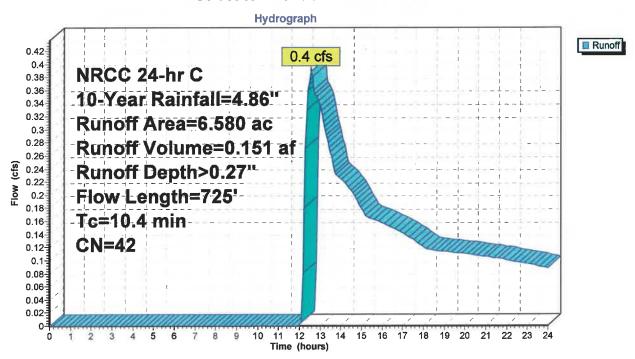
0.4 cfs @ 12.57 hrs, Volume=

0.151 af, Depth> 0.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

2.50	Area	(ac) C	N Desc	cription			
-	0.	040 9	98 Root	s, HSG A			
	0.	610 9	8 Pave	ed parking	, HSG A		
	5.	930 3	36 Woo	ds, Fair, F	ISG A		
- 77	6.	.580 4	12 Weig	thted Aver	age		
	5.	930	90.1	2% Pervio	us Area		
	0.	650	9.88	% Impervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	1.2	50	0.0050	0.69		Sheet Flow,	
						Smooth surfaces n= 0.011 P2= 3.20"	
	1.1	140	0.0100	2.03		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
	8.1	535	0.0480	1.10		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	10.4	725	Total				

Subcatchment 3S: Watershed EA



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Summary for Subcatchment 7S: Watershed EB

Runoff =

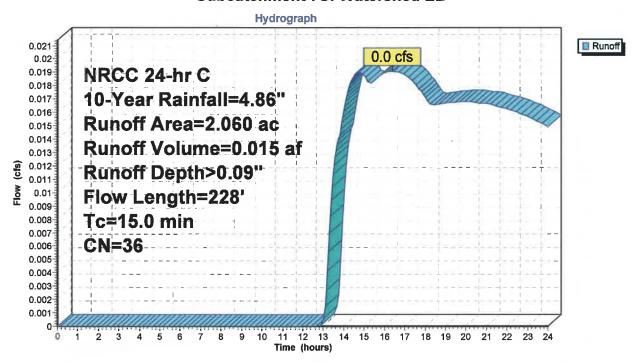
0.0 cfs @ 16.39 hrs, Volume=

0.015 af, Depth> 0.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

Area	(ac) C	N Des	cription		
2.	060 3	36 Woo	ds, Fair, H		
2.	060	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.3	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
2.7	178	0.0500	1.12		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
15.0	228	Total			

Subcatchment 7S: Watershed EB



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Summary for Subcatchment 8S: Watershed EC

Runoff 15.7 cfs @ 12.44 hrs, Volume= 2.229 af, Depth> 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

Area	(ac) C	N Des	cription			
0.	.040 9	8 Root	fs, HSG A			
7.	.110		ed parking			
			ds, Fair, F		9	
2	.100 7	79 Woo	ds, Fair, F	ISG D		_
23.	.400 5		ghted Aver			
	.250		4% Pervio			
7.	.150	30.5	6% Impen	ious Area		
Tc	Longth	Slone	\/olooitr	Capacity	Description	
(min)	Length (feet)	(ft/ft)	Velocity (ft/sec)	(cfs)	Description	
1.2	50	0.0050	0.69	(013)	Sheet Flow,	_
1.2	50	0.0000	0.00		Smooth surfaces n= 0.011 P2= 3.20"	
3.7	580	0.0170	2.65		Shallow Concentrated Flow,	
0.,		0.01.0			Paved Kv= 20.3 fps	
8.1	487	0.0400	1.00		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
8.5	542	0.0050	1.06		Shallow Concentrated Flow,	
					Grassed Waterway Kv= 15.0 fps	
5.2	350	0.0500	1.12		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
1.5	458	0.0040	5.06	91.16	Trap/Vee/Rect Channel Flow,	
					Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'	
	0.407				n= 0.022 Earth, clean & straight	
28.2	2,467	Total				

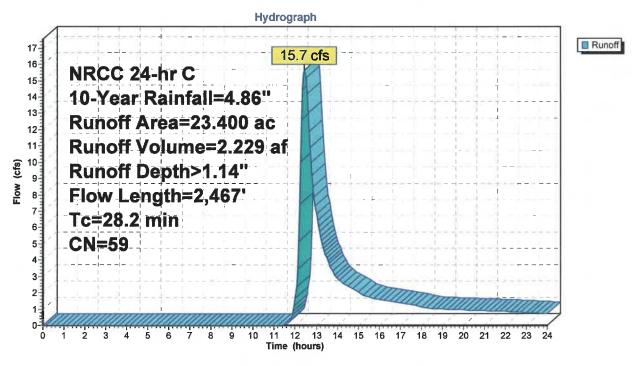
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Subcatchment 8S: Watershed EC



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Summary for Reach 4R: South Property Line

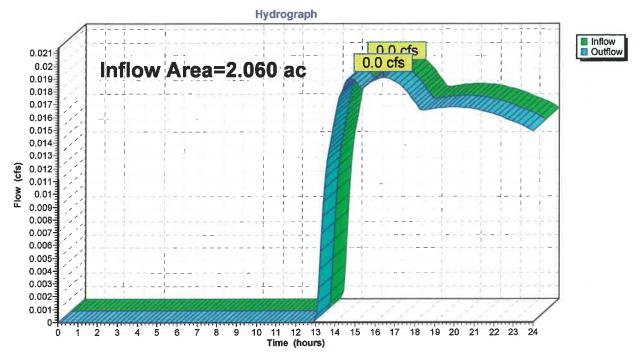
Inflow Area = 2.060 ac, 0.00% Impervious, Inflow Depth > 0.09" for 10-Year event

Inflow = 0.0 cfs @ 16.39 hrs, Volume= 0.015 af

Outflow = 0.0 cfs @ 16.39 hrs, Volume= 0.015 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 4R: South Property Line



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Summary for Reach 5R: Rabbit Hill Brook

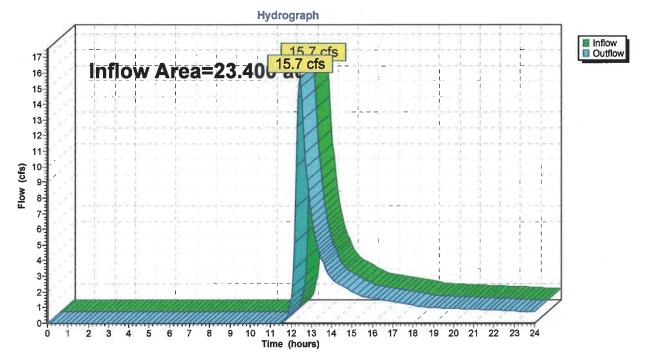
Inflow Area = 23.400 ac, 30.56% Impervious, Inflow Depth > 1.14" for 10-Year event

15.7 cfs @ 12.44 hrs, Volume= 2.229 af Inflow

15.7 cfs @ 12.44 hrs, Volume= Outflow 2.229 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 5R: Rabbit Hill Brook



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Summary for Reach 6R: Isolated Depression

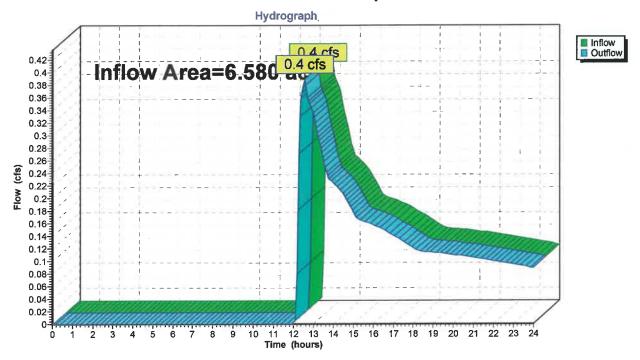
6.580 ac, 9.88% Impervious, Inflow Depth > 0.27" for 10-Year event Inflow Area =

0.4 cfs @ 12.57 hrs, Volume= 0.151 af Inflow

0.4 cfs @ 12.57 hrs, Volume= 0.151 af, Atten= 0%, Lag= 0.0 min Outflow

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 6R: Isolated Depression



Wrentham Business Park - BoH Supplement NRCC 24-hr C 50-Year Rainfall=7.35"

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment3S: Watershed EA Runoff Area=6.580 ac 9.88% Impervious Runoff Depth>1.14"

Flow Length=725' Tc=10.4 min CN=42 Runoff=5.8 cfs 0.625 af

Subcatchment7S: Watershed EB Runoff Area=2.060 ac 0.00% Impervious Runoff Depth>0.66"

Flow Length=228' Tc=15.0 min CN=36 Runoff=0.5 cfs 0.114 af

Subcatchment8S: Watershed EC Runoff Area=23.400 ac 30.56% Impervious Runoff Depth>2.73"

Flow Length=2,467' Tc=28.2 min CN=59 Runoff=41.8 cfs 5.319 af

Reach 4R: South Property Line Inflow=0.5 cfs 0.114 af

Outflow=0.5 cfs 0.114 af

Reach 5R: Rabbit Hill Brook Inflow=41.8 cfs 5.319 af

Outflow=41.8 cfs 5.319 af

Reach 6R: Isolated Depression Inflow=5.8 cfs 0.625 af

Outflow=5.8 cfs 0.625 af

Total Runoff Area = 32.040 ac Runoff Volume = 6.057 af Average Runoff Depth = 2.27"
75.66% Pervious = 24.240 ac 24.34% Impervious = 7.800 ac

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Summary for Subcatchment 3S: Watershed EA

Runoff =

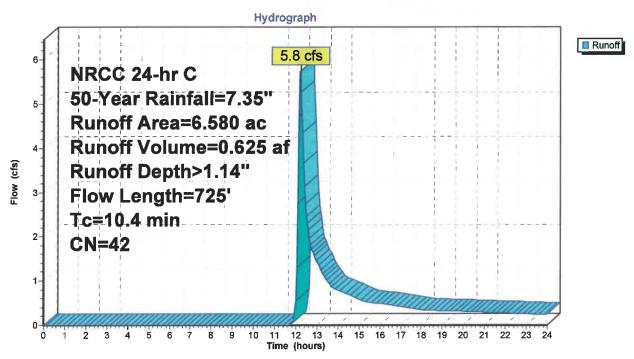
5.8 cfs @ 12.21 hrs, Volume=

0.625 af, Depth> 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 50-Year Rainfall=7.35"

	Area	(ac) C	N Des	cription		
	0.	040	98 Roo	fs, HSG A		
				ed parking	HSG A	
				ds, Fair, H		
_						
				hted Aver		
		930		2% Pervio		
	0.	650	9.88	% Impervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
-	1.2	50	0.0050	0.69		Sheet Flow,
			0.0000	0.00		Smooth surfaces n= 0.011 P2= 3.20"
	1.1	140	0.0100	2.03		Shallow Concentrated Flow,
	1.1	140	0.0100	2.03		
	0.4	-0-	0.0400	4.40		Paved Kv= 20.3 fps
	8.1	535	0.0480	1.10		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	10.4	725	Total			

Subcatchment 3S: Watershed EA



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Summary for Subcatchment 7S: Watershed EB

Runoff =

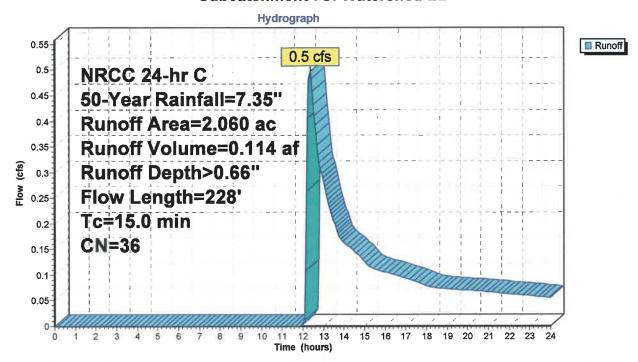
0.5 cfs @ 12.35 hrs, Volume=

0.114 af, Depth> 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 50-Year Rainfall=7.35"

Area	Area (ac) CN Description										
2	2.060 36 Woods, Fair, HSG A										
2	.060	100.	00% Pervi	ous Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
12.3	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"						
2.7	178	0.0500	1.12		Shallow Concentrated Flow, Woodland Kv= 5.0 fps						
15.0	228	Total									

Subcatchment 7S: Watershed EB



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Summary for Subcatchment 8S: Watershed EC

Runoff = 41.8 cfs @ 12.41 hrs, Volume=

5.319 af, Depth> 2.73"

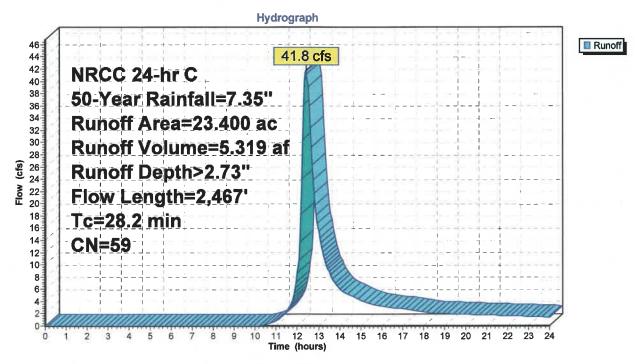
Area	(ac) C	N Des	cription		
0.	.040 9	8 Roo	fs, HSG A		
7.	.110		ed parking		
14.	.150 3	36 Woo	ds, Fair, F	ISG A	
2.	.100	79 Woo	ds, Fair, F	ISG D	
23.	400 5	59 Weig	hted Aver	age	
16.	.250	69.4	4% Pervio	us Area	
7.	150	30.5	6% Imper	ious Area	
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
1.2	50	0.0050	0.69		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 3.20"
3.7	580	0.0170	2.65		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
8.1	487	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
8.5	542	0.0050	1.06		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
5.2	350	0.0500	1.12		Shallow Concentrated Flow,
4.5	450	0.0040	= 00	04.40	Woodland Kv= 5.0 fps
1.5	458	0.0040	5.06	91.16	Trap/Vee/Rect Channel Flow,
					Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'
					n= 0.022 Earth, clean & straight
28.2	2,467	Total			

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Subcatchment 8S: Watershed EC



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Summary for Reach 4R: South Property Line

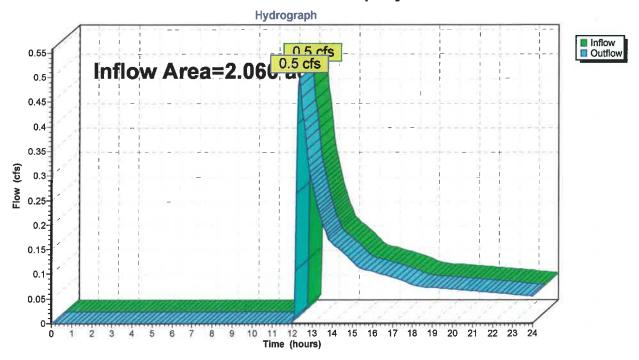
2.060 ac, 0.00% Impervious, Inflow Depth > 0.66" for 50-Year event 0.5 cfs @ 12.35 hrs, Volume= 0.114 af 0.5 cfs @ 12.35 hrs, Volume= 0.114 af, Atten= 0%, Lag= 0.0 m Inflow Area =

Inflow

Outflow 0.114 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 4R: South Property Line



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Summary for Reach 5R: Rabbit Hill Brook

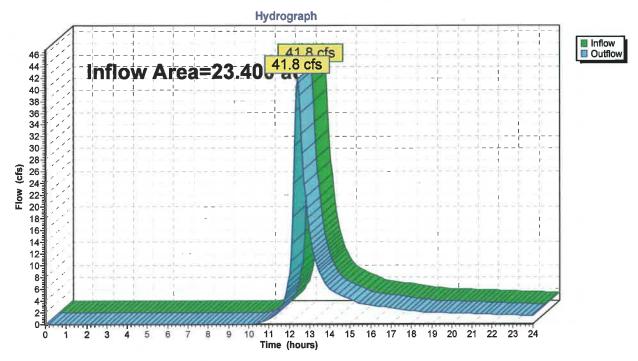
23.400 ac, 30.56% Impervious, Inflow Depth > 2.73" for 50-Year event Inflow Area =

41.8 cfs @ 12.41 hrs, Volume= 41.8 cfs @ 12.41 hrs, Volume= 5.319 af Inflow

Outflow 5.319 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 5R: Rabbit Hill Brook



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Summary for Reach 6R: Isolated Depression

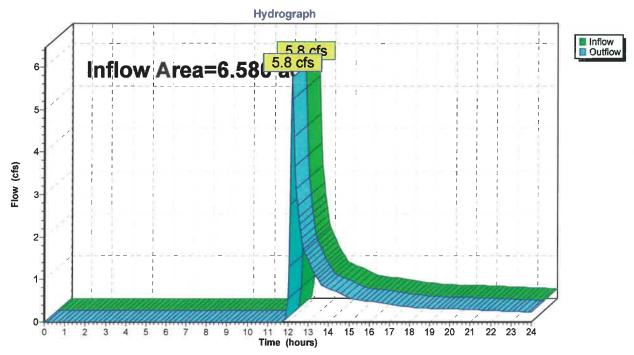
Inflow Area = 6.580 ac, 9.88% Impervious, Inflow Depth > 1.14" for 50-Year event

Inflow 5.8 cfs @ 12.21 hrs, Volume= 0.625 af

Outflow 5.8 cfs @ 12.21 hrs, Volume= 0.625 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 6R: Isolated Depression



Wrentham Business Park - BoH Supplement NRCC 24-hr C 100-Year Rainfall=8.80"
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: Watershed EA

Runoff Area=6.580 ac 9.88% Impervious Runoff Depth>1.83"
Flow Length=725' Tc=10.4 min CN=42 Runoff=10.8 cfs 1.003 af

Subcatchment 7S: Watershed EB Runoff Area=2.060 ac 0.00% Impervious Runoff Depth>1.19"

Flow Length=228' Tc=15.0 min CN=36 Runoff=1.4 cfs 0.204 af

Subcatchment 8S: Watershed EC Runoff Area=23.400 ac 30.56% Impervious Runoff Depth>3.79" Flow Length=2,467' Tc=28.2 min CN=59 Runoff=59.2 cfs 7.396 af

Reach 4R: South Property Line Inflow=1.4 cfs 0.204 af
Outflow=1.4 cfs 0.204 af

Reach 5R: Rabbit Hill Brook Inflow=59.2 cfs 7.396 af Outflow=59.2 cfs 7.396 af

Reach 6R: Isolated Depression Inflow=10.8 cfs 1.003 af
Outflow=10.8 cfs 1.003 af

Total Runoff Area = 32.040 ac Runoff Volume = 8.603 af Average Runoff Depth = 3.22"
75.66% Pervious = 24.240 ac 24.34% Impervious = 7.800 ac

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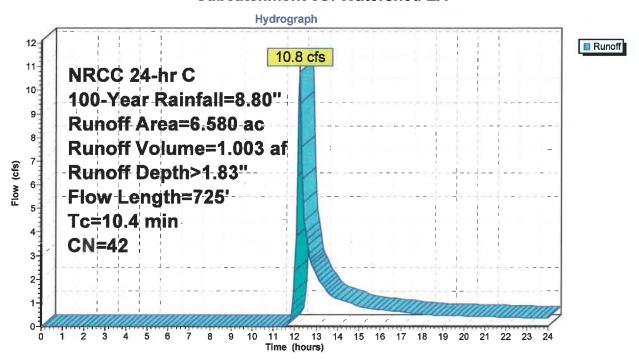
Summary for Subcatchment 3S: Watershed EA

Runoff = 10.8 cfs @ 12.20 hrs, Volume= 1.003 af, Depth> 1.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 100-Year Rainfall=8.80"

	Area	(ac) C	N Des	cription		
	0.	.040 9	98 Roo	fs, HSG A		
	0.	610 9	8 Pave	ed parking	, HSG A	
	5.	.930	36 Woo	ds, Fair, F	ISG A	
	6.	.580 4	12 Weid	hted Aver	age	
		930		2% Pervio		
	0.	650		% Impervi	•	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
•	1.2	50	0.0050	0.69		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 3.20"
	1.1	140	0.0100	2.03		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	8.1	535	0.0480	1.10		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	10 4	725	Total			

Subcatchment 3S: Watershed EA



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Summary for Subcatchment 7S: Watershed EB

Runoff =

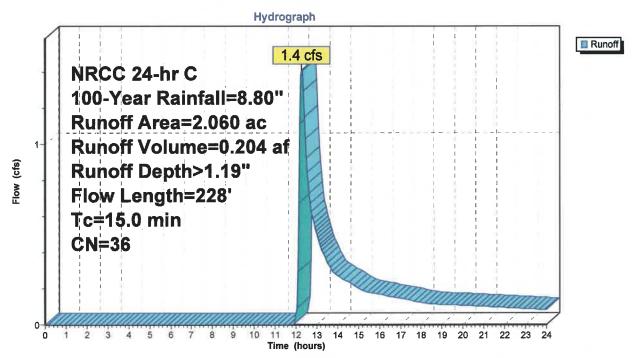
1.4 cfs @ 12.29 hrs, Volume=

0.204 af, Depth> 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 100-Year Rainfall=8.80"

Area	Area (ac) CN Description									
2.060 36 Woods, Fair, HSG A										
2	.060	100.	00% Pervi	ous Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
12.3	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"					
2.7	178	0.0500	1.12		Shallow Concentrated Flow, Woodland Kv= 5.0 fps					
15.0	228	Total								

Subcatchment 7S: Watershed EB



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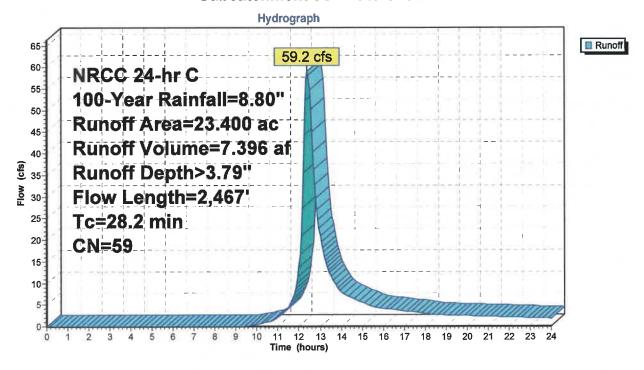
Summary for Subcatchment 8S: Watershed EC

Runoff 59.2 cfs @ 12.41 hrs, Volume= 7.396 af, Depth> 3.79"

	y 191 G	5			
Area	(ac) C		cription		
_		98 Roo	fs, HSG A		
7	.110 9	98 Pave	ed parking	, HSG A	
14	.150 3	36 Woo	ds, Fair, F	ISG A	
2	.100	79 Woo	ds, Fair, F	ISG D	
23	.400 5	59 Weig	hted Aver	age	
	.250	•	4% Pervio		
_	.150			ious Area	
•	. 100	00.0	o 70 miper	71003 71100	
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Boompaon
1.2	50	0.0050	0.69	(0.0)	Sheet Flow,
1.2	50	0.0000	0.09		Smooth surfaces n= 0.011 P2= 3.20"
3.7	E00	0.0170	2.65		
3.7	500	0.0170	2.00		Shallow Concentrated Flow,
0.4	407	0.0400	4.00		Paved Kv= 20.3 fps
8.1	487	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
8.5	542	0.0050	1.06		Shallow Concentrated Flow,
					Grassed Waterway Kv= 15.0 fps
5.2	350	0.0500	1.12		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
1.5	458	0.0040	5.06	91.16	
					Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'
					n= 0.022 Earth, clean & straight
28.2	2,467	Total			
	_, •				

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Subcatchment 8S: Watershed EC



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Summary for Reach 4R: South Property Line

Inflow Area =

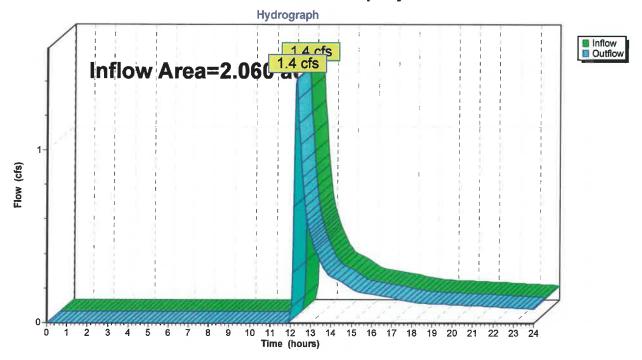
2.060 ac, 0.00% Impervious, Inflow Depth > 1.19" for 100-Year event

Inflow Outflow 1.4 cfs @ 12.29 hrs, Volume= 1.4 cfs @ 12.29 hrs, Volume=

0.204 af 0.204 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 4R: South Property Line



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Summary for Reach 5R: Rabbit Hill Brook

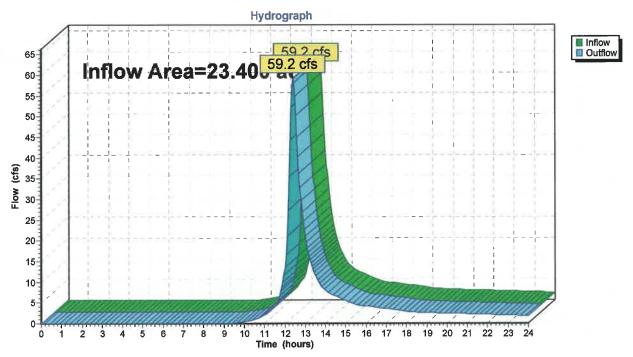
Inflow Area = 23.400 ac, 30.56% Impervious, Inflow Depth > 3.79" for 100-Year event

Inflow = 59.2 cfs @ 12.41 hrs, Volume= 7.396 af

Outflow = 59.2 cfs @ 12.41 hrs, Volume= 7.396 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 5R: Rabbit Hill Brook



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Summary for Reach 6R: Isolated Depression

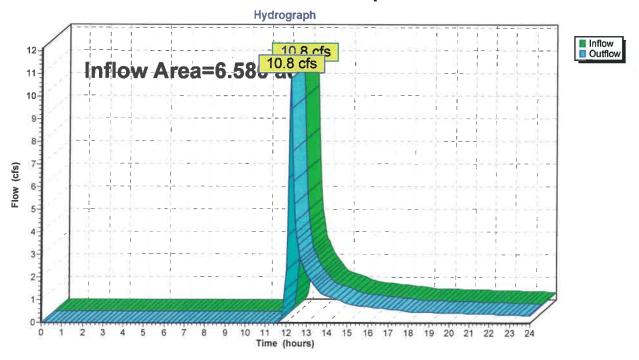
Inflow Area = 6.580 ac, 9.88% Impervious, Inflow Depth > 1.83" for 100-Year event

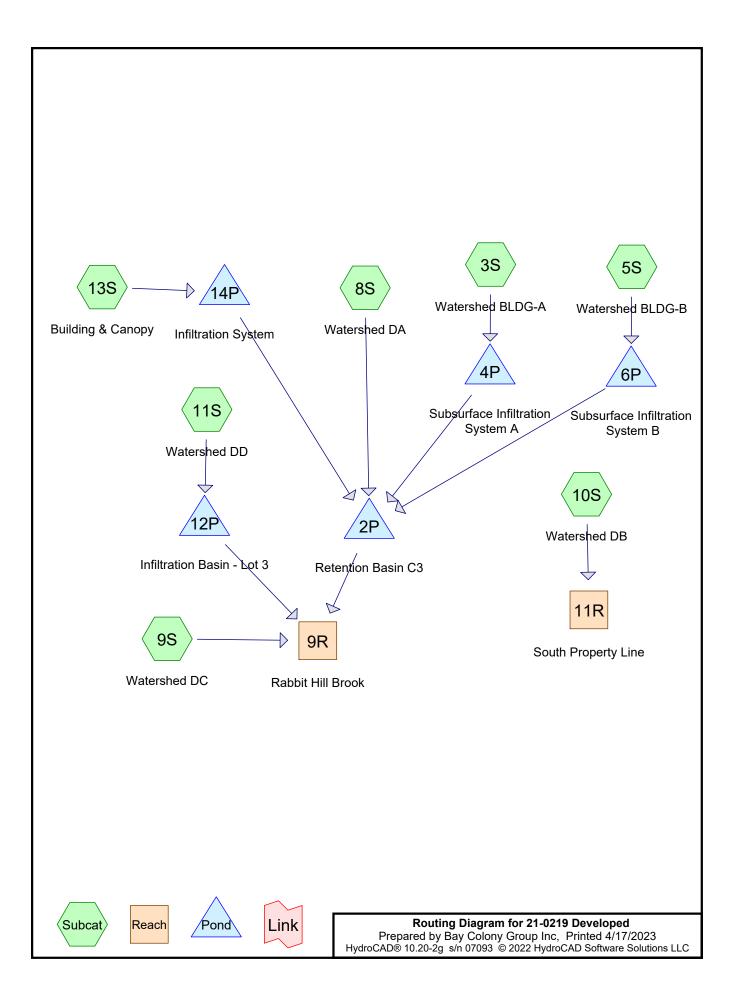
10.8 cfs @ 12.20 hrs, Volume= 10.8 cfs @ 12.20 hrs, Volume= Inflow 1.003 af

Outflow 1.003 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach 6R: Isolated Depression





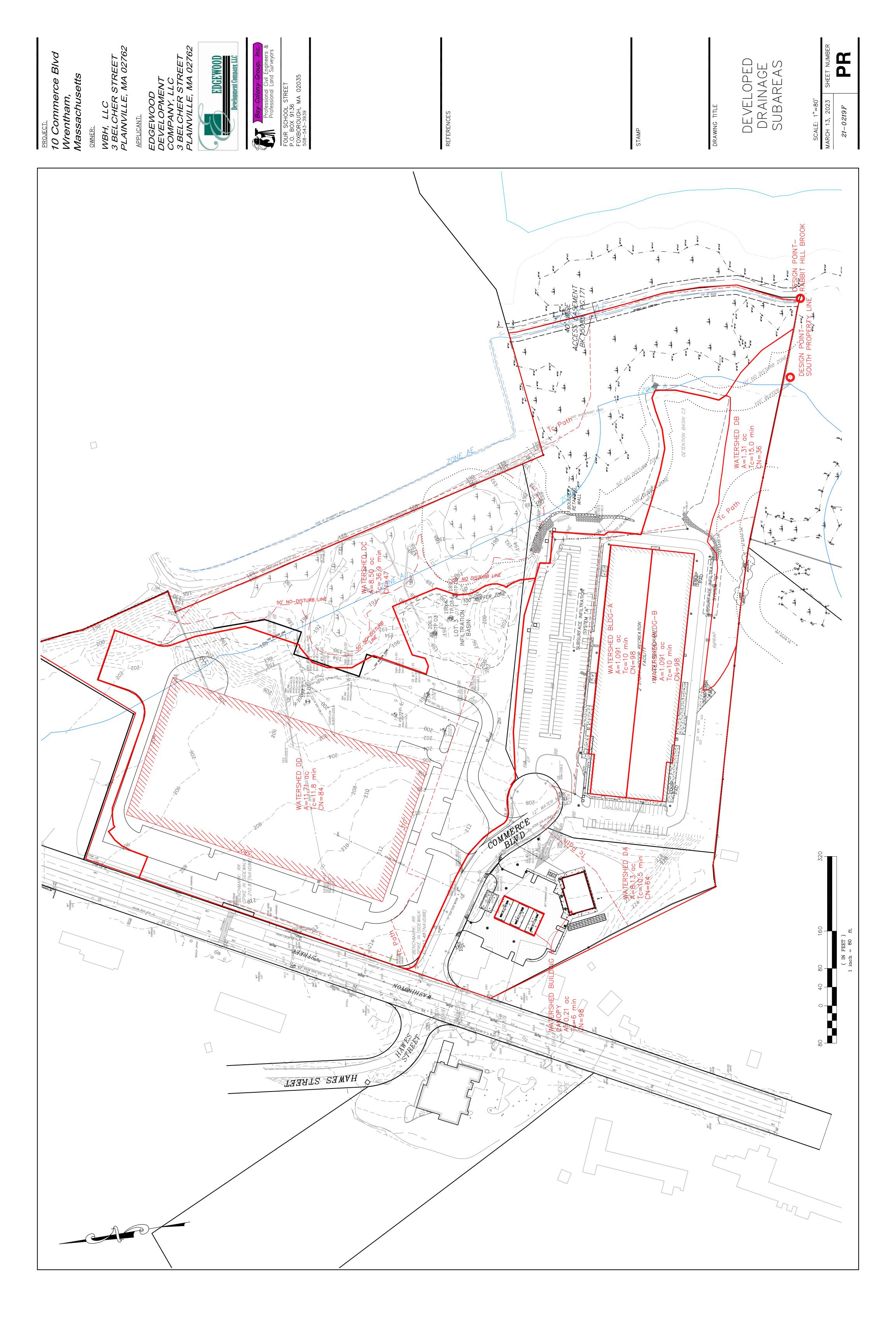
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Rainfall Events Listing (selected events)

_	Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
	1	2-Inch	NRCC 24-hr	С	Default	24.00	1	2.00	2
	2	2-Year	NRCC 24-hr	С	Default	24.00	1	3.22	2
	3	10-Year	NRCC 24-hr	С	Default	24.00	1	4.86	2
	4	50-Year	NRCC 24-hr	С	Default	24.00	1	7.35	2
	5	100-Year	NRCC 24-hr	С	Default	24.00	1	8.80	2



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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
5.980	39	>75% Grass cover, Good, HSG A (8S, 9S, 10S, 11S)
6.640	98	Paved parking, HSG A (8S, 11S)
0.690	98	Paved roads w/curbs & sewers, HSG A (8S)
6.512	98	Unconnected roofs, HSG A (3S, 5S, 11S, 13S)
0.440	98	Water Surface, 0% imp, HSG A (8S)
0.610	98	Water Surface, HSG A (11S)
9.070	36	Woods, Fair, HSG A (8S, 9S, 10S)
2.100	79	Woods, Fair, HSG D (9S)
32.042	68	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
29.942	HSG A	3S, 5S, 8S, 9S, 10S, 11S, 13S
0.000	HSG B	
0.000	HSG C	
2.100	HSG D	9S
0.000	Other	
32.042		TOTAL AREA

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Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Width	Diam/Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
1	8S	0.00	0.00	50.0	0.0300	0.013	0.0	12.0	0.0
2	8S	0.00	0.00	745.0	0.0150	0.013	0.0	30.0	0.0
3	11S	0.00	0.00	640.0	0.0100	0.013	0.0	12.0	0.0
4	4P	200.66	199.00	118.0	0.0141	0.013	0.0	12.0	0.0
5	6P	198.60	192.00	95.0	0.0695	0.013	0.0	12.0	0.0
6	14P	214.00	211.58	142.0	0.0170	0.013	0.0	6.0	0.0

Wrentham Business Center Wrentham, MA Phase 3 NRCC 24-hr C 2-Inch Rainfall=2.00"

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: Watershed BLDG-A Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>1.77"

Tc=10.0 min CN=98 Runoff=1.9 cfs 0.161 af

Subcatchment 5S: Watershed BLDG-B Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>1.77"

Tc=10.0 min CN=98 Runoff=1.9 cfs 0.161 af

Subcatchment 8S: Watershed DARunoff Area=8.130 ac 37.52% Impervious Runoff Depth>0.12"

Flow Length=1,075' Tc=10.5 min CN=64 Runoff=0.2 cfs 0.079 af

Subcatchment 9S: Watershed DCRunoff Area=8.500 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=1,792' Tc=36.9 min CN=47 Runoff=0.0 cfs 0.000 af

Subcatchment 10S: Watershed DB Runoff Area=1.310 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=228' Tc=15.0 min CN=36 Runoff=0.0 cfs 0.000 af

Subcatchment 11S: Watershed DD Runoff Area=11.710 ac 76.94% Impervious Runoff Depth>0.74" Flow Length=752' Tc=11.8 min CN=84 Runoff=8.5 cfs 0.723 af

Subcatchment 13S: Building & Canopy Runoff Area=0.210 ac 100.00% Impervious Runoff Depth>1.77"

Tc=10.0 min CN=98 Runoff=0.4 cfs 0.031 af

Reach 9R: Rabbit Hill Brook Inflow=0.0 cfs 0.000 af Outflow=0.0 cfs 0.000 af

Reach 11R: South Property Line Inflow=0.0 cfs 0.000 af Outflow=0.0 cfs 0.000 af

Pond 2P: Retention Basin C3 Peak Elev=186.13' Storage=3,449 cf Inflow=0.2 cfs 0.079 af

 $\label{eq:decomposition} \mbox{Discarded=0.0 cfs} \ \ 0.000 \ \mbox{af} \ \ \mbox{Outflow=0.0 cfs} \ \ 0.000 \ \mbox{af}$

Pond 4P: Subsurface Infiltration System A Peak Elev=198.73' Storage=0.001 af Inflow=1.9 cfs 0.161 af Discarded=1.8 cfs 0.161 af Primary=0.0 cfs 0.000 af Outflow=1.8 cfs 0.161 af

Pond 6P: Subsurface Infiltration System B Peak Elev=195.05' Storage=0.002 af Inflow=1.9 cfs 0.161 af Discarded=1.8 cfs 0.161 af Primary=0.0 cfs 0.000 af Outflow=1.8 cfs 0.161 af

Pond 12P: Infiltration Basin - Lot 3 Peak Elev=191.42' Storage=5,651 cf Inflow=8.5 cfs 0.723 af Discarded=2.9 cfs 0.722 af Primary=0.0 cfs 0.000 af Outflow=2.9 cfs 0.722 af

Pond 14P: Infiltration System

Peak Elev=212.62' Storage=0.001 af Inflow=0.4 cfs 0.031 af

Discarded=0.2 cfs 0.031 af Primary=0.0 cfs 0.000 af Outflow=0.2 cfs 0.031 af

Total Runoff Area = 32.042 ac Runoff Volume = 1.156 af Average Runoff Depth = 0.43" 54.90% Pervious = 17.590 ac 45.10% Impervious = 14.452 ac

Wrentham Business Center Wrentham, MA Phase 3 NRCC 24-hr C 2-Inch Rainfall=2.00"

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Summary for Subcatchment 3S: Watershed BLDG-A

Runoff = 1.9 cfs @ 12.17 hrs, Volume= 0.161 af, Depth> 1.77"

Routed to Pond 4P: Subsurface Infiltration System A

	Area	(ac)	CN	Desc	cription						
	1.	091	98	Unconnected roofs, HSG A							
	1.091 100.00% Impervious Area										
1.091 100.00% Unconnected						nnected					
	_						-				
		Leng		Slope	,	Capacity	Description				
((min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	10.0						Direct Entry,				

Wrentham Business Center Wrentham, MA Phase 3 NRCC 24-hr C 2-Inch Rainfall=2.00"

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Summary for Subcatchment 5S: Watershed BLDG-B

Runoff = 1.9 cfs @ 12.17 hrs, Volume= 0.161 af, Depth> 1.77"

Routed to Pond 6P: Subsurface Infiltration System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-Inch Rainfall=2.00"

_	Area	(ac)	c) CN Description							
	1.091 98 Unconnected roofs, HSG A									
1.091 100.00% Impervious Area										
	1	.091		100.	00% Unco	nnected				
	_									
	Tc	Leng	th		,	Capacity	Description			
_	(min)	(fee	eet) (ft/ft) (ft/sec) (cfs)							
	10.0						Direct Entry			

10.0 Direct Entry,

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Summary for Subcatchment 8S: Watershed DA

Runoff = 0.2 cfs @ 12.57 hrs, Volume= 0.079 af, Depth> 0.12"

Routed to Pond 2P: Retention Basin C3

Area	(ac) C	N Des	cription				
0.	0.690 98 Paved roads w/curbs & sewers, HSG A						
2.360 98 Paved parking, HSG A							
1.030 39 >75% Grass cover, Good, HSG A							
0.440 98 Water Surface, 0% imp, HSG A							
1.890 36 Woods, Fair, HSG A							
1.	720	39 >75°	% Grass co	over, Good	, HSG A		
8.	.130	64 Weig	ghted Aver	age			
	.080	62.4	8% Pervio	us Area			
3.	.050	37.5	2% Imper	/ious Area			
_				_			
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.5	50	0.1000	0.13		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.20"		
2.7	230	0.0400	1.40		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
0.1	50	0.0300	7.86	6.17	Pipe Channel,		
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'		
4.0	7.45	0.0450	40.00	50.04	n= 0.013		
1.2	745	0.0150	10.23	50.24	•		
					30.0" Round Area= 4.9 sf Perim= 7.9' r= 0.63'		
					n= 0.013		
10.5	1,075	Total					

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Summary for Subcatchment 9S: Watershed DC

Runoff = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Routed to Reach 9R : Rabbit Hill Brook

Area	(ac) C	N Desc	cription				
0.430 39 >75% Grass cover, Good,					, HSG A		
_			ds, Fair, H				
	2.100 79 Woods, Fair, HSG D						
			hted Aver				
8.	.500	100.	00% Pervi	ous Area			
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Becompact		
16.3	50	0.0100	0.05		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.20"		
0.6	86	0.2000	2.24		Shallow Concentrated Flow,		
40.0	0.40	0.0050	4.00		Woodland Kv= 5.0 fps		
13.3	848	0.0050	1.06		Shallow Concentrated Flow,		
F 2	250	0.0500	1 10		Grassed Waterway Kv= 15.0 fps		
5.2	350	0.0500	1.12		Shallow Concentrated Flow, Woodland Kv= 5.0 fps		
1.5	458	0.0040	5.06	91.16	Trap/Vee/Rect Channel Flow,		
1.0	400	0.0040	0.00	31.10	Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'		
					n= 0.022		
36.9	1,792	Total					

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Summary for Subcatchment 10S: Watershed DB

Runoff = 0.0 cfs @ 0.00 hrs, Volume=

0.000 af, Depth= 0.00"

Routed to Reach 11R : South Property Line

	Area	(ac) C	N Desc	cription			
	1.210 36 Woods, Fair, HSG A						
_	0.100 39 >75% Grass cover, Good, HSG A						
	1.310 36 Weighted Average						
	1.	310	100.	00% Pervi	ous Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·	
	12.3	50	0.0200	0.07		Sheet Flow,	
						Woods: Light underbrush n= 0.400 P2= 3.20"	
	2.7	178	0.0500	1.12		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	15.0	228	Total	-	-		

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Summary for Subcatchment 11S: Watershed DD

Runoff = 8.5 cfs @ 12.20 hrs, Volume=

0.723 af, Depth> 0.74"

Routed to Pond 12P: Infiltration Basin - Lot 3

Area	(ac) C	N Des	Description					
2.700 39 >75% Grass cover, Good, HSG A								
4.120 98 Unconnected roofs, HSG A								
4.280 98 Paved parking, HSG A								
0.610 98 Water Surface, HSG A								
11.	710	84 Wei	ghted Aver	age				
	700		6% Pervio					
_	010		•	∕ious Area				
4.	120	45.7	3% Uncon	inected				
To	Longth	Slope	Volocity	Canacity	Description			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
9.0	50	0.0160	0.09	(613)	Sheet Flow,			
9.0	50	0.0100	0.09		Grass: Dense n= 0.240 P2= 3.20"			
0.4	62	0.1300	2.52		Shallow Concentrated Flow,			
0.4	02	0.1000	2.02		Short Grass Pasture Kv= 7.0 fps			
2.4	640	0.0100	4.54	3.56	Pipe Channel,			
	0.0			0.00	12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'			
					n= 0.013			
11.8	752	Total						

Wrentham Business Center Wrentham, MA Phase 3 NRCC 24-hr C 2-Inch Rainfall=2.00"

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Summary for Subcatchment 13S: Building & Canopy

Runoff = 0.4 cfs @ 12.17 hrs, Volume= 0.031 af, Depth> 1.77"

Routed to Pond 14P: Infiltration System

Area ((ac) C	N Desc	cription				
0.2	0.210 98 Unconnected roofs, HSG A						
_	0.210 100.00% Impervious Area 0.210 100.00% Unconnected				a		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.0					Direct Entry,		
6.0	0	Total, Increased to minimum Tc = 10.0 min					

Wrentham Business Center Wrentham, MA Phase 3 NRCC 24-hr C 2-Inch Rainfall=2.00"

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Summary for Reach 9R: Rabbit Hill Brook

Inflow Area = 30.732 ac, 47.03% Impervious, Inflow Depth = 0.00" for 2-Inch event

0.00 hrs, Volume= 0.00 hrs, Volume= 0.000 af Inflow 0.0 cfs @

Outflow 0.0 cfs @ 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Wrentham Business Center Wrentham, MA Phase 3 NRCC 24-hr C 2-Inch Rainfall=2.00"

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Summary for Reach 11R: South Property Line

Inflow Area = 1.310 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Inch event

0.00 hrs, Volume= 0.00 hrs, Volume= 0.000 af Inflow 0.0 cfs @

Outflow 0.000 af, Atten= 0%, Lag= 0.0 min 0.0 cfs @

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Pond 2P: Retention Basin C3

Inflow Area = 10.522 ac, 51.72% Impervious, Inflow Depth > 0.09" for 2-Inch event

Inflow = 0.2 cfs @ 12.57 hrs, Volume= 0.079 af

Outflow = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Discarded = 0.0 cfs @ 0.00 hrs, Volume= 0.000 afPrimary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 186.13' @ 24.00 hrs Surf.Area= 29,292 sf Storage= 3,449 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

#1	186.01'	14	3,139 cf	Custom Stage Data (Irregular)Listed below (Recalc)					
Elevation (feet)	Surf. <i>l</i> (s	Area q-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
186.01	28.	,880	726.0	0	0	28,880			
187.50	34,	,273	765.0	46,992	46,992	33,639			
189.20	39,	,846	811.0	62,942	109,933	39,561			
190.00	43,	,190	1,045.0	33,205	143,139	74,130			

Device	Routing	invert	Outlet Devices
#1	Primary	189.20'	10.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Discarded	188.00'	21.038 in/hr Exfiltration over Surface area above 188.00'
			Conductivity to Groundwater Elevation = 186.00'
			Excluded Surface area = 35,869 sf

Discarded OutFlow Max=0.0 cfs @ 0.00 hrs HW=186.01' (Free Discharge) **2=Exfiltration** (Controls 0.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=186.01' (Free Discharge)
1=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 4P: Subsurface Infiltration System A

Inflow Area = 1.091 ac,100.00% Impervious, Inflow Depth > 1.77" for 2-Inch event
Inflow = 1.9 cfs @ 12.17 hrs, Volume= 0.161 af
Outflow = 1.8 cfs @ 12.18 hrs, Volume= 0.161 af, Atten= 0%, Lag= 0.5 min
Discarded = 1.8 cfs @ 12.18 hrs, Volume= 0.161 af
Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af
Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 198.73' @ 12.18 hrs Surf.Area= 0.081 ac Storage= 0.001 af

Plug-Flow detention time= 0.3 min calculated for 0.161 af (100% of inflow) Center-of-Mass det. time= 0.3 min (773.4 - 773.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	198.70'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	199.20'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	200.66'	0.000 af	1.50'D x 2.84'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	198.70'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	200.66'	12.0" Round Culvert
			L= 118.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 200.66' / 199.00' S= 0.0141 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf
#3	Primary	202.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Discarded OutFlow Max=3.4 cfs @ 12.18 hrs HW=198.72' (Free Discharge) 1=Exfiltration (Controls 3.4 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=198.70' TW=193.50' (Fixed TW Elev= 193.50')

—2=Culvert (Controls 0.0 cfs)

-3=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Pond 4P: Subsurface Infiltration System A - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

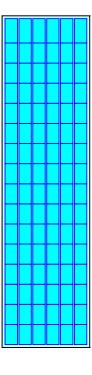
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 6P: Subsurface Infiltration System B

Inflow Area = 1.091 ac,100.00% Impervious, Inflow Depth > 1.77" for 2-Inch event
Inflow = 1.9 cfs @ 12.17 hrs, Volume= 0.161 af
Outflow = 1.8 cfs @ 12.18 hrs, Volume= 0.161 af, Atten= 1%, Lag= 0.8 min
Discarded = 1.8 cfs @ 12.18 hrs, Volume= 0.161 af
Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af
Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 195.05' @ 12.18 hrs Surf.Area= 0.081 ac Storage= 0.002 af

Plug-Flow detention time= 0.6 min calculated for 0.161 af (100% of inflow) Center-of-Mass det. time= 0.5 min (773.6 - 773.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	195.00'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	195.50'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	197.00'	0.000 af	1.50'D x 7.00'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	195.00'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	198.60'	12.0" Round Culvert
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 198.60' / 192.00' S= 0.0695 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Discarded OutFlow Max=3.5 cfs @ 12.18 hrs HW=195.05' (Free Discharge) 1=Exfiltration (Controls 3.5 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=195.00' TW=191.00' (Fixed TW Elev= 191.00') **2=Culvert** (Controls 0.0 cfs)

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Pond 6P: Subsurface Infiltration System B - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

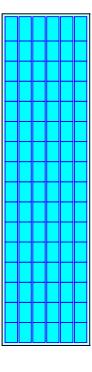
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 12P: Infiltration Basin - Lot 3

Inflow Area = 11.710 ac, 76.94% Impervious, Inflow Depth > 0.74" for 2-Inch event

Inflow = 8.5 cfs @ 12.20 hrs, Volume= 0.723 af

Outflow = 2.9 cfs @ 12.51 hrs, Volume= 0.722 af, Atten= 66%, Lag= 18.3 min

Discarded = 2.9 cfs @ 12.51 hrs, Volume= 0.722 af Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 191.42' @ 12.51 hrs Surf.Area= 13,898 sf Storage= 5,651 cf

Plug-Flow detention time= 13.8 min calculated for 0.722 af (100% of inflow)

Center-of-Mass det. time= 12.6 min (881.7 - 869.1)

Volume	Inver	t Avail.	Storage	Storage Description				
#1	191.00' 145,471		5,471 cf	Custom Stage Data (Irregular)Listed below (Recalc)				
Elevatio		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
191.0	00	13,068	493.0	0	0	13,068		
192.0	00	15,090	518.0	14,067	14,067	15,142		
194.0	00	19,436	568.0	34,434	48,501	19,597		
196.0	00	24,184	619.0	43,534	92,035	24,559		
198.0	00	29,335	669.0	53,436	145,471	29,843		
Device	Routing	Inve	ert Outle	et Devices				
#1	Discarded	191.0	00' 8.27	0 in/hr Exfiltration	over Wetted area	1		
			nductivity to Groundwater Elevation = 187.00' O' long Sharp-Crested Rectangular Weir 2 End Contraction(s)					

Discarded OutFlow Max=2.9 cfs @ 12.51 hrs HW=191.42' (Free Discharge) 1=Exfiltration (Controls 2.9 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=191.00' (Free Discharge) 2=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 14P: Infiltration System

Inflow Area = 0.210 ac,100.00% Impervious, Inflow Depth > 1.77" for 2-Inch event Inflow = 0.4 cfs @ 12.17 hrs, Volume= 0.031 af Outflow = 0.2 cfs @ 12.27 hrs, Volume= 0.031 af, Atten= 31%, Lag= 5.8 min Discarded = 0.0 cfs @ 12.27 hrs, Volume= 0.031 af Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 212.62' @ 12.27 hrs Surf.Area= 0.029 ac Storage= 0.001 af

Plug-Flow detention time= 1.3 min calculated for 0.031 af (100% of inflow) Center-of-Mass det. time= 1.2 min (774.3 - 773.1)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1A	212.50'	0.021 af	14.75'W x 85.29'L x 2.71'H Infiltration System
			0.078 af Overall - 0.026 af Embedded = 0.052 af x 40.0% Voids
#2A	213.00'	0.026 af	Cultec R-180 x 52 Inside #1
			Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf
			Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap
			Row Length Adjustment= +1.00' x 3.44 sf x 4 rows
		0.047 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.50'	8.270 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 192.90'
#2	Primary	214.00'	6.0" Round Culvert
	•		L= 142.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 214.00' / 211.58' S= 0.0170 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.20 sf

Discarded OutFlow Max=0.2 cfs @ 12.27 hrs HW=212.61' (Free Discharge) 1=Exfiltration (Controls 0.2 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=212.50' (Free Discharge) = 2=Culvert (Controls 0.0 cfs)

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Pond 14P: Infiltration System - Chamber Wizard Infiltration System

Chamber Model = Cultec R-180 (Cultec Recharger® 180HD)

Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap Row Length Adjustment= +1.00' x 3.44 sf x 4 rows

36.0" Wide + 3.0" Spacing = 39.0" C-C Row Spacing

13 Chambers/Row x 6.33' Long +1.00' Row Adjustment = 83.29' Row Length +12.0" End Stone x 2 = 85.29' Base Length

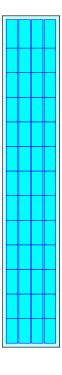
4 Rows x 36.0" Wide + 3.0" Spacing x 3 + 12.0" Side Stone x 2 = 14.75' Base Width 6.0" Stone Base + 20.5" Chamber Height + 6.0" Stone Cover = 2.71' Field Height

52 Chambers x 21.8 cf +1.00' Row Adjustment x 3.44 sf x 4 Rows = 1,145.9 cf Chamber Storage

3,407.2 cf Field - 1,145.9 cf Chambers = 2,261.3 cf Stone x 40.0% Voids = 904.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,050.4 cf = 0.047 af Overall Storage Efficiency = 60.2% Overall System Size = 85.29' x 14.75' x 2.71'

52 Chambers 126.2 cy Field 83.8 cy Stone





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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: Watershed BLDG-A Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>2.98"

Tc=10.0 min CN=98 Runoff=3.0 cfs 0.271 af

Subcatchment 5S: Watershed BLDG-B Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>2.98" Tc=10.0 min CN=98 Runoff=3.0 cfs 0.271 af

Subcatchment 8S: Watershed DA

Runoff Area=8.130 ac 37.52% Impervious Runoff Depth>0.57"
Flow Length=1,075' Tc=10.5 min CN=64 Runoff=3.8 cfs 0.384 af

Subcatchment 9S: Watershed DCRunoff Area=8.500 ac 0.00% Impervious Runoff Depth>0.07"
Flow Length=1,792' Tc=36.9 min CN=47 Runoff=0.1 cfs 0.052 af

Subcatchment 10S: Watershed DB Runoff Area=1.310 ac 0.00% Impervious Runoff Depth=0.00" Flow Length=228' Tc=15.0 min CN=36 Runoff=0.0 cfs 0.000 af

Subcatchment 11S: Watershed DD Runoff Area=11.710 ac 76.94% Impervious Runoff Depth>1.69" Flow Length=752' Tc=11.8 min CN=84 Runoff=19.9 cfs 1.653 af

Subcatchment 13S: Building & Canopy Runoff Area=0.210 ac 100.00% Impervious Runoff Depth>2.98" Tc=10.0 min CN=98 Runoff=0.6 cfs 0.052 af

Reach 9R: Rabbit Hill Brook Inflow=0.1 cfs 0.052 af
Outflow=0.1 cfs 0.052 af

Reach 11R: South Property Line Inflow=0.0 cfs 0.000 af Outflow=0.0 cfs 0.000 af

Pond 2P: Retention Basin C3 Peak Elev=186.57' Storage=16,697 cf Inflow=3.8 cfs 0.384 af

Discarded=0.0 cfs 0.000 af Primary=0.0 cfs 0.000 af Outflow=0.0 cfs 0.000 af

Pond 4P: Subsurface Infiltration System A Peak Elev=198.74' Storage=0.001 af Inflow=3.0 cfs 0.271 af Discarded=3.0 cfs 0.271 af Primary=0.0 cfs 0.000 af Outflow=3.0 cfs 0.271 af

Pond 6P: Subsurface Infiltration System B Peak Elev=195.08' Storage=0.003 af Inflow=3.0 cfs 0.271 af Discarded=3.0 cfs 0.271 af Primary=0.0 cfs 0.000 af Outflow=3.0 cfs 0.271 af

Pond 12P: Infiltration Basin - Lot 3 Peak Elev=192.41' Storage=20,353 cf Inflow=19.9 cfs 1.653 af Discarded=4.0 cfs 1.650 af Primary=0.0 cfs 0.000 af Outflow=4.0 cfs 1.650 af

Pond 14P: Infiltration System

Peak Elev=212.96' Storage=0.005 af Inflow=0.6 cfs 0.052 af Discarded=0.3 cfs 0.052 af Primary=0.0 cfs 0.000 af Outflow=0.3 cfs 0.052 af

Total Runoff Area = 32.042 ac Runoff Volume = 2.683 af Average Runoff Depth = 1.00" 54.90% Pervious = 17.590 ac 45.10% Impervious = 14.452 ac

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Summary for Subcatchment 3S: Watershed BLDG-A

Runoff = 3.0 cfs @ 12.17 hrs, Volume= 0.271 af, Depth> 2.98" Routed to Pond 4P : Subsurface Infiltration System A

Routed to Folid 41 . Odbourhade illilitation bystem /

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

	Area	(ac)	CN	Desc	ription						
	1.	091	98	Unco	Jnconnected roofs, HSG A						
		1.091 100.00% Impervious Area 1.091 100.00% Unconnected					a				
_	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	10.0						Direct Entry				

10.0 Direct Entry,

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Summary for Subcatchment 5S: Watershed BLDG-B

Runoff = 3.0 cfs @ 12.17 hrs, Volume= 0.271 af, Depth> 2.98"

Routed to Pond 6P : Subsurface Infiltration System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

	Area	(ac)	CN	Desc	Description						
	1.	091	91 98 Unconnected roofs, HSG A								
	1.091 100.00% Impervious Area										
1.091 100.00% Unconnected					00% Unco	nnected					
	_					• "	—				
	Tc Length Slope Velocity Capacity				,	Capacity	Description				
	(min)	(feet) (ft/ft) (ft/sec) (cfs)									
	10.0						Direct Entry				

10.0 Direct Entry,

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Summary for Subcatchment 8S: Watershed DA

Runoff = 3.8 cfs @ 12.21 hrs, Volume= 0.384 af, Depth> 0.57"

Routed to Pond 2P: Retention Basin C3

Area	(ac) C	N Des	cription					
0.690 98 Paved roads w/curbs & sewers, HSG A								
2.360 98 Paved parking, HSG A								
1.030 39 >75% Grass cover, Good, HSG A								
0.440 98 Water Surface, 0% imp, HSG A								
			ds, Fair, H					
1.	.720	39 >75°	% Grass co	over, Good	, HSG A			
8.	.130	64 Weig	ghted Aver	age				
	.080	62.4	8% Pervio	us Area				
3.	.050	37.5	2% Imper\	/ious Area				
_				_				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.5	50	0.1000	0.13		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.20"			
2.7	230	0.0400	1.40		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.1	50	0.0300	7.86	6.17	Pipe Channel,			
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'			
4.0	7.45	0.0450	40.00	50.04	n= 0.013			
1.2	745	0.0150	10.23	50.24	•			
					30.0" Round Area= 4.9 sf Perim= 7.9' r= 0.63'			
					n= 0.013			
10.5	1,075	Total						

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Summary for Subcatchment 9S: Watershed DC

Runoff = 0.1 cfs @ 14.81 hrs, Volume= 0.052 af, Depth> 0.07"

Routed to Reach 9R : Rabbit Hill Brook

Area	(ac) C	N Desc	cription							
_				over, Good,	, HSG A					
_	5.970 36 Woods, Fair, HSG A									
	2.100 79 Woods, Fair, HSG D									
			hted Aver							
8.	.500	100.	00% Pervi	ous Area						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	'					
16.3	50	0.0100	0.05		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 3.20"					
0.6	86	0.2000	2.24		Shallow Concentrated Flow,					
40.0	0.40	0.0050	4.00		Woodland Kv= 5.0 fps					
13.3	848	0.0050	1.06		Shallow Concentrated Flow,					
5.2	350	0.0500	1.12		Grassed Waterway Kv= 15.0 fps Shallow Concentrated Flow,					
5.2	330	0.0000	1.12		Woodland Kv= 5.0 fps					
1.5	458	0.0040	5.06	91.16	Trap/Vee/Rect Channel Flow,					
					Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'					
					n= 0.022					
36.9	1,792	Total								

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Summary for Subcatchment 10S: Watershed DB

Runoff = 0.0 cfs @ 0.00 hrs, Volume= 0.0 s

0.000 af, Depth= 0.00"

Routed to Reach 11R: South Property Line

	Area	(ac) C	N Desc	cription		
	1.210 36 Woods, Fair, HSG A					
	0.	100 3	39 >759	% Grass co	over, Good	, HSG A
1.310 36 Weighted Average						
	1.	310	100.	00% Pervi	ous Area	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.3	50	0.0200	0.07		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.20"
	2.7	178	0.0500	1.12		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	15.0	228	Total			

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Summary for Subcatchment 11S: Watershed DD

Runoff = 19.9 cfs @ 12.20 hrs, Volume=

1.653 af, Depth> 1.69"

Routed to Pond 12P: Infiltration Basin - Lot 3

Area	(ac) C	N Desc	cription						
2	.700 3	9 >75% Grass cover, Good, HSG A							
4.	.120	98 Unco	onnected r	oofs, HSG	A				
4.	.280 9	98 Pave	ed parking	, HSG A					
0	.610	98 Wate	er Surface	, HSG A					
11.	.710 8	34 Weig	ghted Aver	age					
2.	.700	23.0	6% Pervio	us Area					
9.	.010	76.9	4% Imper	/ious Area					
4.	.120	45.7	3% Uncon	nected					
_									
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
9.0	50	0.0160	0.09		Sheet Flow,				
					Grass: Dense n= 0.240 P2= 3.20"				
0.4	62	0.1300	2.52		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
2.4	640	0.0100	4.54	3.56	Pipe Channel,				
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'				
					n= 0.013				
11.8	752	Total							

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Summary for Subcatchment 13S: Building & Canopy

0.6 cfs @ 12.17 hrs, Volume= 0.052 af, Depth> 2.98" Runoff

Routed to Pond 14P: Infiltration System

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

Area	(ac)	CN Des	cription			
0.210 98 Unconnected roofs, HSG A						
0.210 100.00% Impervious 0.210 100.00% Unconnected					a	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	•	
6.0					Direct Entry,	
6.0	0	Total I	ncreased t	o minimum	n Tc = 10.0 min	

Total, increased to minimum Tc = 10.0 min

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Summary for Reach 9R: Rabbit Hill Brook

Inflow Area = 30.732 ac, 47.03% Impervious, Inflow Depth > 0.02" for 2-Year event

0.1 cfs @ 14.81 hrs, Volume= 0.1 cfs @ 14.81 hrs, Volume= Inflow 0.052 af

Outflow 0.052 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Reach 11R: South Property Line

Inflow Area = 1.310 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-Year event

0.00 hrs, Volume= 0.00 hrs, Volume= 0.000 af Inflow 0.0 cfs @

Outflow 0.000 af, Atten= 0%, Lag= 0.0 min 0.0 cfs @

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Pond 2P: Retention Basin C3

Inflow Area = 10.522 ac, 51.72% Impervious, Inflow Depth > 0.44" for 2-Year event

Inflow = 3.8 cfs @ 12.21 hrs, Volume= 0.384 af

Outflow = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Discarded = 0.0 cfs @ 0.00 hrs, Volume= 0.000 afPrimary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R : Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 186.57' @ 24.00 hrs Surf.Area= 30,850 sf Storage= 16,697 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.	Storage	Storage Description	1	
#1	186.01'	143,139 cf		Custom Stage Dat	a (Irregular)Liste	d below (Recalc)
Elevation	Surf.A	Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(feet)	(s	q-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)
186.01	28,	,880	726.0	0	0	28,880
187.50	34,	,273	765.0	46,992	46,992	33,639
189.20	39,	846	811.0	62,942	109,933	39,561
190.00	43,	,190	1,045.0	33,205	143,139	74,130

Device	Routing	Invert	Outlet Devices
#1	Primary	189.20'	10.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#2	Discarded	188.00'	21.038 in/hr Exfiltration over Surface area above 188.00'
			Conductivity to Groundwater Elevation = 186.00'
			Excluded Surface area = 35,869 sf

Discarded OutFlow Max=0.0 cfs @ 0.00 hrs HW=186.01' (Free Discharge) **2=Exfiltration** (Controls 0.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=186.01' (Free Discharge)
1=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 4P: Subsurface Infiltration System A

Inflow Area = 1.091 ac,100.00% Impervious, Inflow Depth > 2.98" for 2-Year event
Inflow = 3.0 cfs @ 12.17 hrs, Volume= 0.271 af
Outflow = 3.0 cfs @ 12.18 hrs, Volume= 0.271 af, Atten= 0%, Lag= 0.5 min
Discarded = 3.0 cfs @ 12.18 hrs, Volume= 0.271 af
Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af
Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 198.74' @ 12.18 hrs Surf.Area= 0.081 ac Storage= 0.001 af

Plug-Flow detention time= 0.3 min calculated for 0.271 af (100% of inflow) Center-of-Mass det. time= 0.3 min (761.2 - 760.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	198.70'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	199.20'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	200.66'	0.000 af	1.50'D x 2.84'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	198.70'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	200.66'	12.0" Round Culvert
			L= 118.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 200.66' / 199.00' S= 0.0141 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf
#3	Primary	202.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Discarded OutFlow Max=3.5 cfs @ 12.18 hrs HW=198.74' (Free Discharge) 1=Exfiltration (Controls 3.5 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=198.70' TW=193.50' (Fixed TW Elev= 193.50')

2=Culvert (Controls 0.0 cfs)

-3=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Pond 4P: Subsurface Infiltration System A - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

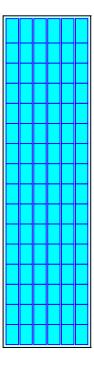
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 6P: Subsurface Infiltration System B

Inflow Area = 1.091 ac,100.00% Impervious, Inflow Depth > 2.98" for 2-Year event
Inflow = 3.0 cfs @ 12.17 hrs, Volume= 0.271 af
Outflow = 3.0 cfs @ 12.18 hrs, Volume= 0.271 af, Atten= 1%, Lag= 0.8 min
Discarded = 3.0 cfs @ 12.18 hrs, Volume= 0.271 af
Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af
Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 195.08' @ 12.18 hrs Surf.Area= 0.081 ac Storage= 0.003 af

Plug-Flow detention time= 0.6 min calculated for 0.271 af (100% of inflow) Center-of-Mass det. time= 0.5 min (761.4 - 760.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	195.00'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	195.50'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	197.00'	0.000 af	1.50'D x 7.00'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	195.00'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	198.60'	12.0" Round Culvert
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 198.60' / 192.00' S= 0.0695 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Discarded OutFlow Max=3.5 cfs @ 12.18 hrs HW=195.08' (Free Discharge) 1=Exfiltration (Controls 3.5 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=195.00' TW=191.00' (Fixed TW Elev= 191.00') **2=Culvert** (Controls 0.0 cfs)

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Pond 6P: Subsurface Infiltration System B - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

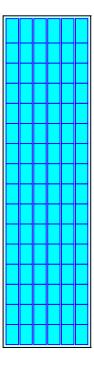
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 12P: Infiltration Basin - Lot 3

Inflow Area = 11.710 ac, 76.94% Impervious, Inflow Depth > 1.69" for 2-Year event

Inflow = 19.9 cfs @ 12.20 hrs, Volume= 1.653 af

Outflow = 4.0 cfs @ 12.72 hrs, Volume= 1.650 af, Atten= 80%, Lag= 31.3 min

Discarded = 4.0 cfs @ 12.72 hrs, Volume= 1.650 af Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 192.41' @ 12.72 hrs Surf.Area= 15,926 sf Storage= 20,353 cf

Plug-Flow detention time= 40.3 min calculated for 1.650 af (100% of inflow)

Center-of-Mass det. time= 39.3 min (882.1 - 842.8)

<u>Volume</u>	Inve	<u>rt Avail.S</u>	Storage	Storage Descriptio	n		
#1	191.00)' 145	,471 cf	Custom Stage Da	ta (Irregular)Listed	d below (Recalc)	
Elevation (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
191.0	00	13,068	493.0	0	0	13,068	
192.0	00	15,090	518.0	14,067	14,067	15,142	
194.0	00	19,436	568.0	34,434	48,501	19,597	
196.0	00	24,184	619.0	43,534	92,035	24,559	
198.0	00	29,335	669.0	53,436	145,471	29,843	
Device	Routing	Inve	rt Outle	et Devices			
#1	Discarded	191.00	0' 8.27	0 in/hr Exfiltration	over Wetted area		
#2	Primary	197.0		ductivity to Groundw ' long Sharp-Crest		87.00' /eir 2 End Contractio	n(s)

Discarded OutFlow Max=4.0 cfs @ 12.72 hrs HW=192.40' (Free Discharge) **1=Exfiltration** (Controls 4.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=191.00' (Free Discharge) 2=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 14P: Infiltration System

Inflow Area = 0.210 ac,100.00% Impervious, Inflow Depth > 2.98" for 2-Year event
Inflow = 0.6 cfs @ 12.17 hrs, Volume= 0.052 af
Outflow = 0.3 cfs @ 12.34 hrs, Volume= 0.052 af, Atten= 55%, Lag= 10.5 min
Discarded = 0.3 cfs @ 12.34 hrs, Volume= 0.052 af
Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af
Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 212.96' @ 12.34 hrs Surf.Area= 0.029 ac Storage= 0.005 af

Plug-Flow detention time= 3.6 min calculated for 0.052 af (100% of inflow) Center-of-Mass det. time= 3.5 min (764.4 - 760.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	212.50'	0.021 af	14.75'W x 85.29'L x 2.71'H Infiltration System
			0.078 af Overall - 0.026 af Embedded = 0.052 af x 40.0% Voids
#2A	213.00'	0.026 af	Cultec R-180 x 52 Inside #1
			Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf
			Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap
			Row Length Adjustment= +1.00' x 3.44 sf x 4 rows
		0.047 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.50'	8.270 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 192.90'
#2	Primary	214.00'	6.0" Round Culvert
			L= 142.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 214.00' / 211.58' S= 0.0170 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.20 sf

Discarded OutFlow Max=0.3 cfs @ 12.34 hrs HW=212.96' (Free Discharge) 1=Exfiltration (Controls 0.3 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=212.50' (Free Discharge) = 2=Culvert (Controls 0.0 cfs)

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Pond 14P: Infiltration System - Chamber Wizard Infiltration System

Chamber Model = Cultec R-180 (Cultec Recharger® 180HD)

Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap Row Length Adjustment= +1.00' x 3.44 sf x 4 rows

36.0" Wide + 3.0" Spacing = 39.0" C-C Row Spacing

13 Chambers/Row x 6.33' Long +1.00' Row Adjustment = 83.29' Row Length +12.0" End Stone x 2 = 85.29' Base Length

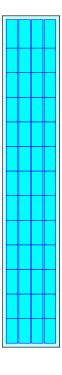
4 Rows x 36.0" Wide + 3.0" Spacing x 3 + 12.0" Side Stone x 2 = 14.75' Base Width 6.0" Stone Base + 20.5" Chamber Height + 6.0" Stone Cover = 2.71' Field Height

52 Chambers x 21.8 cf +1.00' Row Adjustment x 3.44 sf x 4 Rows = 1,145.9 cf Chamber Storage

3,407.2 cf Field - 1,145.9 cf Chambers = 2,261.3 cf Stone x 40.0% Voids = 904.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,050.4 cf = 0.047 af Overall Storage Efficiency = 60.2% Overall System Size = 85.29' x 14.75' x 2.71'

52 Chambers 126.2 cy Field 83.8 cy Stone





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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: Watershed BLDG-A Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>4.62"

Tc=10.0 min CN=98 Runoff=4.6 cfs 0.420 af

Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>4.62" Subcatchment 5S: Watershed BLDG-B

Tc=10.0 min CN=98 Runoff=4.6 cfs 0.420 af

Subcatchment 8S: Watershed DA Runoff Area=8.130 ac 37.52% Impervious Runoff Depth>1.49"

Flow Length=1,075' Tc=10.5 min CN=64 Runoff=12.0 cfs 1.007 af

Subcatchment 9S: Watershed DC Runoff Area=8.500 ac 0.00% Impervious Runoff Depth>0.48"

Flow Length=1,792' Tc=36.9 min CN=47 Runoff=1.2 cfs 0.339 af

Subcatchment 10S: Watershed DB Runoff Area=1.310 ac 0.00% Impervious Runoff Depth>0.09"

Flow Length=228' Tc=15.0 min CN=36 Runoff=0.0 cfs 0.010 af

Runoff Area=11.710 ac 76.94% Impervious Runoff Depth>3.13" Subcatchment 11S: Watershed DD

Flow Length=752' Tc=11.8 min CN=84 Runoff=36.4 cfs 3.059 af

Runoff Area=0.210 ac 100.00% Impervious Runoff Depth>4.62" Subcatchment 13S: Building & Canopy

Tc=10.0 min CN=98 Runoff=0.9 cfs 0.081 af

Inflow=1.2 cfs 0.339 af Reach 9R: Rabbit Hill Brook Outflow=1.2 cfs 0.339 af

Inflow=0.0 cfs 0.010 af Reach 11R: South Property Line

Outflow=0.0 cfs 0.010 af

Pond 2P: Retention Basin C3 Peak Elev=187.41' Storage=43,818 cf Inflow=12.0 cfs 1.007 af

Discarded=0.0 cfs 0.000 af Primary=0.0 cfs 0.000 af Outflow=0.0 cfs 0.000 af

Peak Elev=199.00' Storage=0.010 af Inflow=4.6 cfs 0.420 af Pond 4P: Subsurface Infiltration System A

Discarded=3.6 cfs 0.420 af Primary=0.0 cfs 0.000 af Outflow=3.6 cfs 0.420 af

Pond 6P: Subsurface Infiltration System B Peak Elev=195.32' Storage=0.010 af Inflow=4.6 cfs 0.420 af

Discarded=3.6 cfs 0.420 af Primary=0.0 cfs 0.000 af Outflow=3.6 cfs 0.420 af

Pond 12P: Infiltration Basin - Lot 3 Peak Elev=193.86' Storage=45,743 cf Inflow=36.4 cfs 3.059 af

Discarded=5.8 cfs 3.054 af Primary=0.0 cfs 0.000 af Outflow=5.8 cfs 3.054 af

Pond 14P: Infiltration System Peak Elev=213.28' Storage=0.012 af Inflow=0.9 cfs 0.081 af

Discarded=0.3 cfs 0.081 af Primary=0.0 cfs 0.000 af Outflow=0.3 cfs 0.081 af

Total Runoff Area = 32.042 ac Runoff Volume = 5.334 af Average Runoff Depth = 2.00" 54.90% Pervious = 17.590 ac 45.10% Impervious = 14.452 ac

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Summary for Subcatchment 3S: Watershed BLDG-A

Runoff = 4.6 cfs @ 12.17 hrs, Volume= 0.420 af, Depth> 4.62"

Routed to Pond 4P: Subsurface Infiltration System A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

	Area	(ac)	CN	Desc	cription						
	1	091 98 Unconnected roofs, HSG A									
	1	.091		100.0	00% Impe	rvious Area	a				
	1.091 100.00% Unconnected										
	_										
	Tc	Leng	th		,	Capacity	Description				
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	10.0					•	Direct Entry				

10.0 Direct Entry,

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Summary for Subcatchment 5S: Watershed BLDG-B

Runoff = 4.6 cfs @ 12.17 hrs, Volume= 0.420 af, Depth> 4.62" Routed to Pond 6P : Subsurface Infiltration System B

Runoff by SCS TR-20 method TIH=SCS Weighted-CN Time Span= 0.00-24.00 brs

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

Area	(ac)	CN	Desc	ription		
1.	.091	98	Unco	nnected re	oofs, HSG	A
	.091 .091			00% Imper 00% Unco	vious Area nnected	a
Tc (min)	Lengt (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0						Direct Entry

0.0 Direct Entry,

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Summary for Subcatchment 8S: Watershed DA

Runoff = 12.0 cfs @ 12.19 hrs, Volume= 1.007 af, Depth> 1.49"

Routed to Pond 2P: Retention Basin C3

Area	(ac) C	N Des	cription		
0.	690	98 Pave	ed roads w	/curbs & se	ewers, HSG A
2.	360	98 Pave	ed parking	, HSG A	
1.	030	39 >759	% Grass co	over, Good	, HSG A
0.	440	98 Wate	er Surface	, 0% imp, H	ISG A
1.	890	36 Woo	ds, Fair, H	ISG A	
1.	720	39 >75°	% Grass co	over, Good,	, HSG A
8.	130	64 Weig	ghted Aver	age	
5.	080	62.4	8% Pervio	us Area	
3.	050	37.5	2% Imperv	/ious Area	
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.5	50	0.1000	0.13		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.20"
2.7	230	0.0400	1.40		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.1	50	0.0300	7.86	6.17	Pipe Channel,
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013
1.2	745	0.0150	10.23	50.24	•
					30.0" Round Area= 4.9 sf Perim= 7.9' r= 0.63'
					n= 0.013
10.5	1,075	Total			

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Summary for Subcatchment 9S: Watershed DC

Runoff = 1.2 cfs @ 12.73 hrs, Volume= 0.339 af, Depth> 0.48"

Routed to Reach 9R : Rabbit Hill Brook

Area	(ac) C	N Desc	cription		
0.	.430 3			over, Good,	, HSG A
			ds, Fair, H		
			ds, Fair, H		
			hted Aver		
8.	.500	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.3	50	0.0100	0.05	(0.0)	Sheet Flow,
10.0	00	0.0100	0.00		Woods: Light underbrush n= 0.400 P2= 3.20"
0.6	86	0.2000	2.24		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
13.3	848	0.0050	1.06		Shallow Concentrated Flow,
	0.50	0.0500	4.40		Grassed Waterway Kv= 15.0 fps
5.2	350	0.0500	1.12		Shallow Concentrated Flow,
1.5	150	0.0040	E 06	01 16	Woodland Kv= 5.0 fps
1.5	458	0.0040	5.06	91.16	Trap/Vee/Rect Channel Flow, Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'
					n= 0.022
36.9	1,792	Total			

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Summary for Subcatchment 10S: Watershed DB

Runoff = 0.0 cfs @ 16.39 hrs, Volume=

0.010 af, Depth> 0.09"

Routed to Reach 11R : South Property Line

	Area	(ac) C	N Desc	cription		
				ds, Fair, H		
_	0.	100 3	39 >75°	<u>% Grass co</u>	over, Good	, HSG A
	1.	310 3	36 Weig	ghted Aver	age	
	1.	310	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.3	50	0.0200	0.07		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.20"
	2.7	178	0.0500	1.12		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	15.0	228	Total	-	-	

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Runoff 36.4 cfs @ 12.20 hrs, Volume= 3.059 af, Depth> 3.13"

Routed to Pond 12P: Infiltration Basin - Lot 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

Summary for Subcatchment 11S: Watershed DD

Area	(ac) C	N Desc	cription		
2	.700 3	39 >759	% Grass co	over, Good	, HSG A
4.	.120	98 Unco	onnected r	oofs, HSG	A
4.	.280 9	98 Pave	ed parking	, HSG A	
0	.610	98 Wate	er Surface	, HSG A	
11.	.710 8	34 Weig	ghted Aver	age	
2.	.700	23.0	6% Pervio	us Area	
9.	.010	76.9	4% Imper	/ious Area	
4.	.120	45.7	3% Uncon	nected	
_					
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.0	50	0.0160	0.09		Sheet Flow,
					Grass: Dense n= 0.240 P2= 3.20"
0.4	62	0.1300	2.52		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
2.4	640	0.0100	4.54	3.56	Pipe Channel,
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013
11.8	752	Total			

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Summary for Subcatchment 13S: Building & Canopy

Runoff = 0.9 cfs @ 12.17 hrs, Volume= 0.081 af, Depth> 4.62"

Routed to Pond 14P : Infiltration System

Area	(ac) C	N Des	cription			
0	210 9	8 Unc	onnected r	oofs, HSG	A	
_	210 210		00% Impe 00% Unco	rvious Area nnected	a	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.0					Direct Entry,	
6.0	0	Total, I	ncreased t	o minimum	n Tc = 10.0 min	

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Summary for Reach 9R: Rabbit Hill Brook

Inflow Area = 30.732 ac, 47.03% Impervious, Inflow Depth > 0.13" for 10-Year event

1.2 cfs @ 12.73 hrs, Volume= 1.2 cfs @ 12.73 hrs, Volume= Inflow 0.339 af

Outflow 0.339 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Reach 11R: South Property Line

Inflow Area = 1.310 ac, 0.00% Impervious, Inflow Depth > 0.09" for 10-Year event

0.0 cfs @ 16.39 hrs, Volume= 0.0 cfs @ 16.39 hrs, Volume= 0.010 af Inflow

Outflow 0.010 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Pond 2P: Retention Basin C3

10.522 ac, 51.72% Impervious, Inflow Depth > 1.15" for 10-Year event Inflow Area =

Inflow 12.0 cfs @ 12.19 hrs, Volume= 1.007 af

0.00 hrs, Volume= Outflow 0.000 af, Atten= 100%, Lag= 0.0 min 0.0 cfs @

Discarded = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Primary 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 187.41' @ 24.00 hrs Surf.Area= 33,923 sf Storage= 43,818 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail	.Storage	Storage Description	on		
#1	186.01'	14	13,139 cf	Custom Stage Da	ata (Irregular)List	ed below (Recalc)	
Elevation (feet)		f.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
186.01	2	8,880	726.0	0	0	28,880	
187.50	3	4,273	765.0	46,992	46,992	33,639	
189.20	3	9,846	811.0	62,942	109,933	39,561	
190.00	4	3,190	1,045.0	33,205	143,139	74,130	
Device R	outing	lnv	ert Outle	et Devices			

10.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) #1 Primary 189.20' Discarded #2 188.00' 21.038 in/hr Exfiltration over Surface area above 188.00'

Conductivity to Groundwater Elevation = 186.00' Excluded Surface area = 35,869 sf

Discarded OutFlow Max=0.0 cfs @ 0.00 hrs HW=186.01' (Free Discharge) **2=Exfiltration** (Controls 0.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=186.01' (Free Discharge) -1=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 4P: Subsurface Infiltration System A

1.091 ac,100.00% Impervious, Inflow Depth > 4.62" for 10-Year event Inflow Area = Inflow 4.6 cfs @ 12.17 hrs, Volume= 0.420 af 3.6 cfs @ 12.24 hrs, Volume= 0.420 af, Atten= 22%, Lag= 4.5 min Outflow Discarded = 3.6 cfs @ 12.24 hrs, Volume= 0.420 af Primary 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond 2P: Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 199.00' @ 12.24 hrs Surf.Area= 0.081 ac Storage= 0.010 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 0.5 min (753.0 - 752.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	198.70'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	199.20'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	200.66'	0.000 af	1.50'D x 2.84'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	198.70'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	200.66'	12.0" Round Culvert
			L= 118.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 200.66' / 199.00' S= 0.0141 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf
#3	Primary	202.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Discarded OutFlow Max=3.6 cfs @ 12.24 hrs HW=199.00' (Free Discharge) -1=Exfiltration (Controls 3.6 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=198.70' TW=193.50' (Fixed TW Elev= 193.50') **-2=Culvert** (Controls 0.0 cfs)

-3=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Pond 4P: Subsurface Infiltration System A - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

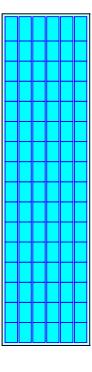
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 6P: Subsurface Infiltration System B

Inflow Area = 1.091 ac,100.00% Impervious, Inflow Depth > 4.62" for 10-Year event Inflow = 4.6 cfs @ 12.17 hrs, Volume= 0.420 af Outflow = 3.6 cfs @ 12.24 hrs, Volume= 0.420 af, Atten= 21%, Lag= 4.4 min Discarded = 3.6 cfs @ 12.24 hrs, Volume= 0.420 af O.000 af Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 195.32' @ 12.24 hrs Surf.Area= 0.081 ac Storage= 0.010 af

Plug-Flow detention time= 0.8 min calculated for 0.420 af (100% of inflow) Center-of-Mass det. time= 0.7 min (753.2 - 752.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	195.00'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	195.50'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	197.00'	0.000 af	1.50'D x 7.00'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	195.00'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	198.60'	12.0" Round Culvert
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 198.60' / 192.00' S= 0.0695 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Discarded OutFlow Max=3.6 cfs @ 12.24 hrs HW=195.31' (Free Discharge) 1=Exfiltration (Controls 3.6 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=195.00' TW=191.00' (Fixed TW Elev= 191.00') **2=Culvert** (Controls 0.0 cfs)

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Pond 6P: Subsurface Infiltration System B - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

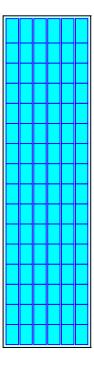
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 12P: Infiltration Basin - Lot 3

Inflow Area = 11.710 ac, 76.94% Impervious, Inflow Depth > 3.13" for 10-Year event

Inflow = 36.4 cfs @ 12.20 hrs, Volume= 3.059 af

Outflow = 5.8 cfs @ 12.87 hrs, Volume= 3.054 af, Atten= 84%, Lag= 40.2 min

Discarded = 5.8 cfs @ 12.87 hrs, Volume= 3.054 af Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 193.86' @ 12.87 hrs Surf.Area= 19,107 sf Storage= 45,743 cf

Plug-Flow detention time= 72.6 min calculated for 3.048 af (100% of inflow)

Aveil Otensons Otensons Description

Center-of-Mass det. time= 71.5 min (895.0 - 823.5)

Volume	Inver	t Avaıl.	.Storage	Storage Description	n		
#1	191.00	' 14	5,471 cf	Custom Stage Da	ta (Irregular)Listed	l below (Recalc)	
Elevation	-	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
191.0	00	13,068	493.0	0	0	13,068	
192.0	00	15,090	518.0	14,067	14,067	15,142	
194.0	00	19,436	568.0	34,434	48,501	19,597	
196.0	00	24,184	619.0	43,534	92,035	24,559	
198.0	00	29,335	669.0	53,436	145,471	29,843	
Device	Routing	Inv	ert Outle	et Devices			
#1	Discarded	191.0		0 in/hr Exfiltration			
#2	Primary	197.0		ductivity to Groundw ' long Sharp-Creste			on(s)
				6			(-)

Discarded OutFlow Max=5.8 cfs @ 12.87 hrs HW=193.86' (Free Discharge) **1=Exfiltration** (Controls 5.8 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=191.00' (Free Discharge) 2=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 14P: Infiltration System

Inflow Area = 0.210 ac,100.00% Impervious, Inflow Depth > 4.62" for 10-Year event
Inflow = 0.9 cfs @ 12.17 hrs, Volume= 0.081 af
Outflow = 0.3 cfs @ 12.42 hrs, Volume= 0.081 af, Atten= 68%, Lag= 15.2 min
Discarded = 0.3 cfs @ 12.42 hrs, Volume= 0.081 af
Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af
Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 213.28' @ 12.42 hrs Surf.Area= 0.029 ac Storage= 0.012 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 8.4 min (760.9 - 752.5)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1A	212.50'	0.021 af	14.75'W x 85.29'L x 2.71'H Infiltration System
			0.078 af Overall - 0.026 af Embedded = 0.052 af x 40.0% Voids
#2A	213.00'	0.026 af	Cultec R-180 x 52 Inside #1
			Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf
			Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap
			Row Length Adjustment= +1.00' x 3.44 sf x 4 rows
		0.047 af	Total Available Storage
		0.0+1 ai	Total / Wallable Otorage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.50'	8.270 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 192.90'
#2	Primary	214.00'	6.0" Round Culvert
			L= 142.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 214.00' / 211.58' S= 0.0170 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.20 sf

Discarded OutFlow Max=0.3 cfs @ 12.42 hrs HW=213.27' (Free Discharge) 1=Exfiltration (Controls 0.3 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=212.50' (Free Discharge) = 2=Culvert (Controls 0.0 cfs)

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Pond 14P: Infiltration System - Chamber Wizard Infiltration System

Chamber Model = Cultec R-180 (Cultec Recharger® 180HD)

Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap Row Length Adjustment= +1.00' x 3.44 sf x 4 rows

36.0" Wide + 3.0" Spacing = 39.0" C-C Row Spacing

13 Chambers/Row x 6.33' Long +1.00' Row Adjustment = 83.29' Row Length +12.0" End Stone x 2 = 85.29' Base Length

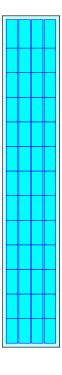
4 Rows x 36.0" Wide + 3.0" Spacing x 3 + 12.0" Side Stone x 2 = 14.75' Base Width 6.0" Stone Base + 20.5" Chamber Height + 6.0" Stone Cover = 2.71' Field Height

52 Chambers x 21.8 cf +1.00' Row Adjustment x 3.44 sf x 4 Rows = 1,145.9 cf Chamber Storage

3,407.2 cf Field - 1,145.9 cf Chambers = 2,261.3 cf Stone x 40.0% Voids = 904.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,050.4 cf = 0.047 af Overall Storage Efficiency = 60.2% Overall System Size = 85.29' x 14.75' x 2.71'

52 Chambers 126.2 cy Field 83.8 cy Stone





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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: Watershed BLDG-A Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>7.10"

Tc=10.0 min CN=98 Runoff=7.0 cfs 0.646 af

Subcatchment 5S: Watershed BLDG-B Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>7.10"

Tc=10.0 min CN=98 Runoff=7.0 cfs 0.646 af

Subcatchment 8S: Watershed DA Runoff Area=8.130 ac 37.52% Impervious Runoff Depth>3.26"

Flow Length=1,075' Tc=10.5 min CN=64 Runoff=27.6 cfs 2.210 af

Subcatchment 9S: Watershed DC Runoff Area=8.500 ac 0.00% Impervious Runoff Depth>1.56"

Flow Length=1,792' Tc=36.9 min CN=47 Runoff=6.5 cfs 1.106 af

Subcatchment 10S: Watershed DB Runoff Area=1.310 ac 0.00% Impervious Runoff Depth>0.66"

Flow Length=228' Tc=15.0 min $\,$ CN=36 $\,$ Runoff=0.3 cfs $\,$ 0.072 af

Subcatchment 11S: Watershed DD Runoff Area=11.710 ac 76.94% Impervious Runoff Depth>5.46"

Flow Length=752' Tc=11.8 min CN=84 Runoff=61.9 cfs 5.329 af

Subcatchment 13S: Building & Canopy Runoff Area=0.210 ac 100.00% Impervious Runoff Depth>7.10"

Tc=10.0 min CN=98 Runoff=1.3 cfs 0.124 af

Reach 9R: Rabbit Hill Brook Inflow=6.5 cfs 1.106 af
Outflow=6.5 cfs 1.106 af

Outilow=0.5 cis 1.100 ai

Reach 11R: South Property Line Inflow=0.3 cfs 0.072 af

Outflow=0.3 cfs 0.072 af

Pond 2P: Retention Basin C3 Peak Elev=188.37' Storage=78,102 cf Inflow=27.6 cfs 2.210 af

Discarded=0.6 cfs 0.427 af Primary=0.0 cfs 0.000 af Outflow=0.6 cfs 0.427 af

Pond 4P: Subsurface Infiltration System A Peak Elev=199.59' Storage=0.043 af Inflow=7.0 cfs 0.646 af

Discarded=3.9 cfs 0.645 af Primary=0.0 cfs 0.000 af Outflow=3.9 cfs 0.645 af

Pond 6P: Subsurface Infiltration System B Peak Elev=195.89' Storage=0.043 af Inflow=7.0 cfs 0.646 af

Discarded=4.0 cfs 0.645 af Primary=0.0 cfs 0.000 af Outflow=4.0 cfs 0.645 af

Pond 12P: Infiltration Basin - Lot 3 Peak Elev=195.91' Storage=89,838 cf Inflow=61.9 cfs 5.329 af

Discarded=8.6 cfs 5.321 af Primary=0.0 cfs 0.000 af Outflow=8.6 cfs 5.321 af

Pond 14P: Infiltration System Peak Elev=213.83' Storage=0.025 af Inflow=1.3 cfs 0.124 af

Discarded=0.3 cfs 0.124 af Primary=0.0 cfs 0.000 af Outflow=0.3 cfs 0.124 af

Total Runoff Area = 32.042 ac Runoff Volume = 10.132 af Average Runoff Depth = 3.79" 54.90% Pervious = 17.590 ac 45.10% Impervious = 14.452 ac

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Summary for Subcatchment 3S: Watershed BLDG-A

Runoff = 7.0 cfs @ 12.17 hrs, Volume= 0.646 af, Depth> 7.10" Routed to Pond 4P : Subsurface Infiltration System A

Area	(ac)	CN	Desc	ription						
1.	.091	98	Unco	nconnected roofs, HSG A						
1.	1.091 100.00% Impervious Area									
1.	1.091 100.00% Unconnected									
Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
10.0						Direct Entry,				

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Summary for Subcatchment 5S: Watershed BLDG-B

Runoff = 7.0 cfs @ 12.17 hrs, Volume= 0.646 af, Depth> 7.10"

Routed to Pond 6P : Subsurface Infiltration System B

Area	(ac)	CN	Desc	ription		
1.	.091	98	Unco	nnected re	oofs, HSG	A
	.091 .091			00% Imper 00% Unco	rvious Area nnected	
Tc (min)	Lengt (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0	·					Direct Entry,

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Summary for Subcatchment 8S: Watershed DA

Runoff = 27.6 cfs @ 12.19 hrs, Volume= 2.210 af, Depth> 3.26"

Routed to Pond 2P: Retention Basin C3

Area	(ac) C	N Des	cription							
0.	0.690 98 Paved roads w/curbs & sewers, HSG A									
2.	2.360 98 Paved parking, HSG A									
1.	1.030 39 >75% Grass cover, Good, HSG A									
0.	440	98 Wate	er Surface	, 0% imp, H	ISG A					
1.	.890	36 Woo	ds, Fair, H	ISG A						
1.	.720	39 >75°	% Grass co	over, Good,	, HSG A					
8.	.130	34 Weig	ghted Aver	age						
5.	.080	62.4	8% Pervio	us Area						
3.	.050	37.5	2% Imperv	/ious Area						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.5	50	0.1000	0.13		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 3.20"					
2.7	230	0.0400	1.40		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
0.1	50	0.0300	7.86	6.17	Pipe Channel,					
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'					
					n= 0.013					
1.2	745	0.0150	10.23	50.24	•					
					30.0" Round Area= 4.9 sf Perim= 7.9' r= 0.63'					
					n= 0.013					
10.5	1,075	Total								

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Summary for Subcatchment 9S: Watershed DC

Runoff = 6.5 cfs @ 12.58 hrs, Volume= 1.106 af, Depth> 1.56"

Routed to Reach 9R: Rabbit Hill Brook

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 50-Year Rainfall=7.35"

0.430 39 >75% Grass cover, Good, HSG A 5.970 36 Woods, Fair, HSG A										
5.970 36 Woods, Fair, HSG A										
, ,										
2.100 79 Woods, Fair, HSG D										
8.500 47 Weighted Average										
8.500 100.00% Pervious Area										
To London Olone Walestine Consisting Description										
Tc Length Slope Velocity Capacity Description										
(min) (feet) (ft/ft) (ft/sec) (cfs)										
16.3 50 0.0100 0.05 Sheet Flow,										
Woods: Light underbrush n= 0.400 P2= 3	5.20"									
0.6 86 0.2000 2.24 Shallow Concentrated Flow,										
Woodland Kv= 5.0 fps										
13.3 848 0.0050 1.06 Shallow Concentrated Flow,										
Grassed Waterway Kv= 15.0 fps										
5.2 350 0.0500 1.12 Shallow Concentrated Flow,										
Woodland Kv= 5.0 fps										
1.5 458 0.0040 5.06 91.16 Trap/Vee/Rect Channel Flow,	001									
Bot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.0	00.									
n= 0.022										

36.9 1,792 Total

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Summary for Subcatchment 10S: Watershed DB

Runoff = 0.3 cfs @ 12.35 hrs, Volume=

0.072 af, Depth> 0.66"

Routed to Reach 11R: South Property Line

Area	(ac) C	N Desc	cription		
1.	210 3	36 Woo	ds, Fair, H	ISG A	
0.	100 3	39 >759	% Grass co	over, Good	, HSG A
1.	310 3	36 Weig	hted Aver	age	
1.	310	100.	00% Pervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.3	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.20"
2.7	178	0.0500	1.12		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
15.0	228	Total			

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Summary for Subcatchment 11S: Watershed DD

Runoff = 61.9 cfs @ 12.19 hrs, Volume=

5.329 af, Depth> 5.46"

Routed to Pond 12P: Infiltration Basin - Lot 3

Area	(ac) C	N Desc	cription		
2	.700 3	39 >759	% Grass co	over, Good	, HSG A
4.	.120	98 Unco	onnected r	oofs, HSG	A
4.	.280 9	8 Pave	ed parking	, HSG A	
0	.610 9	98 Wate	er Surface	, HSG A	
11.	.710 8	34 Weig	ghted Aver	age	
2.	.700		6% Pervio		
_	.010			/ious Area	
4.	.120	45.7	3% Uncon	nected	
_		-			—
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.0	50	0.0160	0.09		Sheet Flow,
					Grass: Dense n= 0.240 P2= 3.20"
0.4	62	0.1300	2.52		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
2.4	640	0.0100	4.54	3.56	Pipe Channel,
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013
11.8	752	Total			

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Summary for Subcatchment 13S: Building & Canopy

Runoff = 1.3 cfs @ 12.17 hrs, Volume= 0.124 af, Depth> 7.10"

Routed to Pond 14P: Infiltration System

Area	(ac)	CN	Desc	ription			
0.	210	98	Unco	nnected re	oofs, HSG	A	
0.	210				rvious Area		
0.	210		100.0	00% Unco	nnected		
Tc (min)	Length (feet		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.0						Direct Entry,	
6.0	() To	otal, Ir	icreased to	o minimum	Tc = 10.0 min	_

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Summary for Reach 9R: Rabbit Hill Brook

Inflow Area = 30.732 ac, 47.03% Impervious, Inflow Depth > 0.43" for 50-Year event

6.5 cfs @ 12.58 hrs, Volume= 6.5 cfs @ 12.58 hrs, Volume= Inflow 1.106 af

Outflow 1.106 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Reach 11R: South Property Line

Inflow Area = 1.310 ac, 0.00% Impervious, Inflow Depth > 0.66" for 50-Year event

0.3 cfs @ 12.35 hrs, Volume= 0.3 cfs @ 12.35 hrs, Volume= Inflow 0.072 af

Outflow 0.072 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Pond 2P: Retention Basin C3

Inflow Area = 10.522 ac, 51.72% Impervious, Inflow Depth > 2.52" for 50-Year event

Inflow = 27.6 cfs @ 12.19 hrs, Volume= 2.210 af

Outflow = 0.6 cfs @ 21.56 hrs, Volume= 0.427 af, Atten= 98%, Lag= 562.6 min

Discarded = 0.6 cfs @ 21.56 hrs, Volume= 0.427 af Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 188.37' @ 21.56 hrs Surf.Area= 37,080 sf Storage= 78,102 cf

Plug-Flow detention time= 488.8 min calculated for 0.427 af (19% of inflow)

Center-of-Mass det. time= 333.7 min (1,189.1 - 855.4)

<u>Volume</u>	Inve	<u>rt Avai</u>	l.Storage	Storage Description	on		
#1	186.0	1' 14	43,139 cf	Custom Stage Da	ata (Irregular) List	ed below (Recalc)	
Elevation		Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>	
186.0	01	28,880	726.0	0	0	28,880	
187.5	50	34,273	765.0	46,992	46,992	33,639	
189.2	20	39,846	811.0	62,942	109,933	39,561	
190.0	00	43,190	1,045.0	33,205	143,139	74,130	
Device	Routing	Inv	vert Outle	et Devices			
#1	Primary	189	.20' 10.0	' long Sharp-Cres	ted Rectangular	Weir 2 End Contraction(s	s)
#2	Discarde	d 188	.00' 21.0 Con	38 in/hr Exfiltratio ductivity to Ground uded Surface area	n over Surface a water Elevation =	rea above 188.00'	,

Discarded OutFlow Max=0.6 cfs @ 21.56 hrs HW=188.37' (Free Discharge) **2=Exfiltration** (Controls 0.6 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=186.01' (Free Discharge)
1=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 4P: Subsurface Infiltration System A

1.091 ac,100.00% Impervious, Inflow Depth > 7.10" for 50-Year event Inflow Area = Inflow 7.0 cfs @ 12.17 hrs, Volume= 0.646 af 3.9 cfs @ 12.30 hrs, Volume= Outflow 0.645 af, Atten= 44%, Lag= 8.1 min Discarded = 3.9 cfs @ 12.30 hrs, Volume= 0.645 af Primary 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond 2P: Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 199.59' @ 12.30 hrs Surf.Area= 0.081 ac Storage= 0.043 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 1.5 min (747.4 - 745.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	198.70'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	199.20'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
<u>#3</u>	200.66'	0.000 af	1.50'D x 2.84'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	198.70'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	200.66'	12.0" Round Culvert
			L= 118.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 200.66' / 199.00' S= 0.0141 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf
#3	Primary	202.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Discarded OutFlow Max=3.9 cfs @ 12.30 hrs HW=199.59' (Free Discharge) -1=Exfiltration (Controls 3.9 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=198.70' TW=193.50' (Fixed TW Elev= 193.50') **-2=Culvert** (Controls 0.0 cfs)

-3=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Pond 4P: Subsurface Infiltration System A - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

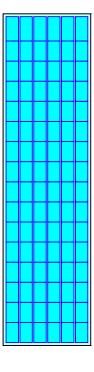
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 6P: Subsurface Infiltration System B

1.091 ac,100.00% Impervious, Inflow Depth > 7.10" for 50-Year event Inflow Area = Inflow 7.0 cfs @ 12.17 hrs, Volume= 0.646 af 4.0 cfs @ 12.30 hrs, Volume= Outflow 0.645 af, Atten= 42%, Lag= 7.8 min Discarded = 4.0 cfs @ 12.30 hrs, Volume= 0.645 af Primary 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond 2P: Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 195.89' @ 12.30 hrs Surf.Area= 0.081 ac Storage= 0.043 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 1.7 min (747.6 - 745.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	195.00'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	195.50'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	197.00'	0.000 af	1.50'D x 7.00'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	195.00'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	198.60'	12.0" Round Culvert
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 198.60' / 192.00' S= 0.0695 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Discarded OutFlow Max=4.0 cfs @ 12.30 hrs HW=195.89' (Free Discharge) 1=Exfiltration (Controls 4.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=195.00' TW=191.00' (Fixed TW Elev= 191.00') 2=Culvert (Controls 0.0 cfs)

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Pond 6P: Subsurface Infiltration System B - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

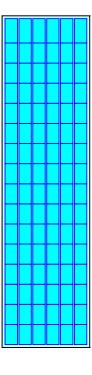
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 12P: Infiltration Basin - Lot 3

Inflow Area = 11.710 ac, 76.94% Impervious, Inflow Depth > 5.46" for 50-Year event

Inflow = 61.9 cfs @ 12.19 hrs, Volume= 5.329 af

Outflow = 8.6 cfs @ 12.96 hrs, Volume= 5.321 af, Atten= 86%, Lag= 46.2 min

Discarded = 8.6 cfs @ 12.96 hrs, Volume= 5.321 af Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Peak Elev= 195.91' @ 12.96 hrs Surf.Area= 23,956 sf Storage= 89,838 cf

Plug-Flow detention time= 107.5 min calculated for 5.321 af (100% of inflow)

Center-of-Mass det. time= 106.6 min (912.8 - 806.2)

<u>Volume</u>	Inve	<u>rt Avail.S</u>	torage	Storage Description	า		
#1	191.00)' 145	471 cf	Custom Stage Dat	ta (Irregular)Listed	l below (Recalc)	
Elevation (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
191.0	00	13,068	493.0	0	0	13,068	
192.0	00	15,090	518.0	14,067	14,067	15,142	
194.0	00	19,436	568.0	34,434	48,501	19,597	
196.0	00	24,184	619.0	43,534	92,035	24,559	
198.0	00	29,335	669.0	53,436	145,471	29,843	
Device	Routing	Inve	t Outle	et Devices			
#1	Discarded	191.00	8.27	0 in/hr Exfiltration o	over Wetted area		
			Cond	ductivity to Groundw	ater Elevation = 18	37.00'	
#2	Primary	197.00)' 10.0	' long Sharp-Creste	ed Rectangular W	eir 2 End Contraction	n(s)

Discarded OutFlow Max=8.6 cfs @ 12.96 hrs HW=195.91' (Free Discharge) 1=Exfiltration (Controls 8.6 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=191.00' (Free Discharge) 2=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 14P: Infiltration System

0.210 ac,100.00% Impervious, Inflow Depth > 7.10" for 50-Year event Inflow Area = Inflow 1.3 cfs @ 12.17 hrs, Volume= 0.124 af 0.3 cfs @ 12.53 hrs, Volume= Outflow 0.124 af, Atten= 77%, Lag= 21.4 min 0.3 cfs @ 12.53 hrs, Volume= 0.124 af Discarded = Primary 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond 2P: Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 213.83' @ 12.53 hrs Surf.Area= 0.029 ac Storage= 0.025 af

Plug-Flow detention time= 18.2 min calculated for 0.124 af (100% of inflow)

Center-of-Mass det. time= 18.1 min (764.0 - 745.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	212.50'	0.021 af	14.75'W x 85.29'L x 2.71'H Infiltration System
			0.078 af Overall - 0.026 af Embedded = 0.052 af x 40.0% Voids
#2A	213.00'	0.026 af	Cultec R-180 x 52 Inside #1
			Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf
			Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap
			Row Length Adjustment= +1.00' x 3.44 sf x 4 rows
	•	0.047 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.50'	8.270 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 192.90'
#2	Primary	214.00'	6.0" Round Culvert
	•		L= 142.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 214.00' / 211.58' S= 0.0170 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.20 sf

Discarded OutFlow Max=0.3 cfs @ 12.53 hrs HW=213.83' (Free Discharge) **1=Exfiltration** (Controls 0.3 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=212.50' (Free Discharge) **2=Culvert** (Controls 0.0 cfs)

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Pond 14P: Infiltration System - Chamber Wizard Infiltration System

Chamber Model = Cultec R-180 (Cultec Recharger® 180HD)

Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap Row Length Adjustment= +1.00' x 3.44 sf x 4 rows

36.0" Wide + 3.0" Spacing = 39.0" C-C Row Spacing

13 Chambers/Row x 6.33' Long +1.00' Row Adjustment = 83.29' Row Length +12.0" End Stone x 2 = 85.29' Base Length

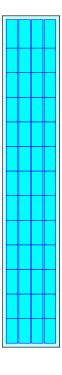
4 Rows x 36.0" Wide + 3.0" Spacing x 3 + 12.0" Side Stone x 2 = 14.75' Base Width 6.0" Stone Base + 20.5" Chamber Height + 6.0" Stone Cover = 2.71' Field Height

52 Chambers x 21.8 cf +1.00' Row Adjustment x 3.44 sf x 4 Rows = 1,145.9 cf Chamber Storage

3,407.2 cf Field - 1,145.9 cf Chambers = 2,261.3 cf Stone x 40.0% Voids = 904.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,050.4 cf = 0.047 af Overall Storage Efficiency = 60.2% Overall System Size = 85.29' x 14.75' x 2.71'

52 Chambers 126.2 cy Field 83.8 cy Stone





Tc=10.0 min CN=98 Runoff=8.4 cfs 0.777 af

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: Watershed BLDG-A Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>8.55"
Tc=10.0 min CN=98 Runoff=8.4 cfs 0.777 af

Subcatchment 5S: Watershed BLDG-B Runoff Area=1.091 ac 100.00% Impervious Runoff Depth>8.55"

Subcatchment8S: Watershed DA

Runoff Area=8.130 ac 37.52% Impervious Runoff Depth>4.42"
Flow Length=1,075' Tc=10.5 min CN=64 Runoff=37.4 cfs 2.993 af

Subcatchment9S: Watershed DC

Runoff Area=8.500 ac 0.00% Impervious Runoff Depth>2.37"
Flow Length=1,792' Tc=36.9 min CN=47 Runoff=10.7 cfs 1.679 af

Subcatchment 10S: Watershed DB Runoff Area=1.310 ac 0.00% Impervious Runoff Depth>1.19" Flow Length=228' Tc=15.0 min CN=36 Runoff=0.9 cfs 0.130 af

Subcatchment 11S: Watershed DD Runoff Area=11.710 ac 76.94% Impervious Runoff Depth>6.85" Flow Length=752' Tc=11.8 min CN=84 Runoff=76.7 cfs 6.685 af

Subcatchment 13S: Building & Canopy Runoff Area=0.210 ac 100.00% Impervious Runoff Depth>8.55"

Tc=10.0 min CN=98 Runoff=1.6 cfs 0.150 af

Reach 9R: Rabbit Hill Brook Inflow=10.7 cfs 1.679 af
Outflow=10.7 cfs 1.679 af

Reach 11R: South Property Line Inflow=0.9 cfs 0.130 af Outflow=0.9 cfs 0.130 af

Pond 2P: Retention Basin C3 Peak Elev=188.72' Storage=91,184 cf Inflow=37.4 cfs 2.995 af Discarded=1.3 cfs 1.057 af Primary=0.0 cfs 0.000 af Outflow=1.3 cfs 1.057 af

Pond 4P: Subsurface Infiltration System A Peak Elev=199.95' Storage=0.068 af Inflow=8.4 cfs 0.777 af Discarded=4.1 cfs 0.777 af Primary=0.0 cfs 0.000 af Outflow=4.1 cfs 0.777 af

Pond 6P: Subsurface Infiltration System B Peak Elev=196.24' Storage=0.067 af Inflow=8.4 cfs 0.777 af Discarded=4.3 cfs 0.777 af Primary=0.0 cfs 0.000 af Outflow=4.3 cfs 0.777 af

Pond 12P: Infiltration Basin - Lot 3 Peak Elev=196.99' Storage=117,299 cf Inflow=76.7 cfs 6.685 af Discarded=10.3 cfs 6.676 af Primary=0.0 cfs 0.000 af Outflow=10.3 cfs 6.676 af

Pond 14P: Infiltration System

Peak Elev=214.18' Storage=0.033 af Inflow=1.6 cfs 0.150 af

Discarded=0.3 cfs 0.147 af Primary=0.1 cfs 0.002 af Outflow=0.4 cfs 0.150 af

Total Runoff Area = 32.042 ac Runoff Volume = 13.191 af Average Runoff Depth = 4.94" 54.90% Pervious = 17.590 ac 45.10% Impervious = 14.452 ac

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Summary for Subcatchment 3S: Watershed BLDG-A

8.4 cfs @ 12.17 hrs, Volume= 0.777 af, Depth> 8.55" Runoff

Routed to Pond 4P: Subsurface Infiltration System A

_	Area	(ac)	CN	Desc	cription			
	1.	.091	98 Unconnected roofs, HSG A					
_	1.091 100.00% Impervious Area				00% Impei	a		
	1.	1.091			00% Unco	nnected		
	Тс	Lengt	th G	Slope	Velocity	Capacity	Description	
	(min)	(fee		(ft/ft)	(ft/sec)	(cfs)	Description	
-	10.0	•					Direct Entry.	

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Summary for Subcatchment 5S: Watershed BLDG-B

Runoff = 8.4 cfs @ 12.17 hrs, Volume= 0.777 af, Depth> 8.55"

Routed to Pond 6P: Subsurface Infiltration System B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs NRCC 24-hr C 100-Year Rainfall=8.80"

	Area	(ac)	CN	Desc	cription		
_	1.	.091	98	Unco	onnected re	oofs, HSG	A
		.091 .091			00% Impei 00% Unco	rvious Area nnected	а
_	Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	10.0						Direct Entry

10.0 Direct Entry,

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Summary for Subcatchment 8S: Watershed DA

Runoff = 37.4 cfs @ 12.18 hrs, Volume= 2.993 af, Depth> 4.42"

Routed to Pond 2P: Retention Basin C3

Area	(ac) C	N Des	cription								
0.	690	98 Pave	ed roads w	/curbs & se	ewers, HSG A						
2.	360	98 Pave	Paved parking, HSG A								
1.	030	39 >759	% Grass co	ver, Good, HSG A							
0.440 98 Water Surface, 0% imp, HSG A 1.890 36 Woods, Fair, HSG A 1.720 39 >75% Grass cover, Good, HSG A											
1.											
1.	, HSG A										
8.	130	64 Weig	ghted Aver	age							
5.	080	62.4	8% Pervio	us Area							
3.	050	37.5	2% Imperv	∕ious Area							
Tc	Length	Slope	Velocity	Capacity	Description						
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)							
6.5	50	0.1000	0.13		Sheet Flow,						
					Woods: Light underbrush n= 0.400 P2= 3.20"						
2.7	230	0.0400	1.40		Shallow Concentrated Flow,						
					Short Grass Pasture Kv= 7.0 fps						
0.1	50	0.0300	7.86	6.17	Pipe Channel,						
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'						
					n= 0.013						
1.2	745	0.0150	10.23	50.24	•						
					30.0" Round Area= 4.9 sf Perim= 7.9' r= 0.63'						
					n= 0.013						
10.5	1,075	Total									

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Summary for Subcatchment 9S: Watershed DC

Runoff = 10.7 cfs @ 12.56 hrs, Volume= 1.679 af, Depth> 2.37"

Routed to Reach 9R: Rabbit Hill Brook

Area	(ac) C	N Desc	cription						
0.			>75% Grass cover, Good, HSG A						
_			ds, Fair, H						
			ds, Fair, F						
			hted Aver		Description Sheet Flow, Voods: Light underbrush n= 0.400 P2= 3.20" Shallow Concentrated Flow, Voodland Kv= 5.0 fps Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps Shallow Concentrated Flow, Voodland Kv= 5.0 fps Shallow Concentrated Flow, Voodland Kv= 5.0 fps Grap/Vee/Rect Channel Flow, Sot.W=5.00' D=2.00' Z= 2.0 '/' Top.W=13.00'				
8.	.500	100.	00% Pervi	ous Area					
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Becompact				
16.3	50	0.0100	0.05		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.20"				
0.6	86	0.2000	2.24		Shallow Concentrated Flow,				
40.0	0.40	0.0050	4.00		•				
13.3	848	0.0050	1.06						
F 2	250	0.0500	1 10						
5.2	350	0.0500	1.12		· · · · · · · · · · · · · · · · · · ·				
1.5	458	0.0040	5.06	91.16	•				
1.0	700	0.0040	3.00	31.10	•				
					n= 0.022				
36.9	1,792	Total							

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Summary for Subcatchment 10S: Watershed DB

Runoff = 0.9 cfs @ 12.29 hrs, Volume=

0.130 af, Depth> 1.19"

Routed to Reach 11R: South Property Line

	Area	(ac) C	N Desc	cription		
	1.	210 3	36 Woo	ds, Fair, H	ISG A	
_	0.	100 3	39 >75°	% Grass co	over, Good	, HSG A
	1.	310 3	36 Weig	ghted Aver	age	
	1.	310	100.	00% Pervi	ous Area	
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.3	50	0.0200	0.07		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.20"
	2.7	178	0.0500	1.12		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	15.0	228	Total			

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Summary for Subcatchment 11S: Watershed DD

Runoff = 76.7 cfs @ 12.19 hrs, Volume=

6.685 af, Depth> 6.85"

Routed to Pond 12P: Infiltration Basin - Lot 3

Area	(ac) CN Description										
2	.700 3	39 >759	% Grass c	over, Good	, HSG A						
4.	.120	98 Unco	onnected r	oofs, HSG	A						
4.	.280 9	98 Pave	Paved parking, HSG A								
0	.610	98 Wate	er Surface	, HSG A							
11.	.710 8	34 Weig	ghted Aver	age							
2.	.700	23.0	6% Pervio	us Area							
9.	.010	76.9	4% Imper	/ious Area							
4.	.120	45.7	3% Uncon	nected							
_											
Tc			Velocity	Capacity	Description						
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)							
9.0	50	0.0160	0.09		Sheet Flow,						
			0.1300 2.52		Grass: Dense n= 0.240 P2= 3.20"						
0.4					Shallow Concentrated Flow,						
					Short Grass Pasture Kv= 7.0 fps						
2.4	640	0.0100	4.54	3.56	Pipe Channel,						
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'						
					n= 0.013						
11.8	752	Total									

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Summary for Subcatchment 13S: Building & Canopy

1.6 cfs @ 12.17 hrs, Volume= 0.150 af, Depth> 8.55" Runoff

Routed to Pond 14P: Infiltration System

_	Area	(ac)	CN	Desc	ription			
	0.	210	98	Unco	nnected r	oofs, HSG	A	
	0.	210		100.0	00% Impe	rvious Area	l	
	0.	210		100.0	00% Unco	nnected		
	Тс	Length	h '	Slope	Velocity	Capacity	Description	
	(min)	(feet		(ft/ft)	(ft/sec)	(cfs)	Description	
	6.0			·			Direct Entry,	
	6.0	(ОТ	otal Ir	creased t	o minimum	$T_{C} = 10.0 \text{ min}$	

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Summary for Reach 9R: Rabbit Hill Brook

Inflow Area = 30.732 ac, 47.03% Impervious, Inflow Depth > 0.66" for 100-Year event

10.7 cfs @ 12.56 hrs, Volume= 10.7 cfs @ 12.56 hrs, Volume= Inflow 1.679 af

Outflow 1.679 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Reach 11R: South Property Line

Inflow Area = 1.310 ac, 0.00% Impervious, Inflow Depth > 1.19" for 100-Year event

0.9 cfs @ 12.29 hrs, Volume= 0.9 cfs @ 12.29 hrs, Volume= Inflow 0.130 af

Outflow 0.130 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

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Summary for Pond 2P: Retention Basin C3

10.522 ac, 51.72% Impervious, Inflow Depth > 3.42" for 100-Year event Inflow Area =

Inflow 37.4 cfs @ 12.18 hrs, Volume= 2.995 af

1.3 cfs @ 16.89 hrs, Volume= Outflow 1.057 af, Atten= 96%, Lag= 282.4 min

Discarded = 1.3 cfs @ 16.89 hrs, Volume= 1.057 af Primary 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 188.72' @ 16.89 hrs Surf.Area= 38,229 sf Storage= 91,184 cf

Plug-Flow detention time= 399.1 min calculated for 1.057 af (35% of inflow)

Center-of-Mass det. time= 260.9 min (1,106.6 - 845.7)

Volume	Invert	Avail	l.Storage	Storage Description	on		
#1	186.01	' 14	43,139 cf	Custom Stage Da	ata (Irregular) List	ed below (Recalc)	
Elevatio		urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	-	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
186.0		28,880	726.0	0	0	28,880	
187.5	U	34,273	765.0	46,992	46,992	33,639	
189.2	0	39,846	811.0	62,942	109,933	39,561	
190.0	0	43,190	1,045.0	33,205	143,139	74,130	
Device	Routing	Inv	ert Outle	et Devices			
#1	Primary	189.	.20' 10.0	long Sharp-Cres	ted Rectangular	Weir 2 End Contra	ction(s)
#2	Discarded	188.		• .	•	rea above 188.00'	` '

Discarded 188.00' 21.038 in/hr Exfiltration over Surface area above 188.00' Conductivity to Groundwater Elevation = 186.00'

Excluded Surface area = 35,869 sf

Discarded OutFlow Max=1.3 cfs @ 16.89 hrs HW=188.72' (Free Discharge) **2=Exfiltration** (Controls 1.3 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=186.01' (Free Discharge) -1=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 4P: Subsurface Infiltration System A

Inflow Area = 1.091 ac,100.00% Impervious, Inflow Depth > 8.55" for 100-Year event Inflow = 8.4 cfs @ 12.17 hrs, Volume= 0.777 af Outflow = 4.1 cfs @ 12.32 hrs, Volume= 0.777 af, Atten= 51%, Lag= 9.3 min Discarded = 4.1 cfs @ 12.32 hrs, Volume= 0.777 af Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 199.95' @ 12.32 hrs Surf.Area= 0.081 ac Storage= 0.068 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 2.4 min (745.9 - 743.5)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1A	198.70'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	199.20'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	200.66'	0.000 af	1.50'D x 2.84'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	198.70'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	200.66'	12.0" Round Culvert
			L= 118.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 200.66' / 199.00' S= 0.0141 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf
#3	Primary	202.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Discarded OutFlow Max=4.1 cfs @ 12.32 hrs HW=199.94' (Free Discharge) 1=Exfiltration (Controls 4.1 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=198.70' TW=193.50' (Fixed TW Elev= 193.50')

—2=Culvert (Controls 0.0 cfs)

-3=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Pond 4P: Subsurface Infiltration System A - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

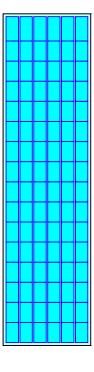
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 6P: Subsurface Infiltration System B

Inflow Area = 1.091 ac,100.00% Impervious, Inflow Depth > 8.55" for 100-Year event Inflow = 8.4 cfs @ 12.17 hrs, Volume= 0.777 af Outflow = 4.3 cfs @ 12.32 hrs, Volume= 0.777 af, Atten= 49%, Lag= 9.1 min Discarded = 4.3 cfs @ 12.32 hrs, Volume= 0.777 af Primary = 0.0 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 196.24' @ 12.32 hrs Surf.Area= 0.081 ac Storage= 0.067 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 2.5 min (746.0 - 743.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	195.00'	0.068 af	30.50'W x 115.50'L x 3.54'H Field A
			0.286 af Overall - 0.116 af Embedded = 0.170 af x 40.0% Voids
#2A	195.50'	0.116 af	Cultec R-330XLHD x 96 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 6 rows
#3	197.00'	0.000 af	1.50'D x 7.00'H Vertical Cone/Cylinder
		0.185 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	195.00'	42.077 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 186.00'
#2	Primary	198.60'	12.0" Round Culvert
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 198.60' / 192.00' S= 0.0695 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Discarded OutFlow Max=4.3 cfs @ 12.32 hrs HW=196.23' (Free Discharge) **1=Exfiltration** (Controls 4.3 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=195.00' TW=191.00' (Fixed TW Elev= 191.00') **2=Culvert** (Controls 0.0 cfs)

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Pond 6P: Subsurface Infiltration System B - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 6 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

16 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 113.50' Row Length +12.0" End Stone x 2 = 115.50' Base Length

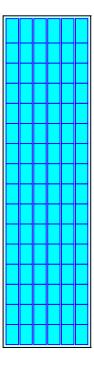
6 Rows x 52.0" Wide + 6.0" Spacing x 5 + 12.0" Side Stone x 2 = 30.50' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

96 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 6 Rows = 5,074.1 cf Chamber Storage

12,476.4 cf Field - 5,074.1 cf Chambers = 7,402.3 cf Stone x 40.0% Voids = 2,960.9 cf Stone Storage

Chamber Storage + Stone Storage = 8,035.0 cf = 0.184 af Overall Storage Efficiency = 64.4% Overall System Size = 115.50' x 30.50' x 3.54'

96 Chambers 462.1 cy Field 274.2 cy Stone



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Summary for Pond 12P: Infiltration Basin - Lot 3

11.710 ac, 76.94% Impervious, Inflow Depth > 6.85" for 100-Year event Inflow Area =

Inflow 76.7 cfs @ 12.19 hrs, Volume= 6.685 af

10.3 cfs @ 12.99 hrs, Volume= Outflow 6.676 af, Atten= 87%, Lag= 48.0 min

10.3 cfs @ 12.99 hrs, Volume= Discarded = 6.676 af Primary 0.0 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Reach 9R: Rabbit Hill Brook

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Peak Elev= 196.99' @ 12.99 hrs Surf.Area= 26,681 sf Storage= 117,299 cf

Plug-Flow detention time= 123.4 min calculated for 6.676 af (100% of inflow)

Center-of-Mass det. time= 122.5 min (921.8 - 799.3)

Volume	Inve	t Avail.	Storage	Storage Description	on		
#1	191.00)' 14	5,471 cf	Custom Stage Da	ata (Irregular)Listo	ed below (Recalc)	
Elevation	on S	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee		(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
191.0	00	13,068	493.0	0	0	13,068	
192.0	00	15,090	518.0	14,067	14,067	15,142	
194.0	00	19,436	568.0	34,434	48,501	19,597	
196.0	00	24,184	619.0	43,534	92,035	24,559	
198.0	00	29,335	669.0	53,436	145,471	29,843	
		_					
Device	Routing	Inve	ert Outle	et Devices			
#1	Discarded	l 191.0	0' 8.27	0 in/hr Exfiltration	over Wetted are	a	
			Cond	ductivity to Ground	water Elevation =	187.00'	
#2	Primary	197.0	0' 10.0	long Sharp-Crest	ted Rectangular \	Weir 2 End Contraction(s)	

Discarded OutFlow Max=10.3 cfs @ 12.99 hrs HW=196.99' (Free Discharge) 1=Exfiltration (Controls 10.3 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=191.00' (Free Discharge) 2=Sharp-Crested Rectangular Weir (Controls 0.0 cfs)

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Summary for Pond 14P: Infiltration System

Inflow Area = 0.210 ac,100.00% Impervious, Inflow Depth > 8.55" for 100-Year event Inflow = 1.6 cfs @ 12.17 hrs, Volume= 0.150 af Outflow = 0.4 cfs @ 12.49 hrs, Volume= 0.150 af, Atten= 75%, Lag= 19.3 min Discarded = 0.3 cfs @ 12.49 hrs, Volume= 0.147 af Primary = 0.1 cfs @ 12.49 hrs, Volume= 0.002 af Routed to Pond 2P : Retention Basin C3

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 214.18' @ 12.49 hrs Surf.Area= 0.029 ac Storage= 0.033 af

Plug-Flow detention time= 22.7 min calculated for 0.150 af (100% of inflow) Center-of-Mass det. time= 22.6 min (766.1 - 743.5)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1A	212.50'	0.021 af	14.75'W x 85.29'L x 2.71'H Infiltration System
			0.078 af Overall - 0.026 af Embedded = 0.052 af x 40.0% Voids
#2A	213.00'	0.026 af	Cultec R-180 x 52 Inside #1
			Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf
			Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap
			Row Length Adjustment= +1.00' x 3.44 sf x 4 rows
		0.047 af	Total Available Storage
		0.0+1 ai	Total / Wallable Otorage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.50'	8.270 in/hr Exfiltration over Wetted area
			Conductivity to Groundwater Elevation = 192.90'
#2	Primary	214.00'	6.0" Round Culvert
			L= 142.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 214.00' / 211.58' S= 0.0170 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.20 sf

Discarded OutFlow Max=0.3 cfs @ 12.49 hrs HW=214.18' (Free Discharge) 1=Exfiltration (Controls 0.3 cfs)

Primary OutFlow Max=0.1 cfs @ 12.49 hrs HW=214.18' (Free Discharge) 2=Culvert (Inlet Controls 0.1 cfs @ 1.27 fps)

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Pond 14P: Infiltration System - Chamber Wizard Infiltration System

Chamber Model = Cultec R-180 (Cultec Recharger® 180HD)

Effective Size= 33.6"W x 20.0"H => 3.44 sf x 6.33'L = 21.8 cf Overall Size= 36.0"W x 20.5"H x 7.33'L with 1.00' Overlap Row Length Adjustment= +1.00' x 3.44 sf x 4 rows

36.0" Wide + 3.0" Spacing = 39.0" C-C Row Spacing

13 Chambers/Row x 6.33' Long +1.00' Row Adjustment = 83.29' Row Length +12.0" End Stone x 2 = 85.29' Base Length

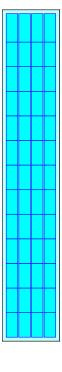
4 Rows x 36.0" Wide + 3.0" Spacing x 3 + 12.0" Side Stone x 2 = 14.75' Base Width 6.0" Stone Base + 20.5" Chamber Height + 6.0" Stone Cover = 2.71' Field Height

52 Chambers x 21.8 cf +1.00' Row Adjustment x 3.44 sf x 4 Rows = 1,145.9 cf Chamber Storage

3,407.2 cf Field - 1,145.9 cf Chambers = 2,261.3 cf Stone x 40.0% Voids = 904.5 cf Stone Storage

Chamber Storage + Stone Storage = 2,050.4 cf = 0.047 af Overall Storage Efficiency = 60.2% Overall System Size = 85.29' x 14.75' x 2.71'

52 Chambers 126.2 cy Field 83.8 cy Stone





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90	Subcat	9S:	Watershed	DC
----	--------	-----	-----------	----

- 91 Subcat 10S: Watershed DB
- 92 Subcat 11S: Watershed DD
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APPENDIX B - Storm Water Worksheets

Required Recharge Volume and Drawdown Worksheet
TSS Removal Worksheet
Cascade Separator Documentation
Mounding Analysis
DEP Stormwater Checklist

Required Recharge Volume Worksheet

PROJECT LOCATION: 10 Commerce Blvd Wrentham

DATE: March, 2023 **PROJECT NUMBER:** 21-0219

<u>Lot 1</u>

SCS Soil Type Hydrologic Group	Target Depth Factor (in)	Total Impervious Area (acre)	Required Volume to Recharge (ac-ft)
HSG A - Building & Parking	0.60	0.86	0.0429
		TOTAL:	0.0429

Volume Recharged		
Infiltration Basin Lot 2 Volume before discharge	3.29 ac-ft	
Lot 1 Infiltration System Volume before discharge	0.047 ac-ft	
Total volume available before discharge	3.337 ac-ft	

Drawdown RRV Within 72 hours	S	
Soil Type:	Sand	
RAWLS Rate (in/hr):	8.27	Sand
Infiltration Area (sf):	1,258	Bottom Lot 1 infiltration system
Drawdown Time (hours):	2.2	

TSS Removal Worksheet

PROJECT LOCATION: 10 Commerce Boulevard Wrentham

DATE: March, 2023 **PROJECT NUMBER:** 21-0219

Cascade Stream

Imp	ervious Area =	0.77	acres		
Runoff depth	to be treated =	1.77	1.77 inches (2" storm		
Runoff volume	to be treated =	0.1136	ac-ft		
	TSS Removal	Starting TSS	Amount	Remaining	
BMP	Rate	Load	Removed	Load	
Deep Sump and					
Hooded CB	0.25	1.00	0.25	0.75	
Cascade CS-8	0.5	0.75	0.38	0.38	
Infiltration Basin (Lot 2)	0.8	0.38	0.30	0.08	
, ,					
	TOTA	AL TSS REMOV	ED =	93 %	%

Standard Method to Convert Required WQV to a Discharge Rate

PROJECT LOCATION: 10 Commerce Boulevard Wrentham

DATE: 03/17/23 **PROJECT NUMBER:** 21-0219

Within or Near a Critical Area: Yes

WQV: 1.0 inch

Structure Location: DMH#4

Impervious Area = 0.001203 square miles

Runoff Curve Number - CN = 98

Time of Concentration - Tc = 0.1 hrs

Unit Peak Discharge - qu = 774 csm/inch see Table in Figure 4

Computed Flow Rate (1.0" of Runoff) $Q_{1.0} = 0.9$ cfs

Reference: Standard Method to Convert Required Water Quality Volume to a Discharge Rate for Sizing Flow Based Manufactured Proprietary Stormwater Treatment Practices, MassDEP 9/10/2013

Rational Method Flow Rate for 2-year event = 2.52 cfs

Transient Water-Table Rise Beneath a Rectangular Recharge Area Groundwater Mounding Solution by Hantush (1967)

Aquifer Properties:

Hydraulic conductivity, K = 16.54 ft/day Specific yield, Sy = 0.25Initial saturated thickness, h(0) = 25 ft

Recharge Area Properties:

Recharge rate, w = 1.1 ft/day Simulation time, t = 1 day Time when recharge stops, t(0) = 0.0833 day X coordinate at center of recharge area, X = 0 ft Y coordinate at center of recharge area, Y = 0 ft Length in x direction, I = 85.29 ft Length in y direction, I = 85.29 ft

Water-Table Rise at Center of Recharge Area:

t (day)	h (ft)
0.1	0.157057
0.2	0.0875722
0.3	0.062672
0.4	0.0488679
0.5	0.0400523
0.6	0.0339308
0.7	0.0294325
0.8	0.0259868
0.9	0.023263
1	0.021056
Note: recovery begins after 0.0833 day.	

Report generated by AQTESOLV v4.50.002 (www.aqtesolv.com) on 03/17/23 at 15:09:29.

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Mounding Analysis

PROJECT LOCATION: 10 Commerce Blvd Wrentham

DATE: 17-Mar-23 PROJECT NUMBER: 21-0219

Infiltration Basin

Aquifer Properties:

Hydraulic Conductivity

(K-ft/day): 16.54 **RAWLS Rate for Sand**

Specific Yield (Sy): 0.25 Medium gravel (USGS Water Supply Paper 1662-D)

Hydrogeological Study Report Wrentham Business

Initial Saturated Thickness Park Wrentham, MA May 22, 2000 by Carr Research

25 (ft): Laboratory, Inc.

Recharge Area Properties:

Required Recharge Volume

(Rv-ft3): 1,350 2-inch storm volume generated to basin

Hydrogeological Study Report Wrentham Business

Depth to Estimated High Park Wrentham, MA May 22, 2000 by Carr Research

Groundwater (ft): 193.00 Laboratory, Inc.

Bottom of Recharge

System (ft): 212.50 Bottom basin el-212.5'

Bottom Area (ft2): 1,258 Bottom basin el-212.5'

Application Rate Calculation:

Rv (ft3) Bottom Area (ft2)

1,350 1.1 ft/day 1,258

Length of Time to Generate

assume Rv generated during a 2 hour period - see DEP Stormwater Handbook, Vol.3, Ch.1, p.20 Rv (days): 0.0833

Groundwater Mounding Solution by Hantush (1967)

Maximum Water Table Rise in Center of Recharge Area

0.2 See output run using AQTESOLV V4.50.002 (ft)

Depth From Top of Mound to Bottom of Recharge Area

(ft): Mound does not breach bottom of system 19.3

Stormwater Report

A Stormwater Report must be submitted with the permit application to document compliance with the Stormwater Management Standards. The Stormwater Report must be organized into sections that correspond to the categories listed in the Checklist (e.g., Project Type, LID Practices, Standard 1 etc.). As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8¹
- Operation and Maintenance Plan required by Standard 9
- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (attached) that certifies that the Stormwater Report contains all required submittals.²

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has

¹ For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.

² The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

Massachusetts Stormwater Report Checklist

not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary to comprise a comprehensive Stormwater Report that addresses the ten Stormwater Standards. *Note:* Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

Stormwater Report 2

Massachusetts Stormwater Report Checklist

rec	oject Type: Is the application for new development, redevelopment, or a mix of new and levelopment? New Development Redevelopment Mix of New Development and Redevelopment
en	D Measures: Stormwater Standards require LID measures to be considered. Document what vironmentally sensitive design and LID Techniques were considered during the planning and sign of the project:
	No disturbance to any Wetland Resource Areas Site Design Practices (e.g. clustered development, reduced frontage setbacks) Reduced Impervious Area (Redevelopment Only) Minimizing disturbance to existing trees and shrubs LID Site Design Credit Requested: Credit 1 Credit 2 Credit 3 Use of "country drainage" versus curb and gutter conveyance and pipe Bioretention Cells (includes Rain Gardens) Constructed Stormwater Wetlands (includes Gravel Wetlands designs) Treebox Filter Water Quality Swale Grass Channel Green Roof Other ———————————————————————————————————
	Standard 1: No New Untreated Discharges
	No new untreated discharges Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.
	Standard 2: Peak Rate Attenuation
	Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding. Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm
	Calculations provided to show that post-development peak discharge rates do not exceed pre- development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off- site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

Soil Analysis provided.
Required Recharge Volume calculation provided
Required Recharge volume reduced through use of the LID site Design Credits.
Sizing the infiltration, BMPs is based on the following method: Circle the method used.
Static Simple Dynamic Dynamic Field ³
Runoff from all impervious areas at the site discharging to the infiltration BMP.
Runoff from all impervious areas at the site is <i>not</i> discharging to the infiltration BMP and
calculations are provided showing that the drainage area contributing runoff to the infiltration
BMPs is sufficient to generate the required recharge volume.
Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
Recharge BMPs have been sized to infiltrate the Required Recharge Volume <i>only</i> to the
maximum extent practicable for the following reason:
☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
☐ Solid Waste Landfill pursuant to 310 CMR 19.000
☐ Project is otherwise subject to Stormwater Management Standards only to the
maximum extent practicable.
Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is
included.
The infiltration BMP is used to attenuate peak flows during storms greater than or equal to
the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and
a mounding analysis is provided.
Documentation is provided showing that infiltration BMPs do not adversely impact nearby
wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;

Stormwater Report 4

³ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

Massachusetts Stormwater Report Checklist

- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergancy contacts for implementing Long Term Pollution Prevention

	List of Emergency contacts for implementing Long-Term Fortution Frevention
_	Plan.
Ц	A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
Ч	Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch
	rule for calculating the water quality volume are included, and discharge:
	is within the Zone II or Interim Wellhead Protection Area
	is near or to other critical areas
	is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
	involves runoff from land uses with higher potential pollutant loads.
	The Required Water Quality Volume is reduced through use of the LID site Design Credits.
	Calculations documenting that the treatment train meets the 80% TSS removal requirement
	and, if applicable, the 44% TSS removal pretreatment requirement, are provided.
	The BMP is sized (and calculations provided) based on:
	☐ The ½" or 1" Water Quality Volume or
	The equivalent flow rate associated with the Water Quality Volume and
	documentation is provided showing that the BMP treats the required water quality
	volume.
	The applicant proposes to use proprietary BMPs, and documentation supporting use of
	proprietary BMP and proposed TSS removal rate is provided. This documentation may be in
	the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts
	Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or
	other third party studies verifying performance of the proprietary BMPs.
	A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation
	showing that the BMPs selected are consistent with the TMDL is provided.
	showing that the Biri s selected are consistent with the Timbe is provided.
	Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)
	The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution
	Prevention Plan (SWPPP) has been included with the Stormwater Report.
	The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be
	submitted <i>prior to</i> the discharge of stormwater to the post-construction stormwater BMPs.
	The NPDES Multi-Sector General Permit does <i>not</i> cover the land use.
	LUHPPLs are located at the site and industry specific source control and pollution prevention
	measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow,
	snow melt and runoff, and been included in the long term Pollution Prevention Plan.
	All exposure has been eliminated

Massachusetts Stormwater Report Checklist

All exposure has <i>not</i> been eliminated and all BMPs selected are on MassDEP LUHPPL list. The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.
Standard 6: Critical Areas
The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
Critical areas and BMPs are identified in the Stormwater Report.
Standard 7: Redevelopments and Other Projects Subject to the Standards only to the
maximum extent practicable
The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a: Limited Project Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area. 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff Bike Path and/or Foot Path Redevelopment Project Redevelopment portion of mix of new and redevelopment.
Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report. The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- o Narrative;
- o Construction Period Operation and Maintenance Plan;
- o Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- o Erosion and Sedimentation Control Plan Drawings;
- o Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- o Vegetation Planning;
- o Site Development Plan;
- o Construction Sequencing Plan;
- o Sequencing of Erosion and Sedimentation Controls;
- o Operation and Maintenance of Erosion and Sedimentation Controls;
- o Inspection Schedule;
- o Maintenance Schedule;
- o Inspection and Maintenance Log Form.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan
containing the information set forth above has been included in the Stormwater Report.
The project is highly complex and information is included in the Stormwater Report that
explains why it is not possible to submit the Construction Period Pollution Prevention and
Erosion and Sedimentation Control Plan with the application. A Construction Period
Pollution Prevention and Erosion and Sedimentation Control has <i>not</i> been included in the
Stormwater Report but will be submitted <i>before</i> land disturbance begins.
The project is <i>not</i> covered by a NPDES Construction General Permit.
The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is
in the Stormwater Report.
The project is covered by a NPDES Construction General Permit but no SWPPP been
submitted. The SWPPP will be submitted BEFORE land disturbance begins.
Standard 9: Operation and Maintenance Plan
The Post Construction Operation and Maintenance Plan is included in the Stormwater Report
and includes the following information:
☐ Name of the stormwater management system owners;
☐ Party responsible for operation and maintenance;
☐ Schedule for implementation of routine and non-routine maintenance tasks;
☐ Plan showing the location of all stormwater BMPs maintenance access areas;
Description and delineation of public safety features;
☐ Estimated operation and maintenance budget; and
Operation and Maintenance Log Form.

Massachusetts Stormwater Report Checklist

The responsible party is <i>not</i> the owner of the parcel where the BMP is located and the
Stormwater Report includes the following submissions:
 □ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs; □ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.
Standard 10: Prohibition of Illicit Discharges
The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges: An Illicit Discharge Compliance Statement is attached; NO Illicit Discharge Compliance Statement is attached but will be submitted <i>prior to</i> the discharge of any stormwater to post-construction BMPs.

APPENDIX C - Operation and Maintenance Plan and Long Term Pollution Prevention Plan for Storm Water BMPs

Appendix C: OPERATION AND MAINTENANCE PLAN FOR STORMWATER BMPs

During Construction Post-construction

BMP Owner: Edgewood Development Co. Lot Owner Party of Plan Responsibility: Edgewood Development Co. Lot Owner

References:

- Definitive Plan of "Commerce Way Wrentham, MA" by Bay Colony Group, Inc.
- Site Development Plan 10 Commerce Boulevard dated April 19, 2023 to last revision
- Stormwater Pollution Prevention Plan for Construction Activities Wrentham Business Park

Operation and Maintenance – During Construction

- Item 1: During construction, **weekly** inspection of the crushed stone construction entrance pad and erosion control silt socks shall be conducted by a qualified staff member of the responsible party or an independent sediment and erosion control expert hired by the responsible party. Any displaced barriers shall be restored or repaired immediately. All barriers shall be installed where possible **5** ft from the property line.
- Item 2: The catch basins within the project site shall be set to base course grade so that they are functional throughout the project. They shall be inspected **before** and **after** rain storms, if the basins are filled with sediment to half of its depth, these basins shall be cleaned out with an orange peel bucket or some other means. Silt sacks shall be installed within the catch basins to ensure that siltation does not enter the catch basin. Any debris in basins should be cleaned out. The roadway will be swept as necessary.
- Item 3: Do not allow unfiltered runoff to enter the storm water basin or it will clog. If it becomes clogged, it is the contractor's responsibility for restoring it.
- Item 4: Inspect storm water basin after every major storm (1.0 inches in 24 hours) and, if necessary, take corrective action.
- Item 5: Sediment basins shall be inspected after every storm and weekly. Clean out sediment when it reaches half of the depth of the basin. Scarify basin bottom after each clean out. Repair any damage to the sides and rip-rap outlet structure.

Operation and Maintenance – Post Construction

Item 1: The catch basins shall be inspected four times a year: beginning of summer, after leaf fall, before the arrival of hurricane season, and in the early or mid-spring after the snow melt and road sweeping. Any debris in basins should be cleaned out. The roadway will be swept twice a year: once after leaf fall, the other in the spring after snow melt.

Item 2: Inspect the subsurface infiltration basin at least twice per year for the following:

- Water within the basin 72 hours after a rain event
- Remove any debris that might clog the system

Item 3: Contech Cascade Separator:

Inspect Cascade structure in accordance with the latest manufacturer's maintenance manual, which can be found at <u>Contech Engineered Solutions</u> Technical Guides (conteches.com)

Item 4: Estimated Operations and Maintenance Budget: The following is an estimate of the O&M Budget, post construction.

Inspections – infiltration basin, Cascade, catch basins (3 times per year): \$420 Cleaning catch basins (4 times per year): \$1,500 Inspecting/cleaning subsurface basin (2 times per year): \$320 Lot Sweeping (2 times per year): \$800

Long Term Pollution Prevention Plan

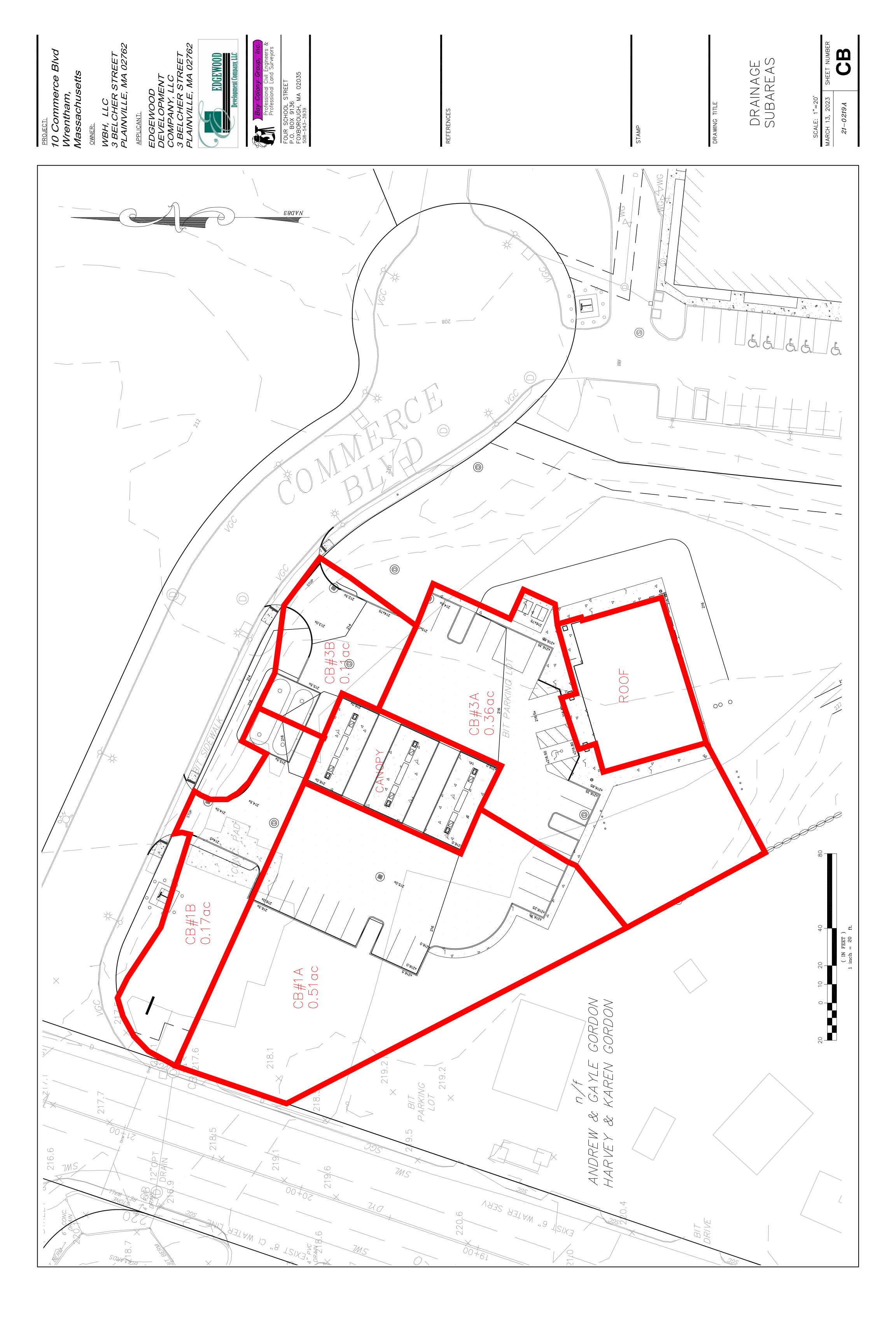
- <u>Item 1 Good housekeeping practices:</u> The site is to be kept clean of trash and debris. No trash or uncovered materials is to be left outside.
- <u>Item 2 Provisions for storing materials and waste products inside or under cover</u>: All waste materials will be stored in enclosed dumpsters and removed by a licensed solid waste company. No waste products will be stored outside the facility unless in dumpsters.
- Item 3 Vehicle washing controls: Vehicles will not be washed on this site.
- <u>Item 4 Requirements for routine inspections and maintenance of stormwater BMPs</u>: Refer to the maintenance schedule provided in the Operation and Maintenance Plan Post Construction.
- <u>Item 5 Spill prevention and response plans</u>: A spill prevention and response plan will be developed and implemented by the building occupant. A draft plan is attached that may be modified once the tenant has been identified.
- <u>Item 6 Provisions for maintenance of planters, gardens, parks and other landscaped areas:</u> Owner will maintain surrounding landscaped area with the purpose of retaining the landscaped as designed.
- <u>Item 7 Requirements for storage and use of fertilizers, herbicides, and pesticides</u>: If present, fertilizers, herbicides and pesticides shall be stored in their appropriate containers within the building. They shall be handled and used in accordance with the manufacturer's recommendations. It is anticipated that a landscape contractor with have the responsibility of maintaining the property and these materials will be stored off site. <u>Item 8 Pet waste management provisions</u>: Outdoor trash receptacles will be placed in convenient areas for vehicle owners to dispose of pet waste.
- <u>Item 9 Provisions for solid waste management:</u> Solid waste material shall be placed in outdoor enclosed containers until emptied by licensed waste management company.
- <u>Item 10 Snow disposal and plowing plans</u>: A snow removal plan will be developed and implemented by the tenant and Owner. A draft plan is attached.
- Item 11 Winter Road Salt/or Sand Use and Storage restriction: See item above.
- <u>Item 12 Sweeping schedules</u>: See Operations and Maintenance Plan Post Construction.
- Item 13 Training for staff or personnel involved with the implementing Long Term Pollution Prevention Plan: The facility Operations Manager will be responsible for training necessary staff or subcontractors to the implement the plan.
- <u>Item 14 List of Emergency contacts for implementing Long-Term Pollution Prevention</u> Plan: TBD

APPENDIX D - Closed Drainage System Design

Closed Drainage System Analysis 10 Commerce Blvd Wrentham, MA

Upstream Node	Downstream Node	Section Size	Length (ft)	Constructed Slope (ft/ft)	Total Flow (cfs)	Capacity Free Flow (cfs)	Rational Coefficient	Average Velocity (ft/sec)	Upstream Ground Elevation (ft)	Upstream Invert Elevation (ft)	Upstream Structure Hydraulic Grade (ft)	Downstream Ground Elevation (ft)	Downstream Invert Elevation (ft)	Downstream Structure Hydraulic Grade (ft)
DMH#4	DMH#2(EXIST)	12 inch	24	0.021	3.9	5.6	N/A	7.7	210.50	204.70	205.54	210.00	204.20	204.20
DMH#3	DMH#4	12 inch	20	0.051	3.9	10.5	A/A	12.4	214.30	208.30	209.14	210.50	204.70	205.54
CB#3B	DMH#3	12 inch	34	900.0	0.3	3.0	7.0	2.5	212.50	208.50	209.36	214.30	208.30	209.23
DMH#2	DMH#3	12 inch	28	600.0	2.4	3.6	A/A	4.9	215.50	208.80	209.46	214.30	208.30	209.23
DMH#1	DMH#2	12 inch	66	0.010	2.4	3.8	A/A	5.1	214.30	209.70	210.36	215.50	208.80	209.50
CB#1B	DMH#1	12 inch	38	0.008	9.0	3.4	7.0	3.3	214.00	210.00	210.38	214.30	209.70	210.40
CB#3A	DMH#3	12 inch	23	0.100	1.3	12.2	7.0	10.1	214.60	210.60	211.08	214.30	208.30	209.23
CB#1A	DMH#1	12 inch	99	0.020	1.8	5.5	7.0	6.2	215.00	211.00	211.57	214.30	209.70	210.40
CANOPY	Infiltration System	6 inch	240	0.005	0.4	0.4	7.0	2.5	216.00	214.20	214.57	216.00	213.00	213.00
ROOF	Infiltration System	6 inch	122	0.014	0.4	0.7	0.7	3.8	216.75	214.75	215.07	216.00	213.00	213.00
Design Storm	Design Storm is 25 year event as shown on TD40 Boston MA	OLOT GO GIVE	N Poston M	·										

Design Storm is 25-year event as shown on TP40 Boston, MA



APPENDIX E – Draft Storm Water Pollution Prevention Plan

Stormwater Pollution Prevention Plan (SWPPP)

For Construction Activities At:

10 Commerce Boulevard Wrentham, MA Telephone: TBD

SWPPP Prepared For:

Edgewood Development Co., Inc. 320 South Street Plainville, MA 02762 508.643.2920

SWPPP Prepared By:

Bay Colony Group, Inc. 4 School Street Foxborough, MA 02035 508.543.3939 508.543.8866 fax

SWPPP Preparation Date:

April, 2023

Estimated Project Dates:

Project Start Date: Spring, 2024
Project Completion Date: Fall, 2024

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SECTION 1: CONTACT INFORMATION/RESPONSIBLE PARTIES

1.1 Operator(s) / Subcontractor(s)

Operator(s):

A. TBD

General Contractor

Subcontractor(s):

Insert Company or Organization Name:

Insert Name:

Insert Address:

Insert City, State, Zip Code:

Insert Telephone Number:

Insert Fax/Email:

Insert area of control (if more than one operator at site):

[Repeat as necessary.]

Emergency 24-Hour Contact:

A. Insert name address, telephone number

1.2 Stormwater Team

Insert Role or Responsibility: Project Manager

Insert Position: **Project Manager**

Insert Name: Name

Insert Telephone Number: number

Insert Email: email

Insert Role or Responsibility:

Insert Position: Insert Name:

Insert Telephone Number:

Insert Email:

Insert Role or Responsibility:

Insert Position:

Insert Name:

Insert Telephone Number:

Insert Email:

[Repeat as necessary.]



SECTION 2: SITE EVALUATION, ASSESSMENT, AND PLANNING

2.1 Project/Site Information

Project Name and Address	
Project/Site Name: 10 Commerce Boulevard Project Street/Location: 10 Commerce Boulevard City: Wrentham State: MA	
ZIP Code: 02093	
County or Similar Subdivision: Norfolk	
Project Latitude/Longitude	
(Use one of three possible formats, and specify meth	nod)
Latitude:	Longitude:
1. 42 ° 02 ' 44" N (degrees, minutes, seconds)	1. 71 ° 18 ' 08" W (degrees, minutes,
2 ° ' N (degrees, minutes, decimal)	seconds) 2°' W (degrees, minutes, decimal)
3 ° N (decimal)	3 ° W (decimal)
Method for determining latitude/longitude: USGS topographic map (specify scale: Other (please specify):)
Horizontal Reference Datum: NAD 27 NAD 83 or WGS 84 Unknown If you used a U.S.G.S topographic map, what was th	ne scale?
, , , , , , , , , , , , , , , , , , , ,	
Additional Project Information	
Is the project/site located on Indian country lands, of cultural significance to an Indian tribe? \square Yes	or located on a property of religious or No
If yes, provide the name of the Indian tribe associated (including the name of Indian reservation if applicable the name of the Indian tribe associated with the pro-	ole), or if not in Indian country, provide
If you are conducting earth-disturbing activities in redocument the cause of the public emergency (e.g., conditions), information substantiating its occurrence	, natural disaster, extreme flooding

a description of the construction necessary to reestablish effective public services: N/A

Are you applying for permit coverage as a "federal operator" as defined in Appendix A of

the 2012 CGP? Tyes No

2.2 Discharge Information

	r project/site discharge stormwater into a Municipal Separate Storm Sewer System Yes No
Are there	any surface waters that are located within 50 feet of your construction disturbances?
☐ Yes	⊠No

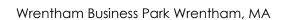


Table 1 – Names of Receiving Waters

Name(s) of the first surface water that receives stormwater directly from your site and/or from the MS4 (note: multiple rows provided where your site has more than one point of discharge that flows to different surface waters) 1. Rabbit Hill Brook 3. તં 4 Ś

Table 2 – Impaired Waters / TMDLs (Answer the following for each surface water listed in Table 1 above)

			vhich	DI						
			Pollutant(s) for which	there is a TMDL						
	If you answered yes, then answer the following:		1.1.1.2.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1							
थ । ज्याना उत्ता व	If you answere	Has a TMDL	peen	completed?	☐ YES ☐ NO	☐ YES ☐ NO ☐	□ YES □ NO	☐ YES ☐ NO ☐	□ YES □ NO □	☐ YES ☐ NO ☐
			What pollutant(s) are causing	the impairment?						
		Is this surface	water listed as	"impaired"?	□ YES ⊠ NO	☐ YES ☐ NO	□ YES □ NO	□ YES □ NO	□ YES □ NO	☐ YES ☐ NO
2					1.	5.		4	5.	6.

[Include additional rows as necessary.]

Describe the method(s) you used to determine whether or not your project/site discharges to an impaired water: Review of the MassDEP 2014 Integrated List of Waters. Rabbit Hill Stream is listed as Category 3 – No TMDL Not Evaluated

Table 3 – Tier 2, 2.5, or 3 Waters (Answer the following for each surface water listed in Table 1 above)

	Is this surface water designated	If you answered yes, specify which
	as a Tier 2, Tier 2.5, or Tier 3	Tier (2, 2.5, or 3) the surface water is
	water?	designated as?
	(see Appendix F)	
1.	TYES NO	INSERT "Tier 2", "Tier 2.5", or "Tier 3"
2	TYES NO	INSERT "Tier 2", "Tier 2.5", or "Tier 3"
ж Э	TYES NO	INSERT "Tier 2", "Tier 2.5", or "Tier 3"
4	TYES NO	INSERT "Tier 2", "Tier 2.5", or "Tier 3"
5.	YES NO	INSERT "Tier 2", 'Tier 2.5", or "Tier 3"

Wrentham Business Park Wrentham, MA

2.3 Nature of the Construction Activity

General Description of Project

Provide a general description of the construction project:

Construction of a 4,500 convenience store w/ fueling facilities with associated utilities, septic system and storm water system.

Size of Construction Project

What is the size of the property (in acres), the total area expected to be disturbed by the construction activities (in acres), and the maximum area expected to be disturbed at any one time?

INSERT SIZE OF PROPERTY - 2.6+/- acres

INSERT TOTAL AREA OF CONSTRUCTION DISTURBANCES - 2+/- acres

INSERT MAXIMUM AREA TO BE DISTURBED AT ANY ONE TIME - 2+/- acres

[Repeat as necessary for individual project phases.]

Construction Support Activities (only provide if applicable)

Describe any construction support activities for the project (e.g., concrete or asphalt batch plants, equipment staging yards, material storage areas, excavated material disposal areas, borrow areas)

INSERT DESCRIPTION OF CONSTRUCTION SUPPORT ACTIVITY

INSERT CONTACT INFORMATION FOR CONSTRUCTION SUPPORT ACTIVITY (Name, Telephone No., Email Address)

INSERT LOCATION INFORMATION FOR CONSTRUCTION SUPPORT ACTIVITY (Address and/or Latitude/Longitude)

[Repeat as necessary.]

2.4 Sequence and Estimated Dates of Construction Activities

Phase I

Clearing of building site and storm water basins, installation of erosion controls, and grubbing of wooded areas, and storm water basins.

- INSERT ESTIMATED START AND END DATES OF CONSTRUCTION DISTURBANCES ASSOCIATED WITH THIS PHASE
- FOR EACH STORMWATER CONTROL, INSERT ESTIMATED DATE(s) OF INSTALLATION OF EACH STORMWATER CONTROL
- FOR AREAS OF THE SITE REQUIRED TO BE STABILIZED, INSERT ESTIMATED DATE(s) OF APPLICATION OF STABILIZATION MEASURES
- INSERT ESTIMATED DATE(s) WHEN STORMWATER CONTROLS WILL BE REMOVED

Phase II

Import and placement of material to bring building and parking to subbase elevation. Construction of storm water basins. Installation of drainage and water mains within site.

- INSERT ESTIMATED START AND END DATES OF CONSTRUCTION DISTURBANCES ASSOCIATED WITH THIS PHASE
- FOR EACH STORMWATER CONTROL, INSERT ESTIMATED DATE(s) OF INSTALLATION OF EACH STORMWATER CONTROL
- FOR AREAS OF THE SITE REQUIRED TO BE STABILIZED, INSERT ESTIMATED DATE(s) OF APPLICATION OF STABILIZATION MEASURES
- INSERT ESTIMATED DATE(s) WHEN STORMWATER CONTROLS WILL BE REMOVED.

Phase III

Installation of base course of parking and construction of building.

- INSERT ESTIMATED START AND END DATES OF CONSTRUCTION DISTURBANCES ASSOCIATED WITH THIS PHASE
- FOR EACH STORMWATER CONTROL, INSERT ESTIMATED DATE(s) OF INSTALLATION OF EACH STORMWATER CONTROL
- FOR AREAS OF THE SITE REQUIRED TO BE STABILIZED, INSERT ESTIMATED DATE(s) OF APPLICATION OF STABILIZATION MEASURES
- INSERT ESTIMATED DATE(s) WHEN STORMWATER CONTROLS WILL BE REMOVED

Phase IV

Construct landscaping, finish building, place finished course of pavement. Remove storm water erosion controls.

- INSERT ESTIMATED START AND END DATES OF CONSTRUCTION DISTURBANCES ASSOCIATED WITH THIS PHASE
- FOR EACH STORMWATER CONTROL, INSERT ESTIMATED DATE(s) OF INSTALLATION OF EACH STORMWATER CONTROL
- FOR AREAS OF THE SITE REQUIRED TO BE STABILIZED, INSERT ESTIMATED DATE(s) OF APPLICATION OF STABILIZATION MEASURES
- INSERT ESTIMATED DATE(s) WHEN STORMWATER CONTROLS WILL BE REMOVED

[Repeat as needed.]

2.5 Allowable Non-Stormwater Discharges

List of Allowable Non-Stormwater Discharges Present at the Site

Type of Allowable Non-Stormwater Discharge	Likely to be Present at Your Site?
Discharges from emergency fire-fighting activities	☐ YES ☒ NO
Fire hydrant flushings	
Landscape irrigation	☐ YES ☐ NO
Waters used to wash vehicles and equipment	☐ YES ☒ NO
Water used to control dust	
Potable water including uncontaminated water line flushings	YES □ NO
Routine external building wash down	☐ YES ☒ NO
Pavement wash waters	☐ YES ☒ NO
Uncontaminated air conditioning or compressor condensate	☐ YES ☐ NO

Uncontaminated, non-turbid discharges of ground water or spring water	☐ YES ☒ NO
Foundation or footing drains	☐ YES ☒ NO
Construction dewatering water	☑ YES □ NO

(Note: You are reminded of the requirement to identify the likely locations of these allowable non-stormwater discharges on your site map. See Section 2.6, below, of the SWPPP Template.)

2.6 Site Maps

See Site Development Plan of 10 Commerce Boulevard by Bay Colony Group, Inc.



SECTION 3: DOCUMENTATION OF COMPLIANCE WITH OTHER FEDERAL REQUIREMENTS

3.1 Endangered Species Protection

Eligibility Crit					
Under which	Criterion liste	ed in Appendix D ar	e you eligible for co	verage under this p	ermit?
				-	
For refer	ence purpo	ses, the eligibility cri	teria listed in Appen	dix D are as follows	:
Criterion			or endangered specie your site's "action are		
Criterion	addresse under eli listed spe certifica under th operator with any was base notificati operator	ed in another operatorigibility Criterion A, C, I ecies or federally-designation may be present of is Criterion, there mustr's certification. By certification. By certification include ion of authorization under coefficient on the certification of authorization under coefficient in the certification under certific	ges and discharge-relor's valid certification of D, E, or F and there is no gnated critical habitator located in the "action be no lapse of NPDES trifying eligibility under conditions upon which in your NOI the tracking this permit. If your Criterion C, you must ped of existing discharge	f eligibility for your actino reason to believe the not considered in the on area". To certify you permit coverage in the this Criterion, you agree the other operator's on the other operator's on the other operator's certification is based oprovide EPA with the research to revise the other operator.	ion area nat federally- prior ur eligibility ne other ee to comply certification her operator's on another elevant
Criterion	are likely discharg endange any storr your disc species o your NO "action o	to occur in or near your related activities are ered species or critical mwater controls and/ocharges and discharges and critical habitat. To lit 1) any federally listed area"; and 2) the distated critical habitat (in	endangered species of our site's "action area, e not likely to adversel I habitat. This determinal or management pract e-related activities are of make this certification dispecies and/or designance between your sit miles). You must also i	" and your site's dischey affect listed threater nation may include coices you will adopt to not likely to adversely on, you must include the gnated habitat located to and the listed specific and th	arges and ned or onsideration of ensure that affect listed ne following in d in your es or
Criterion	must hav activities designat relevant likely to d	ve addressed the effects on federally-listed thread critical habitat, an Service(s) that your sit adversely affect listed	nd the Services has been cts of your site's dischalated or endangered must have resulted ite's discharges and disspecies or critical hab urself and the Services	arges and discharge-re ed species and federd in a written concurren scharge-related activi itat. You must include	elated ally- ce from the ties are not e copies of the
Criterion	the Nation The constand disc	onal Marine Fisheries So sultation must have ac harge-related activitie	ral Agency and the U.: ervice under section 7 ddressed the effects of es on federally-listed th cal habitat. The result	of the ESA has been of the construction site's preatened or endange	concluded. s discharges ered species

- i. a biological opinion that concludes that the action in question (taking into account the effects of your site's discharges and discharge-related activities) is not likely to jeopardize the continued existence of listed species, nor the destruction or adverse modification of critical habitat; or
- ii. written concurrence from the applicable Service(s) with a finding that the site's discharges and discharge-related activities are not likely to adversely affect federally-listed species or federally-designated habitat.

You must include copies of the correspondence between yourself and the Services in your SWPPP and your NOI.

Criterion F.

Your construction activities are authorized through the issuance of a permit under section 10 of the ESA, and this authorization addresses the effects of the site's discharges and discharge-related activities on federally-listed species and federally-designated critical habitat. You must include copies of the correspondence between yourself and the Services in your SWPPP and your NOI.

Supporting Documentation

Provide documentation for the applicable eligibility criterion you select in Appendix D, as follows:

For criterion A, indicate the basis for your determination that no federally-listed threatened or endangered species or their designated critical habitat(s) are likely to occur in your site's action area (as defined in Appendix A of the permit). Check the applicable source of information you relied upon:

	Specific communication with staff of the U.S. Fish & Wildlife Service or National Marine
	Fisheries Service. INSERT DATE OF COMMUNICATION AND WHO YOU SPOKE WITH
\boxtimes	Publicly available species list. MASS GIS Website – NHESP Tabs
	Other source: INSERT SPECIFIC SOURCE

For criterion B, provide the Tracking Number from the other operator's notification of permit authorization: INSERT AUTHORIZATION TRACKING NUMBER FROM OTHER OPERATOR'S NOTIFICATION LETTER/EMAIL

Provide a brief summary of the basis used by the other operator for selecting criterion A, B, C, D, E, or F: INSERT TEXT HERE

For criterion C, provide the following information:

- INSERT LIST OF FEDERALLY-LISTED SPECIES OR FEDERALLY-DESIGNATED CRITICAL HABITAT LOCATED IN YOUR ACTION AREA
- INSERT DISTANCE BETWEEN YOUR SITE AND THE LISTED SPECIES OR CRITICAL HABITAT (in miles)

Also, provide a brief summary of the basis used for determining that your site's discharges and discharge-related activities are not likely to adversely affect listed species or critical habitat: INSERT TEXT HERE

For criterion D, E, or F, attach copies of any letters or other communication between you and the U.S. Fish & Wildlife Service or National Marine Fisheries Service concluding consultation or

coordination activities. INSERT COPIES OF LETTERS OR OTHER COMMUNICATIONS HERE

3.2 Historic Preservation

Appendix E, Step 1
Do you plan on installing any of the following stormwater controls at your site? Check all that
apply below, and proceed to Appendix E, Step 2.
Dike
☐ Berm
☐ Catch Basin
☐ Pond
Stormwater Conveyance Channel (e.g., ditch, trench, perimeter drain, swale, etc.)
□ Culvert □ Culve
Other type of ground-disturbing stormwater control: INSERT SPECIFIC TYPE OF
STORMWATER CONTROL
(Note: If you will not be installing any ground-disturbing stormwater controls, no further
documentation is required for Section 3.2 of the Template.)
Annanda F Chan O
Appendix E, Step 2
If you answered yes in Step 1, have prior surveys or evaluations conducted on the site already
determined that historic properties do not exist, or that prior disturbances at the site have precluded the existence of historic properties? \square YES \bowtie NO
produced in existence of historic properties.
If yes, no further documentation is required for Section 3.2 of the Template.
If no, proceed to Appendix E, Step 3.
Appendix E, Step 3
If you answered no in Step 2, have you determined that your installation of subsurface earth-
disturbing stormwater controls will have no effect on historic properties? 🛛 YES 🔲 NO
If yes, provide documentation of the basis for your determination. Reference to the
Massachusetts Cultural Resources Information System shows no historical areas, buildings, buric grounds, objects or structures on or near the site.
grounds, objects of siloctores on of fledrine sile.
If no, proceed to Appendix E, Step 4.
Appendix E, Step 4
If you answered no in Step 3, did the State Historic Preservation Officer (SHPO), Tribal Historic
Preservation Office (THPO), or other tribal representative (whichever applies) respond to you
within 15 calendar days to indicate whether the subsurface earth disturbances caused by the
installation of stormwater controls affect historic properties? \square YES \square NO

f no, no further documentation is required for Section 3.2 of the Template.
f yes, describe the nature of their response:
■ Written indication that adverse effects to historic properties from the installation of stormwater controls can be mitigated by agreed upon actions. INSERT COPIES OF LETTERS, EMAILS, OR OTHER COMMUNICATION BETWEEN YOU AND THE APPLICABLE SHPO, THPO, OR OTHER TRIBAL REPRESENTATIVE
☐ No agreement has been reached regarding measures to mitigate effects to historic properties from the installation of stormwater controls. INSERT COPIES OF LETTERS, EMAILS, OR OTHER COMMUNICATION BETWEEN YOU AND THE APPLICABLE SHPO, THPO, OR OTHER TRIBAL REPRESENTATIVE
Other: INSERT COPIES OF LETTERS, EMAILS, OR OTHER COMMUNICATION BETWEEN YOU AND THE APPLICABLE SHPO, THPO, OR OTHER TRIBAL REPRESENTATIVE
3.3 Safe Drinking Water Act Underground Injection Control Requirements
Do you plan to install any of the following controls? Check all that apply below.
Infiltration trenches (if stormwater is directed to any bored, drilled, driven shaft or dug hole that is deeper than its widest surface dimension, or has a subsurface fluid distribution system)
Commercially manufactured pre-cast or pre-built proprietary subsurface detention vaults, chambers, or other devices designed to capture and infiltrate stormwater flow
Drywells, seepage pits, or improved sinkholes (if stormwater is directed to any bored, drilled, driven shaft or dug hole that is deeper than its widest surface dimension, or has a subsurface fluid distribution system)
f yes, INSERT COPIES OF LETTERS, EMAILS, OR OTHER COMMUNICATION BETWEEN YOU AND THE STATE AGENCY OR EPA REGIONAL OFFICE - no correspondence undertaken. Project is permitted under the MassDEP Stormwater Regulations which are enforced by the local Board of Health and Planning Board.

SECTION 4: EROSION AND SEDIMENT CONTROLS

4. I	Natural Buffers or Equivalent Seament Controls
Are the	Compliance Alternatives ere any surface waters within 50 feet of your project's earth disturbances? ☐ YES ☒ NO ote: If no, no further documentation is required for the SWPPP Template.)
Check	the compliance alternative that you have chosen:
	I will provide and maintain a 50-foot undisturbed natural buffer. (Note (1): You must show the 50-foot boundary line of the natural buffer on your site map.) (Note (2): You must show on your site map how all discharges from your construction disturbances through the natural buffer area will first be treated by the site's erosion and sediment controls. Also, show on the site map any velocity dissipation devices used to preven erosion within the natural buffer area.)
	I will provide and maintain an undisturbed natural buffer that is less than 50 feet and is supplemented by additional erosion and sediment controls, which in combination achieves the sediment load reduction equivalent to a 50-foot undisturbed natural buffer.
	(Note (1): You must show the boundary line of the natural buffer on your site map.) (Note (2): You must show on your site map how all discharges from your construction disturbances through the natural buffer area will first be treated by the site's erosion and

- INSERT WIDTH OF NATURAL BUFFER TO BE RETAINED
- INSERT EITHER ONE OF THE FOLLOWING:

erosion within the natural buffer area.)

(1) THE ESTIMATED SEDIMENT REMOVAL FROM A 50-FOOT BUFFER USING APPLICABLE TABLES IN APP. G, ATTACHMENT 1. INCLUDE INFORMATION ABOUT THE BUFFER VEGETATION AND SOIL TYPE THAT PREDOMINATE AT YOUR SITE

sediment controls. Also, show on the site map any velocity dissipation devices used to prevent

(2) IF YOU CONDUCTED A SITE-SPECIFIC CALCULATION FOR THE ESTIMATED SEDIMENT REMOVAL OF A 50-FOOT BUFFER, PROVIDE THE SPECIFIC REMOVAL EFFICIENCY, AND INFORMATION YOU RELIED UPON TO MAKE YOUR SITE-SPECIFIC CALCULATION.

- INSERT DESCRIPTION OF ADDITIONAL EROSION AND SEDIMENT CONTROLS TO BE USED IN COMBINATION WITH NATURAL BUFFER AREA
- INSERT THE FOLLOWING INFORMATION:
 - (1) SPECIFY THE MODEL OR OTHER TOOL USED TO ESTIMATE SEDIMENT LOAD REDUCTIONS FROM THE COMBINATION OF THE BUFFER AREA AND ADDITIONAL EROSION AND SEDIMENT CONTROLS INSTALLED AT YOUR SITE, AND
 - (2) INCLUDE THE RESULTS OF CALCULATIONS SHOWING THAT THE COMBINATION OF YOUR BUFFER AREA AND THE ADDITIONAL EROSION AND SEDIMENT CONTROLS INSTALLED AT YOUR SITE WILL MEET OR EXCEED THE SEDIMENT REMOVAL EFFICIENCY OF A 50-FOOT BUFFER

It is infeasible to provide and maintain an undisturble implement erosion and sediment controls that act a 50-foot undisturbed natural buffer.	· · · · · · · · · · · · · · · · · · ·
 DESCRIPTION OF WHY IT IS NOT FEASIBLE INSERT EITHER ONE OF THE FOLLOWING: 	
(1) THE ESTIMATED SEDIMENT REMOVAL FRO	MA 50-FOOT BUFFER USING APPLICABLE TABLES MATION ABOUT THE BUFFER VEGETATION AND
OR	
(2) IF YOU CONDUCTED A SITE-SPECIFIC CA REMOVAL OF A 50-FOOT BUFFER, PROVIDE INFORMATION YOU RELIED UPON TO MAKE INSERT DESCRIPTION OF ADDITIONAL EROSI- COMBINATION WITH NATURAL BUFFER AREA INSERT THE FOLLOWING INFORMATION:	THE SPECIFIC REMOVAL EFFICIENCY, AND YOUR SITE-SPECIFIC CALCULATION. ON AND SEDIMENT CONTROLS TO BE USED IN
 (1) SPECIFY THE MODEL OR OTHER TOO REDUCTIONS FROM THE EROSION AND SAND 	L USED TO ESTIMATE SEDIMENT LOAD SEDIMENT CONTROLS INSTALLED AT YOUR SITE,
	ONS SHOWING THAT THE ADDITIONAL EROSION T YOUR SITE WILL MEET OR EXCEED THE SEDIMENT JFFER
☐ I qualify for one of the exceptions in Part 2.1.2.1.e. information on the applicable buffer exception the Buffer Exceptions	
Which of the following exceptions to the buffer requirement	nts applies to your site?
There is no discharge of stormwater to the surface construction disturbances.	water that is located 50 feet from my
(Note: If this exception applies, no further doc Template.)	umentation is required for Section 4.1 of the
No natural buffer exists due to preexisting develop initiation of planning for this project.	
(Note (1): If this exception applies, no further of Template.)	documentation is required for Section 4.1 of the
(Note (2): Where some natural buffer exists bu surface water are occupied by preexisting dewith the one of the CGP Part 2.1.2.1.a complication.	velopment disturbances, you must still comply
For a "linear project" (defined in Appendix A), site infeasible for me to meet any of the CGP Part 2.1.: DOCUMENTATION HERE OF THE FOLLOWING: (1) W THE BUFFER COMPLIANCE ALTERNATIVES, AND (2) E EROSION AND SEDIMENT CONTROLS TO TREAT DISC	2.1.a compliance alternatives. INCLUDE //HY IT IS INFEASIBLE FOR YOU TO MEET ONE OF SUFFER WIDTH RETAINED AND/OR SUPPLEMENTAL

The project qualifies as "small residential lot" construction (defined in Part 2.1.2.1.e.iv and in Appendix A).

For Alternative 1 (see Appendix G, Part G.2.3.2.a):

- INSERT WIDTH OF NATURAL BUFFER TO BE RETAINED
- INSERT APPLICABLE REQUIREMENTS BASED ON TABLE G-1
- INSERT DESCRIPTION OF HOW YOU WILL COMPLY WITH THESE REQUIREMENTS

For Alternative 2 (see Appendix G, Part G.2.3.2.b):

- INSERT (1) THE ASSIGNED RISK LEVEL BASED ON APPLICABLE TABLE IN APP. G, PART G.2.3.2.b, AND (2) THE PREDOMINANT SOIL TYPE AND AVERAGE SLOPE AT YOUR SITE
- INSERT APPLICABLE REQUIREMENTS BASED ON APP. G, TABLE G-7
- INSERT DESCRIPTION OF HOW YOU WILL COMPLY WITH THESE REQUIREMENTS

Buffer disturbances are authorized under a CWA Section 404 permit. INSERT DESCRIPTION OF ANY
EARTH DISTURBANCES THAT WILL OCCUR WITHIN THE BUFFER AREA
(Note (1): If this exception applies, no further documentation is required for Section 4.1 of the
Template.)

(Note (2): This exception only applies to the limits of disturbance authorized under the Section 404 permit, and does not apply to any upland portion of the construction project.)

Buffer disturbances will occur for the construction of a water-dependent structure or water access area (e.g., pier, boat ramp, and trail). INSERT DESCRIPTION OF ANY EARTH DISTURBANCES THAT WILL OCCUR WITHIN THE BUFFER AREA

(Note (1): If this exception applies, no further documentation is required for Section 4.1 of the Template.)

4.2 Perimeter Controls

General

The perimeter of the site where the proposed work is could impact wetlands through runoff to the storm drain system will contain a silt sock barrier that will capture siltation and runoff.

Specific Perimeter Controls

Perimeter Control # 1

Perimeter Control Description

- Silt sock barrier
- See SWPP Plan Appendix A and Site Plan

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

Weekly inspection and removal of sediment once it reaches at least ½ way up the barrier.

[Repeat as needed for individual perimeter controls.]

4.3 Sediment Track-Out

General

Rip rap stabilized construction entrance.

Specific Track-Out Controls

Track-Out Control # 1

Track-Out Control Description

- Rip rap stabilized construction entrances at points where they meet existing Commerce Blvd pavement
- See SWPP Plan Appendix A and Site Plan

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

- Monitor and maintain the Stabilized Construction Entrance shown on the SWPPP Plan to ensure that
 it is cleaned and functioning correctly to prevent tracking of sediment by construction that exit the
 Site.
- Where sediment has been tracked-out from the site onto the surface of off-site streets, other paved areas, and sidewalks, you must remove the deposited sediment by the end of the same work day in which the track-out occurs or by the end of the next work day if track-out occurs on a non-work day. You must remove the track-out by sweeping, shoveling, or vacuuming these surfaces, or by using other similarly effective means of sediment removal. Hosing or sweeping tracked-out sediment into any stormwater conveyance (unless it is connected to a sediment basin, sediment trap, or similarly effective control), storm drain inlet, or surface water.") is prohibited.

[Repeat as needed for individual track-out controls.]

4.4 Stockpiled Sediment or Soil

General

 Stockpiled Material will be at least 100' from any wetlands and will be encircled with a silt sock and construction fence in accordance with detail on Sheet 6.2

Specific Stockpile Controls

Stockpile Control # 1

Stockpiled Sediment/Soil Control Description

- Silt sock and construction fence will be placed around the perimeter of the stockpiled material.
- A tarp or mulch or temporary seeding may also be used to cover stockpiles.
- See SWPP Plan Appendix A and Site Plan

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

• Inspect barriers weekly or after a rain storm and remove sediment if it has reached $\frac{1}{2}$ way up the barrier.

[Repeat as needed for individual stockpile controls.]

4.5 Minimize Dust

General

A water truck will be used for dust control.

Specific Dust Controls

Dust Control # 1

Dust Control Description

A water truck will be used for dust control.

Installation

n/a

Maintenance Requirements

n/a

[Repeat as needed for individual dust controls.]

4.6 Minimize the Disturbance of Steep Slopes

General

There are no steep slopes on the project site.

Specific Steep Slope Controls

Steep Slope Control # 1

Steep Slope Control Description

- INSERT DESCRIPTION OF STEEP SLOPE CONTROL TO BE INSTALLED
- INCLUDE COPIES OF DESIGN SPECIFICATIONS HERE

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE STEEP SLOPE CONTROL

[Repeat as needed for individual steep slope controls.]

4.7 Topsoil

General

 The existing topsoil will stripped and stockpiled on the site and reused in areas of the site where it is appropriate: lawns, road side slopes, storm water basin slopes, etc. The remainder will be removed from the site to locations TBD.

Specific Topsoil Controls

Topsoil Control # 1

Topsoil Control Description

- Topsoil will be stripped and stockpiled on the site and handled in accordance with the specifications of other stockpiles
- See Section 4.4
- See SWPP Plan Appendix A

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

Same as Section 4.4

[Repeat as needed for individual topsoil controls.]

4.8 Soil Compaction

General

 Areas of landscaping will be handled in accordance with local landscaping practice. Storm water basin construction will be handled in accordance with the guidance in the MA DEP Stormwater standards.

Specific Soil Compaction Controls

Soil Compaction Control # 1

Soil Compaction Control Description

- Storm water basin construction will be in accordance with MA DEP Stormwater standards.
- See definitive plans Appendix A

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

- Storm water basins will be cleaned on an annual basis, or more if necessary.
- [Repeat as needed for individual soil compaction controls.]

4.9 Storm Drain Inlets

General

• Storm drain inlets will be protected through the use of silt socks within drainage swales. Catch basins will protected by silt socks around the grates or with silt bags inserted in the structure.

Specific Storm Drain Inlet Controls

Storm Drain Inlet Control # 1

Storm Drain Inlet Control Description

- Silt socks in drainage swales
- See SWPP Plan Appendix A

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

Clean, or remove and replace, the protection measures as sediment accumulates, the filter becomes clogged, and/or performance is compromised. Where there is evidence of sediment accumulation adjacent to the inlet protection measure, you must remove the deposited sediment by the end of the same work day in which it is found or by the end of the following work day if removal by the same work day is not feasible.

Storm Drain Inlet Control # 2

Storm Drain Inlet Control Description

- Silt socks around grates or silt sacks in catch basins
- See SWPP Plan Appendix A

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

Clean, or remove and replace, the protection measures as sediment accumulates, the filter becomes clogged, and/or performance is compromised. Where there is evidence of sediment accumulation adjacent to the inlet protection measure, you must remove the deposited sediment by the end of the same work day in which it is found or by the end of the following work day if removal by the same work day is not feasible.

[Repeat as needed for individual storm drain inlet controls.]

4.10 Constructed Stormwater Conveyance Channels

General

Rip rap devices will be used at all outlets.

Specific Conveyance Channel Controls

<u>Stormwater Conveyance Channel Control # 1</u> Stormwater Conveyance Channel Control Description

- Rip rap outlet to drain outlet pipes
- See Town of Wrentham Construction Standards and Specifications and Site Plan

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

 Rip rap shall be inspected weekly and after every rainsform. If erosion is taking place the stone shall be replenished.

[Repeat as needed for individual stormwater conveyance channel controls.]

4.11 Sediment Basins

General

 Sediment basins will be used as necessary during construction. Once construction is completed they will be revegetated as necessary to bring them in compliance with the permit.

Specific Sediment Basin Controls

Sediment Basin Control # 1

Sediment Basin Control Description

 Sediment basins will be used as necessary during construction. Once construction is completed they will be revegetated as necessary to bring them in compliance with the permit.

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

- Sediment basins will be inspected weekly and after every rain event greater than 0.5". Once the sediment in the forebay reaches 18" of depth the sediment will be removed.
- Once construction has stopped and the site is fully stabilized the basin will be revegetated as necessary to bring it into compliance with the definitive plans.

(Note: At a minimum, you must comply with following requirement in CGP Part 2.1.3.2.b: "Keep in effective operating condition and remove accumulated sediment to maintain at least ½ of the design capacity of the sediment basin at all times.")

[Repeat as needed for individual sediment basin controls.]

4.12 Chemical Treatment

Soil Types

List all the soil types (including soil types expected to be found in fill material) that are expected to be exposed during construction and that will be discharged to locations where chemicals will be applied:

None anticipated

Treatment Chemicals

List all treatment chemicals that will be used at the site and explain why these chemicals are suited to the soil characteristics: INSERT TEXT HERE

Describe the dosage of all treatment chemicals you will use at the site or the methodology you will use to determine dosage: INSERT TEXT HERE

Provide information from any applicable Material Safety Data Sheets (MSDS): INSERT TEXT HERE

Describe how each of the chemicals will stored: INSERT TEXT HERE

Include references to applicable state or local requirements affecting the use of treatment chemicals, and copies of applicable manufacturer's specifications regarding the use of your specific treatment chemicals and/or chemical treatment systems: INSERT TEXT HERE

Special Controls for Cationic Treatment Chemicals (if applicable)

If you have been authorized by your applicable Regional Office to use cationic treatment chemicals, include the official EPA authorization letter or other communication, and identify the specific controls and implementation procedures you are required to implement to ensure that your use of cationic treatment chemicals will not lead to a violation of water quality standards: INSERT (1) ANY LETTERS OR OTHER DOCUMENTS SENT FROM THE EPA REGIONAL OFFICE CONCERNING YOUR USE OF CATIONIC TREATMENT CHEMICALS, AND (2) DESCRIPTION OF ANY SPECIFIC CONTROLS YOU ARE REQUIRED TO IMPLEMENT

Schematic Drawings of Stormwater Controls/Chemical Treatment Systems

Provide schematic drawings of any chemically-enhanced stormwater controls or chemical treatment systems to be used for application of treatment chemicals: INSERT TEXT HERE

Training

Describe the training that personnel who handle and apply chemicals have received prior to permit coverage, or will receive prior to the use of treatment chemicals: INSERT TEXT HERE

4.13 Dewatering Practices

General

Dewatering is not expected to be necessary

Specific Dewatering Practices

Dewatering Practice # 1

Dewaterina Practice Description

- Installation of a sump pipe with trash pump in the area of the excavation
- Discharge will take place in the area designated on the SWPP Plan, which will allow the water to infiltrate into the ground away from the wetlands.

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

 Water removed by dewatering will be discharged to an upland area at least 100' away from the wetlands. Create a stone sump if necessary to ameliorate velocity and to encourage infiltration. If necessary, use silt socks or hay bales to contain. [Repeat as needed for individual dewatering practices.]

4.14 Other Stormwater Controls

General

INSERT GENERAL DESCRIPTION OF THE PROBLEM THIS CONTROL IS DESIGNED TO ADDRESS

Specific Stormwater Control Practices

Stormwater Control Practice # 1

Description

- INSERT DESCRIPTION OF PRACTICE TO BE INSTALLED
- IF APPLICABLE INCLUDE COPIES OF DESIGN SPECIFICATIONS HERE

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE STORMWATER CONTROL PRACTICE

[Repeat as needed.]

4.15 Site Stabilization

Site Stabilization Practice (only use this if you are <u>not</u> located in an arid, semi-arid, or drought-stricken area)
 Description of Practice Temporary stabilization of disturbed areas. No later than 14 days after initiation of soil stabilization measures the portion of the site in question will be planted with temporary cover using either standard seeding or hydroseeding. Seed mixture shall be based on the Massachusetts Conservation Guide Vol. II – Vegetated Practices in Site Development Table 1 – Seedings for Temporary Cover and is dependent on the time of year and the weather conditions.
Installation INSERT APPROXIMATE DATE OF INSTALLATION INSERT APPROXIMATE COMPLETION DATE CONSISTENT WITH CGP PART 2.2.1.2
Maintenance Requirements Seeded areas should be refertilized with $\frac{1}{2}$ of the establishment amount in the second growing season and subsequently as needed.
[Repeat as needed for additional stabilization practices.]
Site Stabilization Practice (only use this if you are located in an arid, semi-arid, or drought-stricken area) Vegetative Non-Vegetative Temporary Permanent

Description of Practice

- Permanent stabilization of disturbed greas.
- Final stabilization in areas to be vegetated will be done in accordance Section 2.2.2 of the general permit.
- Seed mixture shall be based on the Massachusetts Conservation Guide Vol. II Vegetated Practices in Site Development Table 2 – Seed Mixtures for Permanent Cover and is dependent on the time of year and the weather conditions.

Installation

- FOR VEGETATIVE STABILIZATION IN ARID OR SEMI-ARID AREAS, INDICATE THE BEGINNING AND ENDING DATES OF THE SEASONALLY DRY PERIOD AND DESCRIBE YOUR SITE CONDITIONS
- INSERT APPROXIMATE DATE OF INSTALLATION
- INSERT APPROXIMATE COMPLETION DATE CONSISTENT WITH CGP PART 2.2.1.3

Maintenance Requirements

Seeded areas should be refertilized with $\frac{1}{2}$ of the establishment amount in the second growing season and subsequently as needed.

[Repeat as needed for additional stabilization practices.]

Site Stabilization Practice (only use this if uncontrollable circumstances have delayed the initiation or completion of stabilization)

(Note: You will not be able to include this information in your initial SWPPP. If you are affected by circumstances such as those described in CGP Part 2.2.1.3.b, you will need to modify your SWPPP to include this information.)
☐ Vegetative ☐ Non-Vegetative☐ Temporary ☐ Permanent
Justification INSERT DESCRIPTION OF CIRCUMSTANCES THAT PREVENT YOU FROM MEETING THE DEADLINES

REQUIRED IN CGP PARTS 2.2.1.1 AND/OR 2.2.1.2 AND THE SCHEDULE YOU WILL FOLLOW FOR

Description of Practice

- INSERT DESCRIPTION OF STABILIZATION PRACTICE TO BE INSTALLED
- NOTE HOW DESIGN WILL MEET REQUIREMENTS OF PART 2.2.2.1 OR 2.2.2.2, WHICHEVER APPLIES
- INCLUDE COPIES OF DESIGN SPECIFICATIONS HERE

INITIATING AND COMPLETING STABILIZATION

Installation

• INSERT DATES OF INITIATION AND COMPLETION OF NON-VEGETATIVE STABILIZATION CONTROLS (must be completed within 14 days of the cessation of construction)

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE STABILIZATION PRACTICE

[Repeat as needed for additional stabilization practices.]

SECTION 5: POLLUTION PREVENTION STANDARDS

5.1 Potential Sources of Pollution

Construction Site Pollutants

Pollutant-Generating Activity	Pollutants or Pollutant Constituents (that could be discharged if exposed to stormwater)	Location on Site (or reference SWPPP site map where this is shown)
Fueling of vehicles	Gasoline or diesel	Only on paved surfaces, to include existing Commerce Boulevard

[Include additional rows as necessary.]

5.2 Spill Prevention and Response

Any spills of petroleum products will be cleaned using available sorbent material, to include sand, gravel, earth, or other dry clean up measures. If the spill is so large that it enters a catch basin then ensure that the basin is properly emptied so that the materials do not exit the structure. If necessary, contact the Wrentham Fire Department at 911 and direct them to the project site.

5.3 Fueling and Maintenance of Equipment or Vehicles

General

Fueling will only take place on pavement where spills can be readily cleaned-up. Ensure that adequate absorbent, spill clean-up materials are available on the site. If necessary, drip pans will be used under vehicles that leak. Those vehicles shall be removed from the site and repaired before being allowed to return. No storage of fuels or lubricants will take place on site. No maintenance will take place on site.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

 Fueling will only take place on pavement and adequate absorbent, spill clean-up materials will be available on site.

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

Ensure that adequate materials are maintained on site.

[Repeat as needed.]

5.4 Washing of Equipment and Vehicles

General

No washing of equipment or vehicles will be done on site.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

- INSERT DESCRIPTION OF PRACTICE TO BE INSTALLED
- IF APPLICABLE INCLUDE COPIES OF DESIGN SPECIFICATIONS HERE

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE POLLUTION PREVENTION PRACTICE

[Repeat as needed.]

5.5 Storage, Handling, and Disposal of Construction Products, Materials, and Wastes

5.5.1 Building Products

(Note: Examples include asphalt sealants, copper flashing, roofing materials, adhesives, concrete admixtures.)

General

Building products not designed to come in contact with rain will be stored under cover.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

Building products not designed to come in contact with rain will be stored under cover.

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE POLLUTION PREVENTION PRACTICE

[Repeat as needed.]

5.5.2 Pesticides, Herbicides, Insecticides, Fertilizers, and Landscape Materials

General

 Shall not be stored on site. Application shall be done at a rate and in amounts consistent with the manufacturer's specifications.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

- Application shall be done at a rate and in amounts consistent with the manufacturer's specifications.
- See manufacturer's specifications

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE POLLUTION PREVENTION PRACTICE

[Repeat as needed.]

5.5.3 Diesel Fuel, Oil, Hydraulic Fluids, Other Petroleum Products, and Other Chemicals

General

No fuels or petroleum products will be stored on site.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

- INSERT DESCRIPTION OF PRACTICE TO BE INSTALLED
- IF APPLICABLE INCLUDE COPIES OF DESIGN SPECIFICATIONS HERE

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE POLLUTION PREVENTION PRACTICE

[Repeat as needed.]

5.5.4 Hazardous or Toxic Waste

(Note: Examples include paints, solvents, petroleum-based products, wood preservatives, additives, curing compounds, acids.)

General

INSERT GENERAL DESCRIPTION OF HOW YOU WILL COMPLY WITH CGP PART 2.3.3.3.d.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

- INSERT DESCRIPTION OF PRACTICE TO BE INSTALLED
- IF APPLICABLE INCLUDE COPIES OF DESIGN SPECIFICATIONS HERE

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE POLLUTION PREVENTION PRACTICE

[Repeat as needed.]

5.5.5 Construction and Domestic Waste

(Note: Examples include packaging materials, scrap construction materials, masonry products, timber, pipe and electrical cuttings, plastics, styrofoam, concrete, and other trash or building materials.)

General

Dumpsters will be used for waste from the commercial building construction.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

 Dumpsters will be used for materials waste for building construction. The location of the dumpsters will be determined on a case by case basis as the building is built.

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

Remove from site when full.

[Repeat as needed.]

5.5.6 Sanitary Waste

General

Porta-johns will be used on the site for human waste.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

 Porta-johns will be used on the site as necessary. The number of porta-johns will be based on the worker population. Typically, one will be sufficient.

Installation

They will be used on the site from the start to the end of construction.

Maintenance Requirements

 Typical maintenance will involve pumping and cleaning once per week depending on the population size.

[Repeat as needed.]

5.6 Washing of Applicators and Containers used for Paint, Concrete or Other Materials

General

 Direct all washwater into leak proof containers designed so that no overflows can occur. Do not dump liquid wastes in storm sewers. Remove and dispose of hardened concrete in accordance with other solid wastes generated on site.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

- INSERT DESCRIPTION OF PRACTICE TO BE INSTALLED
- IF APPLICABLE INCLUDE COPIES OF DESIGN SPECIFICATIONS HERE

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE POLLUTION PREVENTION PRACTICE

[Repeat as needed.]

5.7 Fertilizers

General

 Shall not be stored on site. Application shall be done at a rate and in amounts consistent with the manufacturer's specifications.

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

 Application shall be done at a rate and in amounts consistent with the manufacturer's specifications.

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE POLLUTION PREVENTION PRACTICE

[Repeat as needed for individual fertilizer practices.]

5.8 Other Pollution Prevention Practices

General

INSERT GENERAL DESCRIPTION OF THE PROBLEM THIS CONTROL IS DESIGNED TO ADDRESS

Specific Pollution Prevention Practices

Pollution Prevention Practice # 1

Description

- INSERT DESCRIPTION OF PRACTICE TO BE INSTALLED
- IF APPLICABLE INCLUDE COPIES OF DESIGN SPECIFICATIONS HERE

Installation

INSERT APPROXIMATE DATE OF INSTALLATION

Maintenance Requirements

INSERT MAINTENANCE REQUIREMENTS FOR THE POLLUTION PREVENTION PRACTICE

[Repeat as needed.]

SECTION 6: INSPECTION AND CORRECTIVE ACTION

6.1 Inspection Personnel and Procedures

Personnel Responsible for Inspections

INSERT NAMES OF PERSONNEL OR TYPES OF PERSONNEL WHO WILL BE CONDUCTING SITE INSPECTIONS HERE

Note: All personnel conducting inspections must be considered a "qualified person." CGP Part 4.1.1 clarifies that a "qualified person" is a person knowledgeable in the principles and practices of erosion and sediment controls and pollution prevention, who possesses the skills to assess conditions at the construction site that could impact stormwater quality, and the skills to assess the effectiveness of any stormwater controls selected and installed to meet the requirements of this permit.

Inspection Schedule

Specific Inspection Frequency

Inspections will take place once every 7 days or more often if a rain event greater then 0.25" has occurred.

Rain Gauge Location (if applicable)

SPECIFY LOCATION(S) OF RAIN GAUGE TO BE USED FOR DETERMINING WHETHER A RAIN EVENT OF 0.25 INCHES OR GREATER HAS OCCURRED (only applies to inspections conducted for Part 4.1.2.2, 4.1.3, or 4.1.4.2)

Reductions in Inspection Frequency (if applicable)

- For the reduction in inspections resulting from stabilization: SPECIFY (1) LOCATIONS WHERE STABILIZATION STEPS HAVE BEEN COMPLETED AND (2) DATE THAT THEY WERE COMPLETED (Note: It is likely that you will not be able to include this in your initial SWPPP. If you qualify for this reduction (see CGP Part 4.1.4.1), you will need to modify your SWPPP to include this information.)
- For the reduction in inspections in arid, semi-arid, or drought-stricken areas: INSERT BEGINNING AND ENDING DATES OF THE SEASONALLY-DEFINED ARID PERIOD FOR YOUR AREA OR THE VALID PERIOD OF DROUGHT
- For reduction in inspections due to frozen conditions: INSERT BEGINNING AND ENDING DATES OF FROZEN CONDITIONS ON YOUR SITE

Inspection Report Forms
See Appendix D

6.2 Corrective Action

Personnel Responsible for Corrective Actions

INSERT NAMES OF PERSONNEL OR TYPES OF PERSONNEL RESPONSIBLE FOR CORRECTIVE ACTIONS

Corrective Action Forms See Appendix E

6.3 Delegation of Authority

Duly Authorized Representative(s) or Position(s):

Insert Company or Organization Name:

Insert Name:

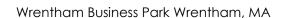
Insert Position:

Insert Address:

Insert City, State, Zip Code:

Insert Telephone Number:

Insert Fax/Email:



SECTION 7: TRAINING

Table 7-1: Documentation for Completion of Training

Name	Date Training Completed
INSERT NAME OF PERSONNEL HERE	INSERT COMPLETION DATE HERE
INSERT NAME OF PERSONNEL HERE	INSERT COMPLETION DATE HERE
INSERT NAME OF PERSONNEL HERE	INSERT COMPLETION DATE HERE
INSERT NAME OF PERSONNEL HERE	INSERT COMPLETION DATE HERE
INSERT NAME OF PERSONNEL HERE	INSERT COMPLETION DATE HERE
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INSERT NAME OF PERSONNEL HERE	INSERT COMPLETION DATE HERE
INSERT NAME OF PERSONNEL HERE	INSERT COMPLETION DATE HERE
INSERT NAME OF PERSONNEL HERE	INSERT COMPLETION DATE HERE



SECTION 8: CERTIFICATION AND NOTIFICATION

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Name:	Title:
Signature:	Date:

[Repeat as needed for multiple construction operators at the site.]

SWPPP APPENDICES

Attach the following documentation to the SWPPP:

Appendix A – Site Maps

Appendix B - Copy of 2017 CGP

Appendix C – NOI and EPA Authorization Email

Appendix D – Inspection Forms
Stormwater Construction Site Inspection Report
Checklist for Catch Basin
Checklist for Cascade

Appendix E - Corrective Action Form

Appendix F – SWPPP Amendment Log

Appendix G – Subcontractor Certifications/Agreements

Appendix H – Grading and Stabilization Activities Log

Appendix I – Training Log

Appendix J – Delegation of Authority

Appendix K – Endangered Species Documentation

Appendix L – Historic Preservation Documentation

Appendix A – Site Maps

INSERT SITE MAPS CONSISTENT WITH TEMPLATE SECTION 2.6



Appendix B - Copy of 2017 CGP

INSERT COPY OF 2017 CGP



Appendix C – Copy of NOI and EPA Authorization email

INSERT COPY OF NOI AND EPA'S AUTHORIZATION EMAIL PROVIDING COVERAGE UNDER THE CGP



Appendix D – Copy of Inspection Form

INSERT COPY OF ANY INSPECTION FORMS YOU WILL USE TO PREPARE INSPECTION REPORTS



Stormwater Construction Site Inspection Report

	General Info	*** *			
Project Name	Wrentham Business Pa	rk Wrentham, MA			
NPDES Tracking No.		Location	10 Commerce Way Wrentham, MA		
Date of Inspection		Start/End Time			
Inspector's Name(s)					
Inspector's Title(s)					
Inspector's Contact Information					
Inspector's Qualifications					
Describe present phase of					
construction					
Type of Inspection:	D.D.:				
☐ Regular ☐ Pre-storm event ☐ During storm event ☐ Post-storm event Weather Information					
Has there been a storm event since If yes, provide:	the last inspection? \(\supersection\)	s D No			
• • •	torm Duration (hrs):	Approximate	Amount of Precipitation (in):		
	. ,				
Weather at time of this inspection?					
☐ Clear ☐ Cloudy ☐ Rain ☐ Other:	☐ Sleet ☐ Fog ☐ Sno	wing High Win	ds		
d Other.	Temperature:				
Have any discharges occurred since the last inspection? □Yes □No If yes, describe:					
Are there any discharges at the tin If yes, describe:	ne of inspection? □Yes □	No			

Site-specific BMPs

- Number the structural and non-structural BMPs identified in your SWPPP on your site map and list them below (add as many BMPs as necessary). Carry a copy of the numbered site map with you during your inspections. This list will ensure that you are inspecting all required BMPs at your site.
- Describe corrective actions initiated, date completed, and note the person that completed the work in the Corrective Action Log.

	BMP	BMP	BMP	Corrective Action Needed and Notes
		Installed?	Maintenance	
			Required?	
1	Stabilized Construction	□Yes □No	□Yes □No	
	Entrances			
2	Silt socks	□Yes □No	□Yes □No	
3	Silt sacks – Catch Basins	□Yes □No	□Yes □No	
4	Sediment Basins	□Yes □No	□Yes □No	
5	Soil Stabilization	□Yes □No	□Yes □No	
6	Material Piles	□Yes □No	□Yes □No	
7	General Housekeeping	□Yes □No	□Yes □No	
9		□Yes □No	□Yes □No	
10		□Yes □No	□Yes □No	
11		□Yes □No	□Yes □No	
12		□Yes □No	□Yes □No	
13		□Yes □No	□Yes □No	

	BMP	BMP	BMP	Corrective Action Needed and Notes
		Installed?	Maintenance	
			Required?	
14		□Yes □No	□Yes □No	
15		□Yes □No	□Yes □No	
16		□Yes □No	□Yes □No	
17		□Yes □No	□Yes □No	
18		□Yes □No	□Yes □No	
19		□Yes □No	□Yes □No	
20		□Yes □No	□Yes □No	

Overall Site Issues

Below are some general site issues that should be assessed during inspections. Customize this list as needed for conditions and phases of construction. Use Project SWPPP (Sheets 6.0 & 61) for inspection.

	BMP/activity	Implemented?	Maintenance Required?	Corrective Action Needed and Notes
5	Are all slopes and disturbed areas not actively being worked properly stabilized?	□Yes □No	□Yes □No	
2,3	Are future infiltration basin area protected with barriers or similar BMPs?	□Yes □No	□Yes □No	
2,3	Are perimeter controls and sediment barriers adequately installed (keyed into substrate) and maintained?	□Yes □No	□Yes □No	
4	Are sediment basins functioning properly?	□Yes □No	□Yes □No	
3	Are storm drain inlets properly protected?	□Yes □No	□Yes □No	
1	Is the construction exit preventing sediment from being tracked into the street?	□Yes □No	□Yes □No	
7	Is trash/litter from work areas collected and placed in covered dumpsters?	□Yes □No	□Yes □No	
7	Are vehicle and equipment fueling, cleaning, and maintenance areas free of spills, leaks, or any other deleterious material?	□Yes □No	□Yes □No	

	BMP/activity	Implemented?	Maintenance Required?	Corrective Action Needed and Notes
	Are material piles covered or seeded and surrounded by sediment	□Yes □No	□Yes □No	
	control barrier?	□Yes □No	□Yes □No	
		☐Yes ☐No	☐Yes ☐No	
		□Yes □No	□Yes □No	
	Non-Compliance			
S	cribe any incidents of non-c	compliance not des	scribed above:	
_		CER	TIFICATION ST	ATEMENT
	supervision in accordance v the information submitted. I directly responsible for gath	aw that this docun vith a system desig Based on my inqu nering the informa omplete. I am awa	nent and all attach gned to assure that iry of the person o tion, the informati re that there are si	ments were prepared under my direction or qualified personnel properly gathered and evaluated r persons who manage the system, or those persons on submitted is, to the best of my knowledge and gnificant penalties for submitting false information,
	supervision in accordance v the information submitted. I directly responsible for gath belief, true, accurate, and co	aw that this docun vith a system design Based on my inqual nering the informatom complete. I am awa fine and imprison	nent and all attach gned to assure that iry of the person o tion, the informati re that there are si ment for knowing	ments were prepared under my direction or qualified personnel properly gathered and evaluated r persons who manage the system, or those persons on submitted is, to the best of my knowledge and gnificant penalties for submitting false information, violations."
	supervision in accordance v the information submitted. I directly responsible for gath belief, true, accurate, and co including the possibility of	aw that this docun with a system design Based on my inqual nering the informatomplete. I am awa fine and imprison	nent and all attach gned to assure that iry of the person o tion, the informati re that there are si ment for knowing	ments were prepared under my direction or qualified personnel properly gathered and evaluate r persons who manage the system, or those persons on submitted is, to the best of my knowledge and gnificant penalties for submitting false information, violations."

CHECKLIST FOR INSPECTION OF CATCH BASIN						
CHECKLIST FOR INSPECTION OF CATCH BASIN						
Location:		,				
Inspector:						
Date/Time:						
Weather:						
Date of Last Rainfall:						
Amount of Last Rainfall:						
Inspection Items:		Satisfactory Unsatisfact		Comments/ Corrective Actions		
Damage to frame/cover		S	U			
Settlement of frame/cover		S	U			
Depth of sediment in basin		S	U			
Condition of water quality hood		S	U			
Condition of inlet from Tree Box	Filter	S	U			
Corrective Action Needed				Due Date		
1						
2						
3						
4						
5						

CHECKLIST FOR INSPECTION OF CASCADE						
Location:						
Inspector:						
Date/Time:						
Weather:						
Date of Last Rainfall:						
Amount of Last Rainfall:						
Inspection Items: Satisfactory Unsatisfact			*	Comments/ Corrective Actions		
Blockage or obstruction in inlet ch	amber, flume, outlet	S	U			
Check for presence of hydrocarbo	ns	S	U			
Depth of Sediment		S	U			
Other issue		S	U			
Corrective Action Needed				Due Date		
1						
2						
3						
4						
5						

Note: Inspection and maintenance shall be in accordance with the most recent Cascades Inspection and Maintenance information which can be found at https://www.conteches.com/technical-guides/search?filter=W0ME2011K1. Inspector shall use that document during the inspection and follow the recomendations therein.

Appendix E – Copy of Corrective Action Form

INSERT COPY OF CORRECTIVE ACTION FORMS YOU WILL USE



Appendix F – SWPPP Amendment Log

No.	Description of the Amendment	Date of Amendment	Amendment Prepared by [Name(s) and Title]

Appendix G – Sample Subcontractor Certifications/Agreements

SUBCONTRACTOR CERTIFICATION STORMWATER POLLUTION PREVENTION PLAN

Project Number:	
Project Title:	
Operator(s):	
As a subcontractor, you are required to comply with the Stormwater Pollution Prevention Pl (SWPPP) for any work that you perform on-site. Any person or group who violates any cond of the SWPPP may be subject to substantial penalties or loss of contract. You are encourage advise each of your employees working on this project of the requirements of the SWPPP. A copy of the SWPPP is available for your review at the office trailer.	dition ged to
Each subcontractor engaged in activities at the construction site that could impact storms must be identified and sign the following certification statement:	vater
I certify under the penalty of law that I have read and understand the terms and conditions the SWPPP for the above designated project and agree to follow the practices described in SWPPP.	
This certification is hereby signed in reference to the above named project:	
Company:	
Address:	
Telephone Number:	
Type of construction service to be provided:	
Signature:	
Title:	
Date:	

Appendix H –Grading and Stabilization Activities Log

Date Grading	Description of Grading Activity	Description of Stabilization Measure and Location	Date Grading Activity Ceased	Date When Stabilization
Activity Initiated			(Indicate Temporary or Permanent)	Measures Initiated

Appendix I - SWPPP Training Log

Stormwater Pollution Prevention Training Log Project Name: Project Location: Instructor's Name(s): Instructor's Title(s): Course Location: Date: Course Length (hours): _____ Stormwater Training Topic: (check as appropriate) Sediment and Erosion □ Emergency Procedures Controls □ Stabilization Controls ☐ Inspections/Corrective Actions ☐ Pollution Prevention Measures Specific Training Objective:

Attendee Roster: (attach additional pages as necessary)

No.	Name of Attendee	Company
1		
2		
3		
4		
5		
6		
7		
8		

Appendix J – Delegation of Authority Form

Del	legation	$\cap f$	Auth	ority
-	icganon.	O1	Δ	

Delegation of Authority
I, (name), hereby designate the person or specifically described position below to be a duly authorized representative for the purpose of overseeing compliance with environmental requirements, including the Construction General Permit, at the construction site. The designee is authorized to sign any reports, stormwater pollution prevention plans and all other documents required by the permit.
(name of person or position) (company) (address) (city, state, zip) (phone)
By signing this authorization, I confirm that I meet the requirements to make such a designation as set forth in Appendix I of EPA's Construction General Permit (CGP), and that the designee above meets the definition of a "duly authorized representative" as set forth in Appendix I.
I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.
Name:
Company:
Title:
Signature:
Date:

Appendix K – Endangered Species Documentation

INSERT DOCUMENTATION CONSISTENT WITH SWPPP TEMPLATE SECTION 3.1



Appendix L – Historic Properties Documentation

INSERT DOCUMENTATION CONSISTENT WITH SWPPP TEMPLATE SECTION 3.2



APPENDIX F - Soils Data

Soil Evaluation Forms NRCS Soil Report Extract

No.	20-0219	Date:	April 27, 2022
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Commonwealth of Massachusetts

Wrentham, Massachusetts

Soil Suitability Assessment for On-Site Sewage Disposal

Performed By: Richard Leslie	Date: April 27, 2022
Witnessed By: Wade Saucier	
Location Address or Lot #:	Owner's Name, Address, and , Telephone #:
10 Commerce Boulevard	Edgewood Development Company, LLC
Wrentham, Ma 02093 New Construction: Repair □	320 South Street, Suite 202 Plainville, MA 02762 508.643.2920
New Construction: Repair	1 Idilivine, 1911 02/02 300.043.2920
Office Review	
Published Soil Survey Available: No 🔲	Yes 🗾
Year Published 1989 Publication	Scale 1:25,000 Soil Map Unit Hinckley SL
Drainage Class A Soil Limita	P 1 1
Surficial Geology Report Available: No	Yes 🔽
Year Published 1992 Publication	Scale 1:250,000
Geologic Material (Map Unit) Coarse Deposits	
Landform Glacial Outwash Plain	
Flood Insurance Rate Map:	
Above 500 year flood boundary No 🔲	Yes 🗹
Within 500 year flood boundary No 🔽	Yes
Within 100 year flood boundary No 🗸	Yes
Wetland Area:	
National Wetland Inventory Map (map unit)	
Wetlands Conservancy Program Map (map unit)	
	N. 1. 2022
Current Water Resource Conditions (USGS): Month	March, 2022
Range: Above Normal Normal V	Below Normal
Other References Reviewed:	

Location Address or Lot No. 10	0 Commerce Boulevard
--------------------------------	----------------------

On-site Review

Deep Hole Num	ber: <u>2022-1</u>	Date: <u>4/26/</u>	2022	Time	e: <u>0830</u>	Weather: 40°/Cloudy		
Location (identif	y on site plan)	See site plan						
Land Use Parking Area Slope (%) 1% Surface Stones None								
Vegetation None								
Landform Glacial Outwash Plain								
Position on landscape (sketch on back) See site plan								
Distances from:								
Open W	ater Body	>200'	_	Drain	nageway >100'			
Possible	e Wet Area	>100'		Prop	erty Line <u>25'+/-</u>			
Drinking	g Water Well	>100'	_	Othe				
		DEEP O)BSERVA	TION HOL	E LOG*			
Depth from Surface (inches)	Soil Horizon	Soil Texture (USDA)	Soil Color (Munsell)	Soil Mottling	(Structure, Stones, I	Other Boulders, Consistency, % Gravel)		
					No A	or B layer		
0" - 168"	С	Loamy Sand	2.5Y5/4		Gravelly,	Cobbly, Coarse		
*MINIMU Parent Material Depth to Groun Estimated Seaso	dwater Standi	acial outwash	l .			to Bedrock:		

REPRODUCTION OF DEP APPROVED FORM DATED 12-07-95

Location Address or Lot No.	10 Commerce Boulevard
-----------------------------	-----------------------

On-site Review

Deep Hole Numl	ber: <u>2022-2</u>	Date: <u>4/26/</u> 2	2022	Time	e: <u>0900</u>	Weather: 40°/Cloudy
Location (identif	y on site plan)	See site plan				
Land Use Park	king Area	Slope	e (%) 1%		Surface Stones	None
Vegetation No	ne					
Landform Gla	cial Outwash	Plain				
Position on lands	scape (sketch or	back) See s	ite plan			
Distances from:						
Open W	ater Body	>200'	_	Drain	nageway >100'	
Possible	Wet Area	>100'	_	Prop	erty Line <u>25'+/-</u>	
Drinking	g Water Well	>100'	_	Othe		
		DEEP O	DBSERVAT	TION HOL	E LOG*	
Depth from Surface (inches)	Soil Horizon	Soil Texture (USDA)	Soil Color (Munsell)	Soil Mottling	(Structure, Stones, I	Other Boulders, Consistency, % Gravel)
0" - 48"	Fill					
48" - 80"	A+B	Sandy Loam				
80" - 176"	С	Loamy Sand	2.5Y5/4			Cobbly, Coarse, s of fine sand
		REQUIRED AT EV		AL AREA		'
Parent Material	(geologic) Gl	acıal outwash	<u> </u>			to Bedrock:
Depth to Ground		-	ole:		Weeping from l	Pit Face:
Estimated Seaso	mai mign Grou	nawater:				

REPRODUCTION OF DEP APPROVED FORM DATED 12-07-95

Location Address or Lot No.	10 Commerce Boulevard
-----------------------------	-----------------------

REPRODUCTION OF DEP APPROVED FORM DATED 12-07-95

On-site Review

D II.1. N 1	2022-3	D. 4. 4/26/	2022	Т:	. 0930	Weather: 40°/Cloudy	
Location (identify	• •						
Land Use Parking Area Slope (%) 1% Surface Stones None							
Vegetation Non	ne						
Landform <u>Gla</u>	cial Outwash	Plain					
Position on lands	cape (sketch or	back) See s	ite plan				
Distances from:							
Open W	ater Body	>200'	_	Draii	nageway >100'		
Possible	Wet Area	>100'	_	Prop	erty Line <u>25'+/-</u>		
Drinking	g Water Well	>100'	_	Othe			
		DEEP O	BSERVA	TION HOL	E LOG*		
Depth from Surface (inches)	Soil Horizon	Soil Texture (USDA)	Soil Color (Munsell)	Soil Mottling	(Structure, Stones, I	Other Boulders, Consistency, % Gravel)	
0" - 48"	Fill						
48" - 80"	A+B	Sandy Loam					
80" - 168"	С	Loamy Sand	2.5Y5/4		Gravelly,	Cobbly, Coarse	
*MINIMUI Parent Material		REQUIRED AT EV acial outwash		AL AKEA	Depth t	to Bedrock:	
Depth to Ground Estimated Seaso		-	ole:			Pit Face:	

Location Address or Lot No. 10 Commerce Boulevard

Commonwealth of Massachusetts

Wrentham, Massachusetts

Percolation Test*						
Date: April 27, 2022 Time: 1130						
Observation Hole #	2022-1	2022-2				
Depth of Perc	36"	80"				
Start Pre-soak	1151	1132				
End Pre-soak	1206	1147				
Time at 12"	1206	1147				
Time at 9"	1208	1156				
Time at 6"	1211	1210				
Time (9" – 6")	3	14				
Rate Min./Inch	2	5				

* Minimum of 1 percolation test must be performed in both the primary area AND reserve area.

Site Passed 🗸	Site Failed
Performed By:	Richard Leslie
Witnessed By:	Wade Saucier
Comments:	
REPRODUCTION OF DEP	APPROVED FORM DATED 12-07-95

Location Address or Lot No. 10 Commerce Boulevard

Commonwealth of Massachusetts

Wrentham, Massachusetts

Percolation Test* Date: April 27, 2022 Time: 1120						
Observation Hole #	2022-3	1				
Depth of Perc	80"					
Start Pre-soak	1125					
End Pre-soak	1140					
Time at 12"	1140					
Time at 9"	1142					
Time at 6"	1145					
Time (9" – 6")	3					
Rate Min./Inch	2					

* Minimum of 1 percolation test must be performed in both the primary area AND reserve area.

Site Passed 🗸	Site Failed		
Performed By:	Richard Leslie		
Witnessed By:	Wade Saucier		
Comments:			
REPRODUCTION OF DEP APPROVED FORM DATED 12-07-95			

Determination for Seasonal High Water Table

Method Used:				
☐ Depth observed standing ☐ Depth weeping from side ☐ Depth to soil mottles ☐ ☐ Ground water adjustmen		inches inches		
Index Well Number	Reading Date	Index well level		
Adjustment factor Adjusted groundwater level				
Depth of Naturally Occurring P	ervious Material			
Does at least four feet of natu throughout the area proposed	rally occurring pervious material for the soil absorption system?	exist in all areas observed Yes		
If not, what is the depth of na	turally occurring pervious materi	al?		
the Department of Environme	7 (date) I have passed the soil ental Protection and that the above raining expertise and experience of			
Signature		Date		



Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Norfolk and Suffolk Counties, Massachusetts

Wrentham Business Park Wrentham, MA



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2 053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

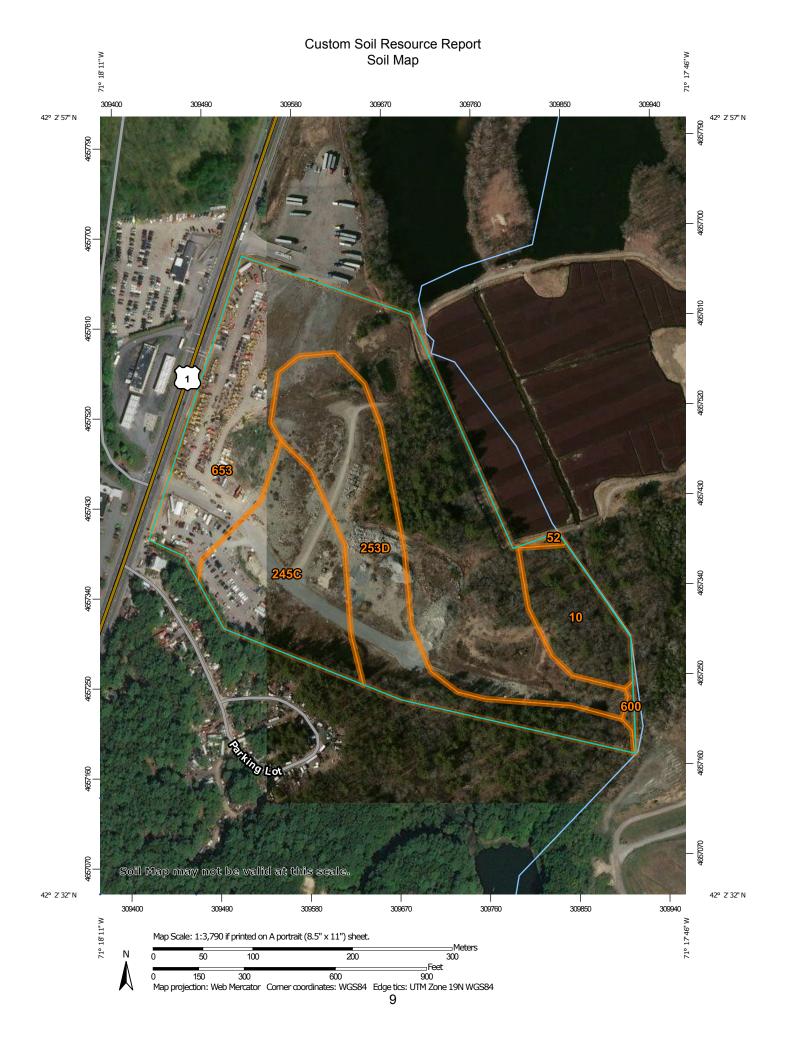
Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



MAP LEGEND

Special Line Features Streams and Canals Interstate Highways Very Stony Spot Major Roads Local Roads Stony Spot US Routes Spoil Area Wet Spot Other Rails Nater Features **Fransportation 3ackground** W 8 ◁ ŧ Soil Map Unit Polygons Area of Interest (AOI) Soil Map Unit Points Soil Map Unit Lines Closed Depression Special Point Features **Gravelly Spot Borrow Pit** Lava Flow Clay Spot **Gravel Pit** Area of Interest (AOI) Blowout Landfill 9 Soils

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857) Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

Aerial Photography

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

Saline Spot Sandy Spot

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Norfolk and Suffolk Counties, Massachusetts Survey Area Data: Version 12, Sep 15, 2016

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Severely Eroded Spot

Slide or Slip Sodic Spot

Sinkhole

Date(s) aerial images were photographed: May 14, 2010—Apr 1, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Norfolk and Suffolk Counties, Massachusetts (MA616)					
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI		
10	Scarboro and Birdsall soils, 0 to 3 percent slopes	2.5	7.6%		
52	Freetown muck, 0 to 1 percent slopes	0.1	0.3%		
245C	Hinckley loamy sand, 8 to 15 percent slopes	5.4	16.5%		
253D	Hinckley loamy sand, 15 to 35 percent slopes	7.6	23.3%		
600	Pits, sand and gravel	0.1	0.3%		
653	Udorthents, sandy	17.1	52.1%		
Totals for Area of Interest	,	32.7	100.0%		

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it

was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Norfolk and Suffolk Counties, Massachusetts

10—Scarboro and Birdsall soils, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: vkxw Elevation: 0 to 2,100 feet

Mean annual precipitation: 45 to 54 inches Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Scarboro and similar soils: 65 percent Birdsall and similar soils: 25 percent Minor components: 10 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Scarboro

Setting

Landform: Terraces

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Loose sandy glaciofluvial deposits

Typical profile

H1 - 0 to 9 inches: mucky fine sandy loam

H2 - 9 to 60 inches: stratified loamy fine sand to gravelly coarse sand

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Very poorly drained

Runoff class: Negligible

Capacity of the most limiting layer to transmit water (Ksat): High to very high (6.00

to 20.00 in/hr)

Depth to water table: About 0 inches

Frequency of flooding: None Frequency of ponding: Frequent

Available water storage in profile: Low (about 5.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 5w

Hydrologic Soil Group: A/D Hydric soil rating: Yes

Description of Birdsall

Setting

Landform: Terraces

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Soft coarse-silty glaciolacustrine deposits

Typical profile

H1 - 0 to 8 inches: very fine sandy loam H2 - 8 to 16 inches: very fine sandy loam

H3 - 16 to 60 inches: silt loam

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Very poorly drained

Runoff class: Negligible

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.20 in/hr) Depth to water table: About 0 inches

Frequency of flooding: None Frequency of ponding: Frequent

Available water storage in profile: Very high (about 12.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 5w

Hydrologic Soil Group: C/D Hydric soil rating: Yes

Minor Components

Swansea

Percent of map unit: 5 percent

Landform: Bogs Hydric soil rating: Yes

Raynham

Percent of map unit: 3 percent Landform: Depressions Hydric soil rating: Yes

Walpole

Percent of map unit: 2 percent

Landform: Terraces
Hydric soil rating: Yes

52—Freetown muck, 0 to 1 percent slopes

Map Unit Setting

National map unit symbol: 2t2q9

Elevation: 0 to 1,110 feet

Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Farmland of unique importance

Map Unit Composition

Freetown and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Freetown

Setting

Landform: Bogs, depressions, depressions, kettles, marshes, swamps

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread, dip

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Highly decomposed organic material

Typical profile

Oe - 0 to 2 inches: mucky peat Oa - 2 to 79 inches: muck

Properties and qualities

Slope: 0 to 1 percent

Percent of area covered with surface fragments: 0.0 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Very poorly drained

Runoff class: Negligible

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high

(0.14 to 14.17 in/hr)

Depth to water table: About 0 to 6 inches

Frequency of flooding: Rare Frequency of ponding: Frequent

Available water storage in profile: Very high (about 19.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 5w

Hydrologic Soil Group: B/D Hydric soil rating: Yes

Minor Components

Whitman

Percent of map unit: 5 percent

Landform: Depressions, drainageways

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Scarboro

Percent of map unit: 5 percent

Landform: Depressions, drainageways

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Base slope, tread, dip

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Swansea

Percent of map unit: 5 percent

Landform: Bogs, depressions, depressions, kettles, marshes, swamps

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread, dip

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

245C—Hinckley loamy sand, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 2svm9

Elevation: 0 to 1,480 feet

Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Hinckley and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hinckley

Setting

Landform: Eskers, kames, kame terraces, outwash plains, outwash terraces, moraines, outwash deltas

Landform position (two-dimensional): Shoulder, toeslope, footslope, backslope Landform position (three-dimensional): Crest, head slope, nose slope, side slope, riser

Down-slope shape: Convex, concave, linear Across-slope shape: Concave, linear, convex

Parent material: Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material

A - 1 to 8 inches: loamy sand

Bw1 - 8 to 11 inches: gravelly loamy sand Bw2 - 11 to 16 inches: gravelly loamy sand BC - 16 to 19 inches: very gravelly loamy sand

C - 19 to 65 inches: very gravelly sand

Properties and qualities

Slope: 8 to 15 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

very high (1.42 to 99.90 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Available water storage in profile: Low (about 3.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4e

Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Windsor

Percent of map unit: 5 percent

Landform: Eskers, kames, kame terraces, outwash plains, outwash terraces, moraines, outwash deltas

Landform position (two-dimensional): Shoulder, backslope, footslope, toeslope Landform position (three-dimensional): Nose slope, side slope, crest, head slope,

Down-slope shape: Convex, concave, linear Across-slope shape: Concave, linear, convex

Hydric soil rating: No

Sudbury

Percent of map unit: 5 percent

Landform: Kame terraces, outwash plains, outwash terraces, moraines, outwash deltas

Landform position (two-dimensional): Backslope, footslope Landform position (three-dimensional): Base slope, tread

Down-slope shape: Concave, linear Across-slope shape: Linear, concave

Hydric soil rating: No

Merrimac

Percent of map unit: 5 percent

Landform: Eskers, kames, outwash plains, outwash terraces, moraines

Landform position (two-dimensional): Shoulder, backslope, footslope, toeslope

Landform position (three-dimensional): Side slope, head slope, nose slope, crest, riser

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

253D—Hinckley loamy sand, 15 to 35 percent slopes

Map Unit Setting

National map unit symbol: 2svmd

Elevation: 0 to 860 feet

Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Hinckley and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hinckley

Setting

Landform: Eskers, kames, kame terraces, outwash plains, outwash terraces,

moraines, outwash deltas

Landform position (two-dimensional): Backslope

Landform position (three-dimensional): Crest, nose slope, side slope, head slope,

riser

Down-slope shape: Linear, convex, concave Across-slope shape: Linear, concave, convex

Parent material: Sandy and gravelly glaciofluvial deposits derived from gneiss

and/or granite and/or schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material

A - 1 to 8 inches: loamy sand

Bw1 - 8 to 11 inches: gravelly loamy sand Bw2 - 11 to 16 inches: gravelly loamy sand BC - 16 to 19 inches: very gravelly loamy sand

C - 19 to 65 inches: very gravelly sand

Properties and qualities

Slope: 15 to 35 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

very high (1.42 to 99.90 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Available water storage in profile: Low (about 3.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6e

Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Windsor

Percent of map unit: 10 percent

Landform: Eskers, kames, kame terraces, outwash plains, outwash terraces,

moraines, outwash deltas

Landform position (two-dimensional): Backslope

Landform position (three-dimensional): Crest, nose slope, side slope, head slope,

riser

Down-slope shape: Concave, convex, linear Across-slope shape: Linear, concave, convex

Hydric soil rating: No

Merrimac

Percent of map unit: 3 percent

Landform: Eskers, kames, kame terraces, outwash plains, outwash terraces,

moraines

Landform position (two-dimensional): Backslope

Landform position (three-dimensional): Side slope, head slope, nose slope, crest,

riser

Down-slope shape: Convex, linear, concave Across-slope shape: Linear, convex, concave

Hydric soil rating: No

Sudburv

Percent of map unit: 2 percent

Landform: Kame terraces, outwash plains, outwash terraces, moraines, outwash

deltas

Landform position (two-dimensional): Backslope, footslope, toeslope

Landform position (three-dimensional): Base slope, tread

Down-slope shape: Linear, concave Across-slope shape: Concave, linear

Hydric soil rating: No

600—Pits, sand and gravel

Map Unit Setting

National map unit symbol: vkxc

Mean annual precipitation: 32 to 50 inches Mean annual air temperature: 45 to 50 degrees F

Frost-free period: 120 to 200 days

Farmland classification: Not prime farmland

Map Unit Composition

Pits: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Pits

Setting

Parent material: Loose, excavated sandy and gravelly glaciofluvial deposits

653—Udorthents, sandy

Map Unit Setting

National map unit symbol: vky8 Elevation: 0 to 3.000 feet

Mean annual precipitation: 45 to 54 inches

Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Udorthents and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents

Setting

Landform position (two-dimensional): Shoulder, summit Landform position (three-dimensional): Riser, tread

Down-slope shape: Convex, linear Across-slope shape: Convex, linear

Parent material: Excavated and filled sandy glaciofluvial deposits

Typical profile

H1 - 0 to 6 inches: variable H2 - 6 to 60 inches: variable

Properties and qualities

Slope: 0 to 25 percent

Depth to restrictive feature: More than 80 inches

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to very

high (0.06 to 20.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: A
Hydric soil rating: Unranked

Minor Components

Udorthents

Percent of map unit: 8 percent Hydric soil rating: Unranked

Urban land

Percent of map unit: 5 percent Hydric soil rating: Unranked

Swansea

Percent of map unit: 2 percent

Landform: Bogs Hydric soil rating: Yes

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